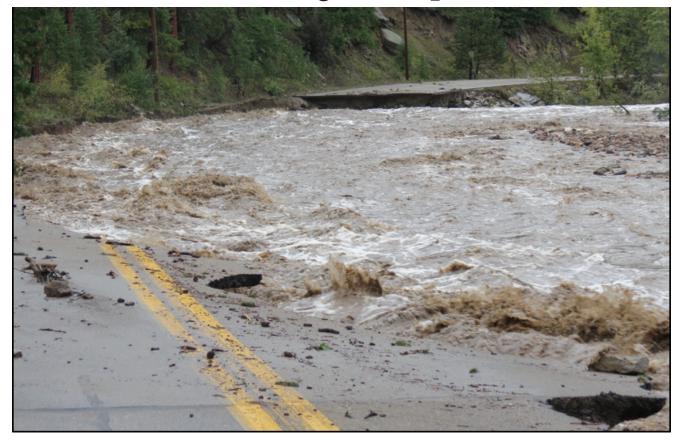
Post-Construction Project Report of the Larimer County Road 43 River Restoration

North Fork of the Big Thompson River



Prepared for:



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Federal Highway Administration

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EXECUTIVE SUMMARY

In September 2013, the Colorado Front Range experienced a large flood event that ravaged rivers, communities, and infrastructure, damaging and destroying large sections of highways, local roads, and public, residential, and commercial properties. Large portions of Larimer County Road 43 (CR 43), which follows the North Fork of the Big Thompson River for approximately 8 miles near downstream of the hamlet of Glen Haven, was heavily damaged as a result of flooding and erosion. In conjunction with the road reconstruction led by the Central Federal Lands (CFL) division of the Federal Highway Administration (FHWA), river restoration activities were conducted along most of the North Fork of the Big Thompson River where the river runs adjacent to the roadway. This memorandum provides the technical details and resources used as a basis of design for the stream restoration project.

This report describes channel reconstruction activities; demonstrates the cost-effectiveness of collaboration between entities; lists lessons learned from this project; and identifies next steps for the CR 43 stream corridor. The CR 43 river restoration project was a success because materials, equipment, manpower, and expertise were leveraged from the highway reconstruction efforts in order to restore the river channel for the benefit of both the river and roadway. This coordinated effort helped to identify cost-effective opportunities and best practices for stream rehabilitation within the roadway corridor.

The primary intent of this stream rehabilitation project was to reconstruct a channel capable of efficient sediment transport, effective water conveyance at a range of high and low flows, and the establishment of relative lateral and vertical stability for the benefit of long-term protection of CR 43. These goals were accomplished through the creation of a multi-stage channel and increased in-channel habitat complexity via the strategic placement of boulders, rock structures, and large wood. In order to develop a comprehensive and holistic approach to the rehabilitation of the channel and its floodplain, adjacent riverfront landowners were approached and asked for permission to incorporate restoration treatments on their sections of the river. The nearly 100 percent acceptance rate eliminated a patchwork of management actions and facilitated a coherent, consistent restoration project.

This river restoration project was designed and built around restoring river processes, not just river form. The features that were built are intended to encourage long-term system stability by providing a framework within which natural sediment movement and ecosystem processes can dominate. The river, and low-flow channel in particular, is expected to move and adjust; the river system is expected to recruit woody vegetation and woody debris; and the bankfull channel is expected to scour and form new deposits as it adjusts to varying flow and sediment loading cycles in the coming years and decades. Evidence of these adjustments and observations of change will deem this project a success, not a failure. A monitoring program to record and measure these changes is recommended. In order to maintain and capitalize on improvements made through this river restoration effort, important future activities to be conducted at the CR 43 project sites include this long-term monitoring as well as active riparian revegetation.

The ability to utilize materials from on site and equipment from the road construction project is crucial to developing a cost-effective and holistic project. Budget data from the CR 43 project was not available for analysis so only generalizations can be made but cost savings have been shown to be substantial in previous projects. As a rough comparison, the US 36 project restored 10,750 linear feet of channel while the CR 43 project restored 17,500 linear feet of channel – a 61% increase. The cost savings between the US 36 project and the CR 43 project are likely not linear but if they were the savings would be \$1.77 million.

The CR 43 river restoration project had several constraints that influenced the recommended design, particularly the encroachment of the roadway embankment on the river channel corridor. Implementation of the river restoration project was influenced by an accelerated timeline, availability of construction equipment, and transport of needed construction materials. However, useful lessons were learned throughout the process that can benefit future flood recovery efforts and other restoration projects that involve a shared roadway/river corridor.

1.0 BACKGROUND

The North Fork Big Thompson River is approximately 7.5 miles long with tributary creeks originating in the northeast corner of Rocky Mountain National Park and flowing in a southeast direction through the hamlet of Glen Haven to its confluence with the Big Thompson River at Drake. The tributary area of the North Fork at Drake is approximately 86 square miles, and the North Fork carries approximately 25 percent of the flows of the Big Thompson River main stem.

In September 2013, significant rainfall along the Front Range of the Colorado Rocky Mountains resulted in the worst flooding seen in these areas in more than 30 years. These floods caused significant damage to the roadway system that links the western regions of Boulder and Larimer counties to the populated plains cities, and many of these mountain communities were isolated for more than a week. Larimer County Road 43—a major thoroughfare between the Glen Haven and Drake—was one of these impacted corridors (Figure 1).



Figure 1. Example of road damage as a result of the 2013 flood

County Road 43 travels along the North Fork of the Big Thompson River as it winds from the town of Estes Park east through Glen Haven toward the Town of Drake at the confluence with the main stem of the Big Thompson River. There was severe damage to the roadway and some residents had to hike out or be rescued by helicopter.

The North Fork of the Big Thompson River originates in the Roosevelt National Forest at the confluence of West Creek and Fox Creek at the Town of Glen Haven and flows in a easterly direction along CR 43 for approximately 8 miles until it joins the mainstem of the Big Thompson River at Drake.

Following the flood, a temporary road was rebuilt quickly with the help of the Larimer County Public Works Department, and permanent road reconstruction was designed and engineered by Central Federal Lands (CFL), a Division of the Federal Highway Administration (FHWA), and administered by Larimer County to improve resiliency and reduce the likelihood of future flood damage. Relocating the road onto inside bend bedrock benches, elevating of the road bed, flipping the location of the road and the river to keep the river shear stress up against the outside bends of the valley bedrock, replacing culvert crossings with bridges and armoring of the road embankment with large mortared boulders were major design goals. Within the road design and reconstruction process, a unique opportunity was presented for the CR 43 reconstruction project to partner with stream restoration experts to restore the stream channel in conjunction with and with benefit to the road work. CFL and Larimer County sought out Crane Associates (referred to collectively as the Stream Recovery Team) to help guide the river restoration efforts. Jeff Crane, Katie Jagt and Michael Blazewicz made up the Stream Team.

The partnership afforded an opportunity to utilize road reconstruction resources to design and construct stream channel enhancements adjacent to roadway repairs. The Stream Recovery Team leveraged materials, equipment, manpower, and expertise on-site to reconstruct North Fork of the Big Thompson River in the vicinity of the road repair sites (Figure 2). As a result of outreach by Larimer County, nearly all riverfront landowners adjacent to these sites agreed to provide access and participate in this comprehensive public/private partnership to eliminate a patchwork of treatments and implement a consistent design for the stream rehabilitation. Major components of the stream corridor reconstruction included: a low-flow channel, gravel bars and benches, large wood structures, habitat boulders and boulder clusters, grade control structures, side channel habitats, and riffle and pool enhancement. The Stream Recovery Team also worked with Wildland Restoration Volunteers and the US Forest Service to revegetate the riparian corridor.

This coordinated effort helped to identify opportunities and best practices for stream rehabilitation within a highway corridor. The purpose of this report is to outline the process by which the channel reconstruction work was conducted; demonstrate the cost effectiveness of this type of collaboration so that it can be replicated in the future; describe lessons learned from this project; and identify next steps for the CR 43 stream corridor.

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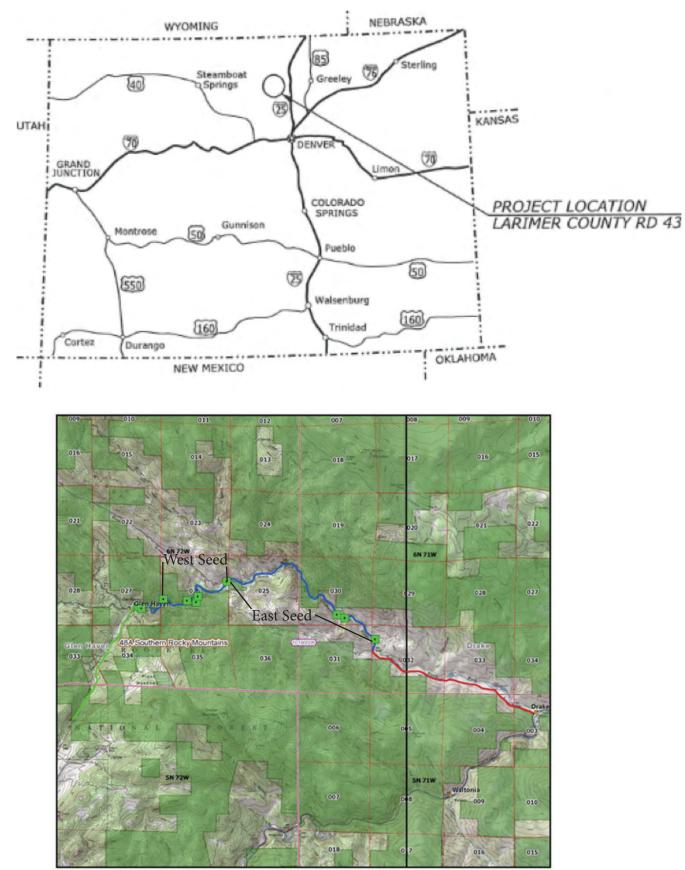


Figure 2. Project location map.

2.0 POST-FLOOD CONDITIONS

The North Fork of the Big Thompson River channel, squeezed by natural canyon walls and encroached on by CR 43, underwent significant bed scour and lateral erosion during the September flood. A National Guard and Larimer County damage assessment team was deployed immediately after the rains stopped. Between the Towns of Glen Haven and Drake, the team identified dozens of sites where CR 43 had been significantly damaged or completely destroyed. At each of these locations, the stream had reclaimed its historic corridor by undermining and overtaking the roadway embankment and washing out culvert crossings. The washouts caused large sections of roadway, including asphalt, road base, guardrail, and signing, to collapse into the river. The road material amassed in the river channel, along with uprooted trees, cars, river rock, and a collection of debris from flooded houses. These and other flood-mobilized materials were deposited in the channel and on the floodplain, leaving debris fields and scour zones.

The resulting post-flood streams were eventually re-channelized into their pre-flood alignment by Larimer County and private contractors hired to provide temporary emergency access. Newly scoured bends and areas of lateral migration were filled during this process and the stream beds were left in a raw and unstable condition. Following the bridge construction and preliminary road alignments (beginning in November 2015), the Stream Recovery Team was provided with materials and equipment to implement stream channel restoration (Fig. 3).



Figure 3. Channelized creek following CR 43 reconstruction and prior to stream restoration.

3.0 DESIGN GOALS

The primary intent of the river restoration project design was to relocate the creek, wherever possible, to keep outside bends of the meanders up against the bedrock of the valley walls; develop a compound, morphologically complex channel capable of transporting future water and sediment inputs while maintaining relative stability within the confining boundaries of the reconstructed roadway embankment; and replacing culvert crossings with free spanning bridges capable of passing the 50-year event. Additional goals included enhanced aquatic habitat along the repaired reach, recreational access in selected areas of the National Forest, and construction of features friendly to fish passage. Major themes introduced included the creation of a low-flow channel, a multi-stage seasonal flow channel created by establishing floodplain benches, and increased in-channel habitat complexity through the strategic placement of boulders and large wood. Due to the emergency nature of the roadway reconstruction and budget constraints, the deployment of a rapid assessment, design, and project build timeline became a necessary goal.

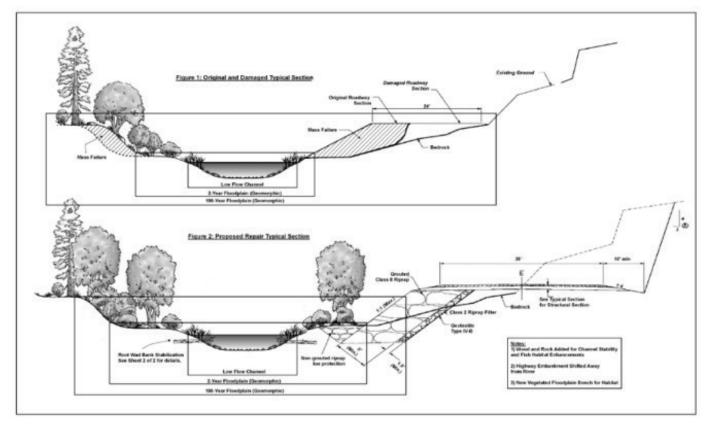
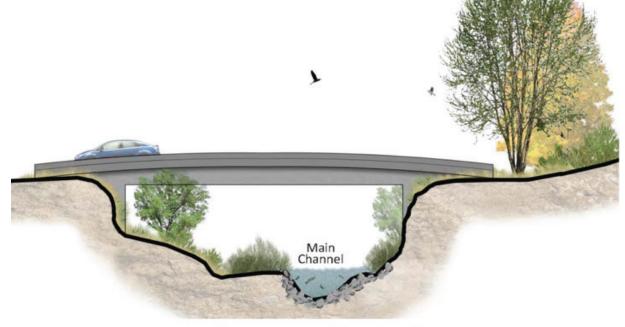


Figure 4. Nested channel concept schematic diagram (Source: CFL and Crane Associates, 2014).

Figure 4 shows a schematic of the nested channel concept with a defined low-flow channel sized for the minimum flows though the driest parts of the year. The 2-year flow channel is intended to overtop in approximately half of all years during spring flows and inundates a low, flat, vegetated floodplain. The cross-section also has capacity to convey the higher, more infrequent flood events.

Pre-flood stream crossing were primarily culverts. This project replaced those culverts with free spanning bridges designed to pass the 50-year flood event. The typical trapezoidal channel associated with most bridge installations were replaced with the same compound channel design used for the river restoration to improve aquatic habitat and morphological integrity through the bridge.



Bridge - Increased Opening Size

4.0 DESIGN BASIS, RECOMMENDED FEATURES, AND FIELD FIT ASSUMPTIONS

This river restoration project was designed and built around restoring river processes. The features that were built are intended to encourage overall system stability and resiliency by providing a framework within which natural processes can dominate. The river is expected to move, adjust, recruit woody vegetation and woody debris, scour, and form new deposits as it adjusts to accommodate fluctuating sediment and water inputs. Evidence of these adjustments and observations of change will deem this project a success, not a failure.

An abbreviated approach to analysis was employed in order to meet the requirement of fall 2015 construction and budget constraints. The budget constraint did not allow for detailed hydraulic modeling and geomorphic and sediment transport analyses, which are typically completed in support of channel restoration projects of this magnitude.

The following rapid approach was undertaken:

• US Geological Survey (USGS) StreamStats were run to determine design flows. These were compared with preliminary reports from the Natural Resources Conservation Service (NRCS) and CWCB/CDOT.

- Approximate channel dimensions were identified for riffle and pool reaches based on 2-year (Q2) and 50year (Q50) events; approximate bench heights were derived for floodplain bench construction from the 2-year flow water surface elevations. The 50-year water surface elevations were calculated by Central Federal Lands (CFL), a Division of the Federal Highway Administration (FHWA), for roadway elevations.
- A preliminary design was proposed based on post-flood channel morphology, as well as knowledge and anecdotal information regarding pre-flood conditions, access, and design goals. The alignment of the roadway and the river were flipped in 11 locations during the preliminary design stage to protect the road in future events.
- Rock, log, and fill quantities were determined by quantifying the number of structures to be built, the ballast required for the installed rootwads, and the floodplain areas to be constructed.
- Preliminary designs were field fit based on professional experience and abbreviated hydraulic modeling done in conjunction with installation.

Several key design features were recommended for use throughout the project area. The main purpose of these features was to enhance stream structure and function by encouraging the natural processes that reinforce river



Figure 5. A constructed low-flow channel, point bar, installed habitat wood, and increased channel sinuosity downstream of bridge 3



Figure 6. Floodplain benches and alternating bars

form and stability. They were sited based on the features of the river in the post-flood and post-road construction condition. The features listed below were incorporated in order to dissipate energy, stabilize the raw and unconsolidated banks, and increase floodplain capacity in anticipation of future high-flow events while integrating aquatic habitat complexity.

Low-flow channels were integrated to ensure connectivity of aquatic habitat during base-flow conditions. The low-flow channel is permitted and expected to migrate laterally within the existing high-flow channel (Figure 5).

Point bars and alternating bars were constructed at most sites to reduce the width-to-depth ratio of the low-flow channel and to encourage natural sediment transport processes though the reach.

These features increase floodplain capacity while adding channel complexity and were constructed mainly using native cobbles, large buried rocks, and large woody debris. Point bars are features on the inside of sweeping bends (Figure 5), and alternating bars are present on both sides of the river, alternating between left bank and right bank, providing low-flow channel sinuosity when the overall river valley is straight.

Floodplain benches were incorporated to maintain a bankfull channel while allowing water during higher flow conditions to access the floodplain and dissipate river energy. These benches exist within the bankfull width of the channel, at an intermediate elevation between the channel bottom and the topographic floodplain or terraces (Figure 6).



Figure 7-Wood debris jam



Figure 8-Boulders for habitat and energy dissipation



Figure 9-Fish friendly irrigation diversion



Figure 10-Riffle/pool sequencing

Large woody debris was utilized in several areas for a variety of purposes, including floodplain roughness, bank stabilization, energy dissipation, and habitat complexity. Root wads were installed facing upstream on the outside of meander bends to dissipate energy and create micro-habitats. Large wood was placed strategically at an upstream, downstream, or perpendicular angle to check the direction and speed of the creek's flow. These features were anchored utilizing large blast rock as ballast.

Boulder clusters and habitat boulders were placed throughout the channel to create habitat for fish and other aquatic species. Secondary benefits include increased channel roughness to slow water velocities and reduce sheer stress on the channel bed and banks (including the roadway embankment). These boulders may also create recreational features.

Grade control and drop structures were built to set the elevation of the streambed to reduce the likelihood of severe scour, as well as to aid in the maintenance of riffle and pool bed features. The pools created by these structures provide cover for fish and other aquatic organisms. Water quality at drop structures is enhanced through aeration, and stream bank erosion is typically reduced as energy is dissipated in the vertical instead of horizontal direction. Low head boulder structures can also be used for fish friendly irrigation diversions (Figure 9).

Riffle-pool sequences were constructed throughout the project reach to provide a heterogeneous physical environment that can be used by a diversity of organisms (Figure 10). They provide a refuge from high velocity waters and extreme temperatures, and promote habitat complexity by offering diverse areas of cover, food, as well as spawning and rearing areas.



Figure 11. Floodplain benching, riffle-pool sequencing, boulder clusters and habitat wood in the beginning of a backwater pool (river left top).

Backwater pools were created to increase aquatic and riparian habitat diversity and provide an area of shallow, slower-moving water for aquatic species, particularly fry and juvenile trout (Figure 11).

Field visits, in conjunction with in-office team design activities, yielded a preliminary design set for the CR 43 project. GIS-based aerial photographs were annotated with text and symbols to identify major areas of work. These design sets were compiled into CAD. Technical drawings of typical features were added to the design set to aid in permitting and construction.

At the outset of construction, the Stream

Recovery Team worked quickly to field fit the design given the available materials and equipment. Equipment operators were trained in the field and tasked on a daily, and sometimes hourly, basis.

4.1 Hydrology

In contrast to the main stem of the Big Thompson River, where the hydrology is heavily regulated by dams and diversions (e.g., irrigation diversions and removals for the Colorado-Big Thompson Project) from the Olympus Dam in Estes Park through its confluence with the South Platte River, flows on the North Fork are completely unregulated and not altered by dams or diversions (BTRRC 2015). Several sources of hydrologic information were used in the preliminary design of channel dimensions for the North Fork stream restoration project.

The USGS StreamStats program was accessed and run to determine design flows, including the base/low-flow, 2-year flow (Q2), 25-year flow (Q25), and 100-year flow (Q100). StreamStats uses several regression equations to estimate the recurrence interval flows, depending on the geography and location of the river. The North Fork of the Big Thompson River has estimates based on both mountain and plains region statistics, and a weighted average was used to determine the combined statistics for the CR 43 stream segments. Results are provided in Table 1 and Table 2 for the upper North Fork near Dunraven (Glen Haven) and the lower North Fork near Streamside (Drake), respectively.

A summary of hydrology information from the most current source to support North Fork Big Thompson River restoration activities, Hydrologic Evaluation of the Big Thompson Watershed: Post September 2013 Flood Event (August 2014), is included here. This report provides a general site description, a catalog of historical flood

events, peak flow estimates for the 2013 flood event, a flood-frequency analysis, a rainfall-runoff model, and HEC-HMS hydrologic model results. The hydrologic model for the North Fork Big Thompson yielded a range of flows for the recurrence intervals identified in Table 1 (CWCB/CDOT at Glen Haven) and Table 2 (CWCB/CDOT at Miller Fork).

The Federal Emergency Management Agency (FEMA) published the effective Larimer County Flood Insurance Study (FIS) in February 2013. The pre-flood information included in the FIS was up-to-date at the time of publication, and no known relevant studies have been conducted between the FIS effective date and the September 2013 flood event. Summaries of FIS peak discharges for the upper and lower portions of the North Fork Big Thompson are provided in Table 1 (FIS at Glen Haven) and Table 2 (FIS at Drake), respectively.

The USGS Streamstats 2-year flow was used to define bankfull stage, and the 5-year flow was used to assess the transition into upland areas. The 50-year FIS was used in the existing conditions model to ensure that proposed improvements do not increase the pre-flood water surface elevation (WSL).

		Str	eamstats at	t Dunrave	n		CWCB/CDOT at Glen Haven	FIS at Glen Haven
							Modeled	Regulatory
		Maximum	/Peak Flow	s (cfs)		Flows (cfs)	Flows (cfs)	(cfs)
	Area- Averaged	Mountain Region	in Plains			Min & Duration	Peak	Peak
Drainage Area (mi^2)	53.7	53.7	53.7	53.7	53.7		51	51
2-year 5-year	300 439	302 437	214 528					
10-year	538	531	822				971	1450
25-year	648	630	1390				1764	2600
50-year	787	760	1900				2640	3400
100-year	894	852	2610				3767	4400
200-year	1020	933	4750					
500-year	1170	1090	4580				7464	11500
7D2Y					207	2.67		
7D10Y					364	1.25		
7D50Y					513	1.64		
D10				95.9				
D25				29.3				
D50				11.9				
D90				4.47				
Sept 2013 Peak (estimated)								

Table 1. Peak Flows from Several Sources, Upper North Fork Big Thompson River

Shaded cells indicate values used in models.

i abie	. Peak Flows from Several Sources, Lower North Fork Big T					nompson kiver			
		Streamsta	ats at Strea	mside			CWCB/CDOT at Miller Fork	FIS at Drake	
	1	Modeled Flows (cfs)	Regulatory (cfs)						
	Area- Averaged	Plains Region	Peak & Duration		Min & Duration	Peak	Peak		
Drainage Area		Region							
(mi^2)	68.5	68.5	68.5	68.5	68.5	68.5	69	83	
2-year	349	353	242						
5-year	514	511	607						
10-year	633	622	949				1108	1500	
25-year	767	737	1610				2083	3000	
50-year	935	890	2200				3184	4100	
100-year	1070	1000	3050				4625	6100	
200-year	1250	1100	5540						
500-year	1420	1280	5350				9460	14100	
7D2Y					243	3.21			
7D10Y					434	1.47			
7D50Y					613	2.02			
D10				114					
D25				35.7					
D50				14.6					
D90				5.67					
Sept 2013									
Peak									
(estimated)								18400	

Table 2. Peak Flows from Several Sources, Lower North Fork Big Thompson River

Shaded cells indicate values used in models.

Chart 1 presents a peak discharge profile for the North Fork Big Thompson River (as published in CDOT 2014). In the upper North Fork Big Thompson near Glen Haven, the predictive peak discharges are approximately 15 to 30 percent lower than the effective FIS discharge results for all design storms. For the 10-year storm, this may be the result of combined rain/snowmelt events. In the lower North Fork Big Thompson, the predictive peak discharges are approximately equal to the effective FIS discharges at Drake for the 10-year, 50-year, and 100-year storms. For the 500-year storm, the predictive model result is approximately 11 percent lower than the effective FIS discharge at Drake (CDOT 2014).

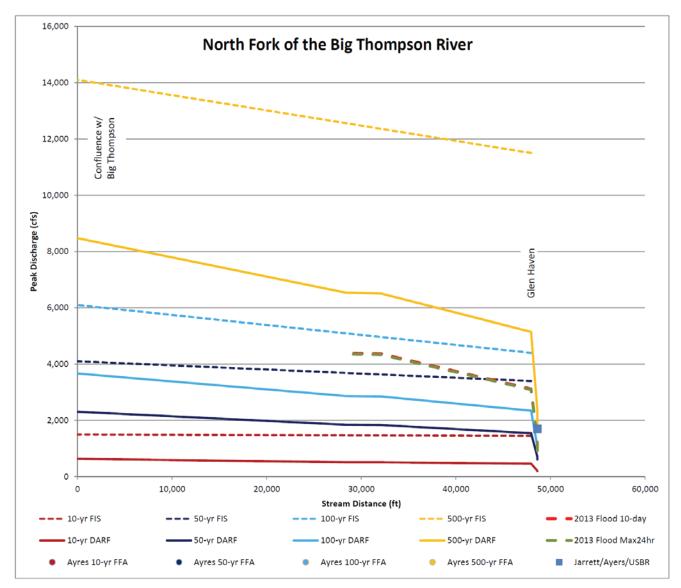


Chart 1. Peak Discharge Profile, North Fork Big Thompson River

In 2015, spring runoff peaked during the second week of June, with flows at about 285 cfs, approximating a 1.5to 2-year recurrence interval (Chart 2). This data was measured using a velocity meter at a stable cross section upstream of the Dunraven bridge.

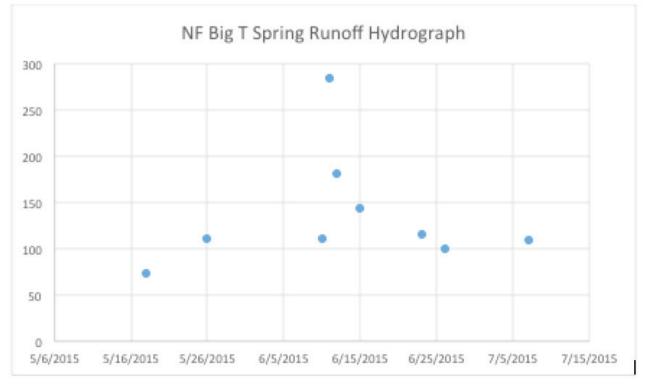


Chart 2. Spring 2015 Hydrograph, North Fork Big Thompson River

The average channel width through this reach is 25 feet, and the channel slope averages 132 feet per mile. Two of the larger tributaries to the North Fork Big Thompson River, West Creek and Devil's Gulch, join near Glen Haven. West Creek and Devil's Gulch have average channel widths of 25 feet, and their slopes are 90 feet per mile and 400 feet per mile, respectively (CDOT 2014).

4.2 Hydraulics

Open channel hydraulic calculations were completed using the CDOT hydrology data to develop a basic lowflow and bankfull channel cross-sections. Channel dimensions were based on pre-flood conditions including available cross-sections, photographs, and landowner reports. These were confirmed with simple Manning's equations and HEC-RAS modeling. Chart 3 provides typical riffle and pool cross-sections for the North Fork Big Thompson River. Ultimately, these dimensions were field fit, recognizing that a natural channel has a high degree of variability and that future runoff events will both erode and deposit sediments as the streams regain and establish relative stability.

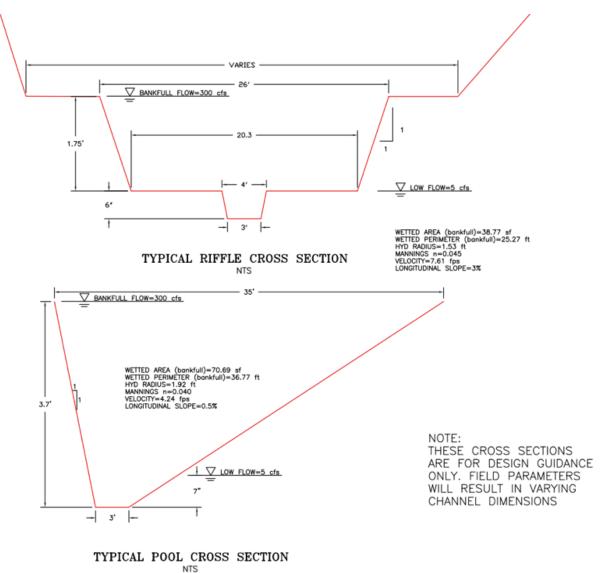


Chart 3. Typical Design Cross Sections

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A HEC-RAS hydraulic model was developed by CFL engineers for both the existing and proposed conditions. Road and bridge designs were developed to contain the 50-year event. The objective of the Stream Team was to maximize floodplain capacity while developing a meandering compound low flow channel within the floodplain.

Follow-up analysis was completed during construction to determine the locations where floodplain benches—to be set at the approximate 2-year flow elevations—would not impact 50-year water surface elevations. Basic HEC-RAS modeling showed that adequate river corridor width to include these features without adverse impacts on the 50-year flood surface elevations was absent at many sites. Lower, mobile, alternating bars and point bars were built at all the sites.

Restoration Project Review

This section presents a site-by-site description of work completed for the CR 43 river restoration project. A map of the project area and key site locations is provided as Figure 12. The project was divided into 2 parts; West Seed and East Seed. The West Seed project started downstream of the Town of Glen Haven at the boundary between private property and the US Forest Service and extended downstream to just below the Dunraven bridge. Most of the West Seed project was in the Roosevelt National Forest. The Stream Team worked closely with Dan Cenderelli and Matt Fairchild of the US Forest Service. The Forest Service Team provided post-flood cross sections and longitudinal profiles that were valuable and served as the existing conditions data needed for the proposed design.

The East Seed project was a patchwork of stream rehabilitation sites on both private and public lands. The restoration effort funded by the FHWA project could only be implemented where the roadway was adjacent to the stream. The East Seed project extended from the Section 25/Section 26 line to the Chenoweth Ranch. The red line in Figure 12 does not include any stream restoration efforts performed under the CR 43 project but does include a separate project funded by the Colorado Water Conservation Board.



Figure 12. US 36 project area and site location map.

4.3 WEST SEED

The West Seed reach is almost exclusively on US Forest Service land and as a result the Forest Service Team was very influential and helpful in the design of the project. They worked closely with CFL staff to maximize floodplain potential. The West Seed project was broken up into reaches based upon stream crossings. There were 6 new bridges constructed in this reach beginning with bridge 3 and continuing downstream to bridge 8. The Dunraven bridge is also included in this reach but is on private property. The total length of the West Seed reach is approximately 1.8 miles. West Seed plans are found in Appendix A. Due to budget constraints river as-builts were never completed.

4.3.1 Bridge 3

The bridge 3 reach begins approximately 700 feet upstream of the bridge and extends approximately 600 feet downstream to a bedrock outcropping that formed a steep cascade. The pre-flood alignment had the river on the inside bend of the valley and the roadway on the outside bend. The flood destroyed the road in this location. Bridge 3 was constructed to move the shear stress from high water events to the bedrock walls along the outside of the valley. A significant amount of blasting was required to move the road inward and the river was completely rebuilt in the location where the road was prior to the flood. Minor horizontal and vertical alignment adjustments were made in the field due to bedrock outcrops. A significant amount of excavation was removed to develop bankfull benches and



Figure 13-Pre-flood alignment

terraces. Additional rock and large woody debris was installed to enhance the aquatic habitat.



Figure 14-Post construction upstream of Bridge 3 on left and downstream on right

4.3.2 Bridge 4

Bridge 4 was also constructed to flip the alignment of the road and the river to increase resiliency in future flood events. It is located approximately 1,220 feet downstream of Bridge 3. Additional field fits were required throughout this reach to account for the varying elevation of bedrock along the south valley wall. Dozens of exploratory holes were drilled upstream of the bridge to determine bedrock elevations and design an appropriate channel approach to the bridge. At the upstream end of this reach channel excavation uncovered a steep cascade that acted as a fish passage impediment. The Stream Team designed and constructed a series of step-pools to improve fish passage in this location. Additional boulders and large woody debris was incorporated for enhanced aquatic habitat. Floodplain benches were maximized throughout this reach by steepening the side slope of the road and stabilizing the toe of the slope with large boulders.



Figure 15-constructed step-pools on left and root wad revetment with habitat wood on right upstream of bridge



Figure 16-Riffle-pool sequencing and expanded floodplain bench above bridge on left and downstream of bridge on right

4.3.3 Bridge 5

The Bridge 5 reach, running halfway between Bridge 4 & 5 to halfway between Bridge 5 & 6, included a significantly expanded floodplain, low-flow meandering with a riffle-pool complex, large woody debris anchored on both the floodplain and in the channel, and a constructed log jam downstream of the bridge to enhance aquatic habitat and trap floating debris. The stream was constructed along the alignment of the pre-flood road and the new road was relocated towards the inside bend. Again, a significant amount of blasting was required to move the road inward. Downstream of the bridge and the log jam the channel was pinched straight due to valley constraints. The Stream Team constructed a low flow meandering channel with a riffle pool longitudinal sequence and utilized large woody debris and boulders and enhance channel complexity.



Figure 17-Expanded floodplain capacity upstream and downstream of Bridge 5



Figure 18-Constructed log jam and enhanced aquatic habitat downstream of Bridge 5



Figure 19-Long confined reach downstream of Bridge 5 before and after construction

4.3.4 Bridge 6

Bridge 6 was also constructed to move the road away from the outside valley walls to increase resiliency. Upstream of the bridge was the site of a previous Forest Service campground. An old chimney still stands at this site. A small tributary enters the creek on river right just above the chimney site. A significant amount of excavation occurred upstream of the bridge to increase floodplain capacity. The channel takes a hard left turn just above the bridge and utilizes the bedrock wall there to direct the flow. A series of deep pools were constructed along this wall for fish holding. A similar scenario exists downstream of the bridge when the river turns hard right again. Large woody debris was utilized extensively through this reach and another log jam was constructed upstream of the bridge.



Figure 20-Pool habitat upstream of tributary & root wad revetments downstream



Figure 21-Expanded floodplain benches up and downstream of Bridge 6

4.3.5 Bridge 7

Both upstream and downstream of Bridge 7 there has been significant floodplain excavation with large amounts of woody material anchored both on the benches and in the channel. This is another location where the road and the river alignments have been flipped to increase resiliency.



Figure 22-Expanded floodplains, boulder clusters, constructed large woody debris up and downstream of Bridge 7

4.3.6 Bridge 8

Bridge 8 is the last crossing on Forest Service land before crossing over onto private property. This reach extends down to the horseshoe bend above the Dunraven bridge. Numerous anchored root wads were installed at the top of the bend. The roadway was moved back into the hillside here and an overflow swale was constructed to allow high flow events to cut off the horseshoe bend, as it did in the flood, and safely divert back to the stream downstream. This will help protect private residences in the area and act somewhat as a pressure relief valve. Revetment has been buried in the top of the bank to allow water to recede back into its primary channel instead of creating a new one.



Figure 23-Root wad revetment at horseshoe bend & channel downstream of Bridge 8

4.3.7 Dunraven Bridge

Dunraven bridge is a Larimer County bridge that survived the flood. The County repaired scouring along the abutment walls and the compound channel constructed both above and below the bridge continued through it. This reach of the West Seed project was completed first in the Fall of 2015 and allowed Wildland Restoration Volunteers (WRV) to come in and do riparian plantings and seeding. This was all paid for by private grants obtained by WRV. Treatments in this reach included root wad revetment, boulder clusters, riffle-pool sequencing, floodplain benching and expanded capacity.



Figure 24-Riparian regeneration after one season of growth near the Dunraven Bridge

4.4 EAST SEED

Time and budget constraints put a considerable amount of pressure on the East Seed reach. Design plans were never completed due to the constraints yet there was a substantial amount of restoration work to complete. Hydrology calculations, channel dimensions and treatment details were all complete from the West Seed reach and were used appropriately downstream. The Stream Team was given an excavator and an operator and told to complete the project as quickly as possible. The experience from the West Seed reach was invaluable in developing appropriate natural channel morphology. Unlike the West Seed project, the East Seed reach was broken up by landowners instead of bridges. The existing conditions plans for the East Seed reach can be found in Appendix B.

4.4.1 Ballard site

The Ballard site begins just past the Crosier mountain trailhead, downstream of the Dunraven bridge, to what was known as Bridge 11. The river was pinched between the roadway and a private driveway that serviced several properties. Considerable excavation was required to create a floodplain and develop a meandering channel through approximately 900 feet of stream. Bridge 11 was built to keep the road on the inside bend of the valley and a culvert was eliminated upstream of that to maintain resiliency and sustainable channel morphology. A small amount of wood was installed but primarily strategically placed boulders were used to create channel complexity and improve aquatic habitat.



Figure 25-Meandering riffle-pool complex using primarily boulders at Ballard site

4.4.2 Bridge 11 to Wilder

The site downstream of Bridge 11 was used by the contractor for staging. The channel in that area had been ditched and straightened. The downstream landowner, Wilder, had requested that it be put back into the pre-flood alignment that took it further away from his residence. Treatments at this reach included Boulder and root wad revetments, channel realignment and riffle-pool sequencing throughout the 800-foot reach.



Figure 26-Channel meandering, root wad revetment and boulder clusters downstream of Bridge 11

4.4.3 Whiteside

The stream leaves the roadway for a short stretch and re-emerges alongside the road at the Whiteside property across from Streamside Drive. The Whiteside property owns approximately 1,800 feet of stream but only spot treatments were requested at this reach. Minor channel alignments were constructed through this reach and several boulders were placed to enhance aquatic habitat.



Figure 27-Floodplain grading and boulder placement at Whiteside

4.4.4 US Forest Service

From Whiteside down to the Forest Service property there is an approximate 1 mile gap where no treatments were done. Some of that didn't need anything and in other locations there was no access. At the Forest Service property the floodplain opened up and a significant amount of excavation was removed from there. A meandering channel with riffle-pool sequencing was constructed, deep pools were built alongside the bedrock walls, a log jam was constructed and large woody material and boulders were installed for aquatic habitat along this 1,800 foot reach.



Figure 28-Strategic rock placement for riffle-pool morphology & log jam on expanded floodplain



Figure 29-Deep constructed pools alongside bedrock with log enhancement & expanded floodplain

4.4.5 Shukle-Shea

Two bridges were eliminated at this site, approximately 1,700 feet downstream of the US Forest Service property, by negotiating with landowners to flip the alignment of the road and the river and putting the river along the outside bend of the valley. This reach was approximately 2,100 feet long and included 1,200 feet of channel realignment. Treatments included floodplain grading, low flow meandering channel with riffle-pool sequencing, large woody material installation and boulder placements.



Figure 30-Expanded floodplain benches, root wad installation, meandering channel, riffle-pool sequencing

4.4.6 Rainbow Bridge

A new bridge at the Rainbow Bridge site replaced a previous box culvert that had plugged and washed out. This was the last site of the East Seed reach. The 600-foot reach runs from the bridge to a bedrock wall on river left where it turns south and is transitioned into another river restoration project funded by the Colorado Water Conservation Board on the Chenoweth Ranch. The site was used for staging by the contractor and a significant amount of excavation was removed to develop floodplain benches. Additional boulders and large woody material was installed for enhanced aquatic habitat.

5.0 ESTIMATED MATERIALS AND CONSTRUCTION COSTS

The ability to utilize materials from on site and equipment from the road construction project is crucial to developing a cost-effective and holistic project. Budget data from the CR 43 project was not available for analysis so only generalizations can be made but cost savings have been shown to be substantial in previous projects.

For example, two cost estimates were generated for the river restoration work throughout the US 36 corridor in 2014. The first was an approximation of actual expenditures of the project, including the construction, construction oversight, design, coordination, and permitting. The second was an engineer's opinion of probable cost for a stand-alone river restoration project that would have produced the same on-the-ground product. That estimate was meant to illustrate what the river restoration cost to an outside agency, a watershed coalition, CDOT or private landowners would have been had there been no collaboration with the permanent road rebuild.

5.1 ACTUAL COSTS

The total executed cost of the US 36 river restoration project was approximately \$191,750. The project restored 8,375 linear feet (LF) of river on North St. Vrain Creek and 2,380 linear feet on the Little Thompson River, for a total of 10,755 linear feet stabilized and enhanced. The average cost of this complete restoration was \$18 per linear foot. This cost, however, excludes revegetation activities, as those did not occur as a part of this project.

The estimated actual cost of the conceptual design, coordination, permitting, survey, and construction oversight for river restoration work on both North St. Vrain Creek and the Little Thompson River was approximately \$90,000. This number is weighed heavily by the initial coordination time in establishing the agency and private landowner collaboration. The construction oversight services for the work is likely a bit higher than would be expected on a "standard" river restoration project. The compressed timeline of the design process meant that construction began when only a conceptual plan for the work was completed; all construction actions necessitated engineering oversight and extensive field fitting was required throughout the entire project reach.

The estimated actual construction cost was approximately \$101,750. 250 hours of equipment operation were used in Phase 1 of the construction process, completing stabilization and channel shaping work before spring runoff in April and May 2014. 300 hours of equipment time were used during Phase 2 of the project during August and September 2014.

5.2 PROBABLE COSTS FOR IDENTICAL WORK

The costs in this section are an engineer's opinion of probable costs for identical work, but completed as a standalone project and executed by a local government agency, watershed coalition, or private landowner(s).

These estimates include design, permitting, landowner coordination, construction mobilization, construction costs, administration, and survey work, as well as similar erosion control, health and safety, and traffic management plans. Because no revegetation was completed, no revegetation costs were included in this estimate.

The total estimated cost for the North St. Vrain Creek river restoration project is \$894,141 (Table 3), and the total estimated cost for the Little Thompson river restoration project is \$403,844 (Table 4), for a total of \$1,297,985.

The estimated total North St. Vrain Creek and Little Thompson River construction costs would have been about \$107/LF.

North Saint Vrain Creek						
Sites 3-11						
Item	Quantity	Units	U	nit Price		Cost*
Pre-Construction Requirements						
Bond, Insurance, Permits, Traffic Control, Mobilization, etc.	1	EA	\$	110,988	\$	110,988
Site Preparation						
SurveyLayout & Boundary	5	DAYS	\$	2,400	S	12,000
Temporary Erosion and Sediment Controls	1	EA	\$	30,092	\$	30,092
Earthwork/Grading						
Excavation & RegradeFloodplain Regrading, Channel Realignment, Bank Work	24,081	CY	\$	14.40	\$	346,768
Instream and Bank Stabilization Structures			-			
Boulders for Grade Control Structuresdelivery & installation	359	TON	\$	120	\$	43,080
Habitat Boulderdelivery & installation	192	TON	\$	70	\$	13,440
Riffle Roughening	135	TON	\$	70	\$	9,450
Habitat Log on Floodplaindelivery & installation	16	EA	\$	330	\$	5,280
Engineered Rootwaddelivery & installation	42	EA	\$	1,200	\$	50,400
Engineered log vanesdelivery & installation	4	EA	\$	1,200	\$	4,800
Trash and Debris Removal						
Trash and Debris Removal	4.4	DAYS	S	1,000	\$	4,400
Closure Actions						
Demobilization and Reclaim	1	EA	\$	9,960	\$	9,960
Final Survey	5	DAYS	\$	2,400	\$	12,000
Construction Subtotal					\$	652,658
Support Services						
Construction Design & Engineering	12	х	\$	652,658	s	78,319
Construction Support Services	10	х	\$	652,658	S	65,266
Administration	5	х	\$	652,658	S	32,633
Stakeholder Outreach & Education	10	х	\$	652,658	S	65,266
Design and Support Services Subtotal					\$	241,483
Project Total					S	894,141

Table 3. Probable Cost for Identical Work, North St. Vrain Creek

Little Thompson River						
Sites 13-16						
Item	Quantity	Units	U	nit Price	1	Cost*
Pre-Construction Requirements						
Bond, Insurance, Permits, Traffic Control, Mobilization, etc.	1	EA	\$	102,888	\$	102,888
Site Preparation						
SurveyLayout & Boundary	3	DAYS	\$	2,400	\$	7,200
Temporary Erosion and Sediment Controls	1	EA	\$	23,432	\$	23,432
Earthwork/Grading						
Excavation & RegradeFloodplain regrading, Channel Realignment, Bank Work	3,667	CY	S	14.40	S	52,805
Instream and Bank Stabilization Structures						
Boulders for Grade Control Structuresdelivery & installation	174	TON	S	120	S	20,880
Habitat Boulderdelivery & installation	49	TON	\$	70	\$	3,430
Plugmaterial delivery & installation	89	CY	\$	14	S	1,281.60
Habitat Log on Floodplaindelivery & installation	20	EA	S	330	\$	6,600
Culvert Installation	6	EA	S	1,200	\$	7,200
Engineered Rootwaddelivery & installation	42	EA	\$	1,200	s	50,400
Trash and Debris Removal						
Trash and Debris Removal	1.5	DAYS	S	1,000	\$	1,500
Closure Actions						
Demobilization and Reclaim	1	EA	S	9,960	\$	9,960
Final Survey	3	DAYS	\$	2,400	\$	7,200
Construction Subtotal					\$	294,776
Support Services			-			
Construction Design & Engineering	12	x	\$	294,776	s	35,373
Construction Support Services	10	x	\$	294,776	S	29,478
Administration	5	х	\$	294,776	S	14,739
Stakeholder Outreach & Education	10	х	\$	294,776	S	29,478
Design and Support Services Subtotal					\$	109,067
Project Total					S	403.844

Table 4. Probable Cost for Identical Work, Little Thompson River

Estimated costs are \$42/LF on North St. Vrain Creek and \$22/LF on the Little Thompson River for the earthwork involved in creating low-flow, bankfull, and high-flow channels. North St. Vrain Creek should have a marginally higher per linear foot cost because there is more width and depth that require working for each segment of stream.

The Little Thompson and Saint Vrain master plans have expected costs for this type of work in these reaches and estimate that on the Little Thompson it would have cost \$48/LF for low flow channel and floodplain capacity earthwork, which is quite similar to the costs presented here. The Saint Vrain master plan, however, has \$250/LF for the same task, which seems unreasonably high for these canyon reaches, but may be reasonable for the plains portions of the river east of Longmont.

5.3 COST COMPARISON

The collaboration between the state, federal, and local agencies for this project resulted in an 85 percent cost savings for the river restoration work on North St. Vrain Creek and the Little Thompson River. The actual cost of the project was \$191,750, while the estimated cost of stand-alone projects on both rivers is \$1,297,985, resulting in a cost savings of more than \$1.1 million.

As a rough comparison, the US 36 project restored 10,750 linear feet of channel while the CR 43 project restored 17,500 linear feet of channel – a 61% increase. The cost savings between the US 36 project and the CR 43 project are likely not linear but if they were the savings would be approximately \$1.77 million.

6.0 PROJECT CONSTRAINTS

The CR 43 project site was constrained by several factors, listed below, which affected the recommended design.

Roadway Embankment Constraints: The maximum extent of the toe of the slope of the CR 43 embankment was determined by the emergency repair work and the subsequent roadway work while keeping the road open for local residential traffic. This required the river to be channelized in a very narrow corridor with immense amounts of fill in the floodplain. The grade of temporary culvert crossings (shoo fly) sometimes were set too high which contributed to the fill by depositing sediment at the inlet. This resulted in large quantities of material to be removed and transported off site before restoration work could be done. The tight valley physical constraints of this particular project limited alternatives for stockpiling fill material but on other projects if river restoration experts are consulted earlier in the design process other alternatives may be developed that might reduce the amount of material to be moved.

Budget Constraints: Despite the fact that there was no budget for stream restoration, the FHWA project manager took advantage of necessary right-of-way work in and adjacent to the stream to implement river restoration treatments. Even a small budget for stream restoration design and implementation work could be leveraged by the roadwork budget to construct a more holistic project.

Timing Constraints: The project had a very rapid timeline between roadway subgrade construction and paving. Most of the stream restoration needed to completed in between those two phases. If equipment was dedicated to the river earlier that constraint would have been reduced.

Agency Competition Constraints: Differing priorities by competing federal agencies made timelines somewhat difficult. What one agency thought was important another did not and, therefore, time went into plans that could not be built in the field.

Material Availability, Delivery, and Equipment Constraints: The construction of CR 43 was the priority for the County, CFL and construction crews. Construction materials were made available to the Stream Recovery Team intermittently depending on blasting schedules and availability of equipment to load and haul. On occasion, some sites were left under-constructed due to a lack of available materials and/or equipment. In-channel equipment and/or their operators were occasionally made unavailable to attend to priority needs of the roadway construction. Mechanical issues shut down equipment for repairs frequently during construction and the lost hours contributed to the timing constraint.

Revegetation Constraints: In most cases, streamside vegetation was preserved only to the extent that it did not interfere with construction of the roadway.

7.0 LESSONS LEARNED AND OPPORTUNITIES FOR FUTURE SUCCESSES

The CR 43 river restoration project was a success, due in large part to the support and encouragement of CFL leadership, the County and survey data provided by the Forest Service team, and despite the accelerated time frame, restricted equipment and material availability. As the project served as an excellent project for testing process and partnerships, lessons learned throughout the process will be beneficial to future flood recovery efforts and to other restoration projects that involve a shared roadway/river corridor. This section details recommendations developed as a result of this project.

1) Design the Road and River as a Single System

In areas involving a shared roadway/river corridor, river planning and road planning should be approached and evaluated as a singular system, as opposed to being treated as two disparate planning projects. A sustainable road/river relationship goes beyond predicting water surface elevations and sizing rip-rap for roadway embankments. It starts with evaluating the locations of both the road and river to minimize the length of embankments that are subject to high shear stresses and sizes crossing structures that take into account sediment loads as well as flow capacity. The singular system design can also make great strides in reducing erosive energy in locations where the roadway is vulnerable, as well as providing the grading and seeding to encourage vegetation that will serve ecologic functions as well as provide cohesive structure to the roadway embankments.

2) Begin Collaboration in the Planning Stages of the Project

Clear, early, and frequent communication between river restoration and roadway reconstruction specialists is paramount. This project began with a tour of the road and river alignments and resulted in the river and road being flipped in 11 locations to improve resiliency in future flood events. Additional consultation during the roadway design may have resulted in cost saving with respect to material movement and construction techniques. For example, there were several instances where the river restoration team needed to make channel and floodplain improvements due to previous construction. Had a river restoration specialist been on site with the authority to direct operators, some work would not have had to been repeated.

3) Encourage Private Landowner Cooperation

River processes do not start or stop on political boundaries and easement lines. Often, the most effective way to change or eliminate a vulnerability in a river corridor is to do work upstream, downstream, or on the opposite bank. Coordination with private landowners is critical to implementing a comprehensive and holistic project without the typical patchwork that frequently results from individual landowners working on their own. The CR 43 project's cooperation with private landowners to conduct system-wide river improvements on both banks and upstream and downstream of property boundaries was absolutely necessary for this type of project and was executed extremely well by Larimer County staff and the CFL project manager. Nearly all of the landowners were cooperative and allowed the restoration to take place almost seamlessly across ownership boundaries.

4) Engage in a Concurrent Design and Construction Process

This project underscored the cost-effectiveness of a collaborative design and construction project (rather than stand-alone separate projects). As stated in the previous section, executing the construction of the stream restoration work as a portion of the roadway rebuild, rather than as a separate contract, likely saved the agencies millions of dollars. The project was able to capitalize on and utilize material generated elsewhere within the project corridor, drastically reducing construction costs. In addition, working within the structure of the concurrent road project meant that mobilization and other project start-up and shut-down costs were minimized.

5) Enlist Equipment Operators with River Construction Experience

While the Stream Recovery Team worked closely and well with the several equipment operators provided by the contractor, equipment operators who have previous experience working in rivers and particularly on river

restoration projects could benefit future projects. Efficiencies could be attained if equipment operators are versed in the language of river restoration design, necessitating fewer construction oversight hours by the design engineers. Furthermore, operators who have a background in environmental restoration are more likely to construct river-appropriate structures and those with the experience of building within the channel corridor are likely to have better focus on completing the critical design components (Figure 32).

6) Preserve Fallen Trees and Large Wood

CFL, Forest Service and contractor staff did a remarkable job stockpiling large fallen trees



Figure 32. Experienced equipment operators reduce the time needed to build complicated features.

for use in the river restoration. Most of those trees came from Forest Service lands and were required to be used on the Forest. However, if the US Forest Service could be allowed to use its tress throughout the watershed we may have had an even more holistic restoration.

Fallen trees with intact root balls, commonly referred to as large woody debris (LWD) or large wood, are a key component of stream stability and stream ecosystems. Decades of research has shown the removal of large wood, and debris jams created during flood events, have a destabilizing effect as the channel loses a significant amount of its ability to dissipate energy though non-destructive means. For a roadway embankment, the presence and use of large wood may actually deflect and dissipate scouring flows providing an additional buffer to the roadway. Furthermore, the complex flow structures, cover, and nutrient loading that in-channel large wood provides have profound benefits to the entire riparian ecosystem, from macroinvertebrates to adult fishes to birds of prey and terrestrial mammals. Wherever and whenever possible, large wood should be stockpiled or left onsite for reuse in channel projects. It should not be burned or removed from the watershed. Chipping some wood debris into mulch for erosion control may be a worthwhile pursuit.

7) Finish the Project with Intensive Revegetation

Revegetation should be conducted or planned in conjunction with channel work (Figure 33). Native vegetation can have specific topographic and final grade requirements that are necessary for the plants to root and thrive. These considerations are best addressed in the design phase of the project. Immediate coordination with native vegetation experts such as the US Forest Service, CWCB-supported Native Riparian Plant Propagation Program and/or volunteer groups such as Wildlands Restoration Volunteers and the Conservation Corps to improve native revegetation and reduce the need for post-construction weed control is recommended. Additionally, cooperation with



Figure 33. Floods are regenerative and vegetation will slowly establish; however, jump-starting the process with native plantings and seeding will help suppress weeds and invasive species while encouraging soil and sediment cohesiveness immediately after construction.

these organizations could lead to cost-sharing or other creative financing agreements for revegetation efforts.

8) Stockpile and Redistribute Topsoil

In conjunction with revegetation, the preservation and perhaps even import of topsoil should be considered at the outset of a restoration project. Topsoil is critical to vegetation establishment, soil and sediment cohesion, and a self-sustaining restoration project. Especially on projects such as this one, where significant excavation into areas with layers of topsoil occurred, this limited resource should be stockpiled and graded into floodplain benches and bare channel banks to the extent possible throughout the project area.

9) Prioritize Trash Removal

Trash removal should be considered a priority from the beginning of a flood recovery project. During the CR 43 project, a significant amount of trash that was not transferred off-site early ended up in danger of re-mobilizing during spring runoff and had to be removed immediately.

10) Create Opportunities for Public Access

The CR 43 project had, and took advantage of, the capacity to extend safe access to stream bank areas within the road right-of-way to the general public for fishing and other recreational activities on US Forest Service land. Exploration of the opportunity to partner with Colorado Parks and Wildlife for this additional public benefit is recommended.

11) Improve Roadway Embankments to Benefit the River

The existing embankments built by CFL to protect the road were enhanced in many cases to dissipate river energy and slow water velocities. Adding complexity to the stream bank can be accomplished by placing boulders in a more random pattern at the toe of the roadway embankment to slow flows along the outside bend of the stream, benefiting the river channel without compromising roadway protection. Naturalizing the embankments with the addition of vegetation, such as willows, would also provide benefits to the stream ecosystem.

12) Highlight the Benefits to the Public and Public Perception

A significant benefit to linking road and river work is the reduced inconvenience to nearby residents and drivers as they will experience only one set of traffic delays and road closures. Similarly, nearby residents are only subjected to the annoyances (dust, noise, etc) of a single construction timeline which may reduce the despair and "aren't they ever done?!" sentiment that would certainly arise as multiple projects are done in a single area. Additionally, designing and constructing the project as one indicates to the public that the government agencies are acting collaboratively and with the environment in mind and that there is a drive to effectively complete the entire project and quickly move to the next area in need.

13) Coordinated Permitting and Public Outreach

Preparing permits for both the road projects and stream restoration work simultaneously (and when possible combined) resulted in significant cost and time savings. Treating the project and its impacts, environmental reviews, and strategies for mitigation as one project reduced the burden on both parties for both application and review.

8.0 NEXT STEPS

In order to maintain and capitalize on improvements and roadway stability gained through river restoration activities and to plan for possible future enhancements, long-term monitoring and revegetation of the CR 43 sites are an important next step. Each of these components is described in more detail below.

8.1.1 Long-term River Monitoring

A formal monitoring plan with the goal of generating sound scientific data over a specified period of time should be developed in conjunction with the Forest Service and CFL. The monitoring plan should be constructed with the goals of the project in mind, the data needs of the decision makers, and a means to turn data into information.

Specific methods, sites, timing, and quality assurance procedures could be outlined in the monitoring plan. Quantity and location of sample sites could be determined based on assessed project goals. While professional judgment on the selection of sample site(s) could be supplemented with advice from project stakeholders and guidance from sampling protocols, an overarching goal could be the selection of sites with enough diversity, spacing, and number to obtain a dataset that adequately represents the overall CR 43 flood recovery effort. Parameters and methods could be selected to measure change specific to the project. Sampling timing and frequency, collection of historic/pre-project data, utilization of reference sites, and/or expected condition could also influence selection of monitoring parameters. Example monitoring parameters are listed in Table 5.

Project Type	Core Parameters	Additional Parameters
Geomorphic enhancements including compound channels, rock structures, large woody debris, and bar/riffle development aimed at maintaining stability	Cross-sections, photo points	Macroinvertebrates and fish, temperature, sediment, thalweg, structures surveys, topographic/ profile survey, hydraulic analysis (HEC-RAS)
Riparian enhancement	Temperature, canopy cover, discharge, nutrients, vegetation survey, macroinvertebrates	Thalweg, cross-sections, photo points, fish

Tuble 51 CR 15 Example Montoling I drameters	Table 5.	CR 43 Example	e Monitoring	Parameters
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8.1.2 Revegetation

In partnership with the adjacent landowners throughout the CR 43 corridor, an aggressive weed management and revegetation plan should be developed and implemented for the bare earth areas throughout the project site. The denuded landscape causes problems from a structural and safety perspective within the river corridor, and also degrades riparian habitat—a critical ecological habitat for a majority of Colorado's wildlife.

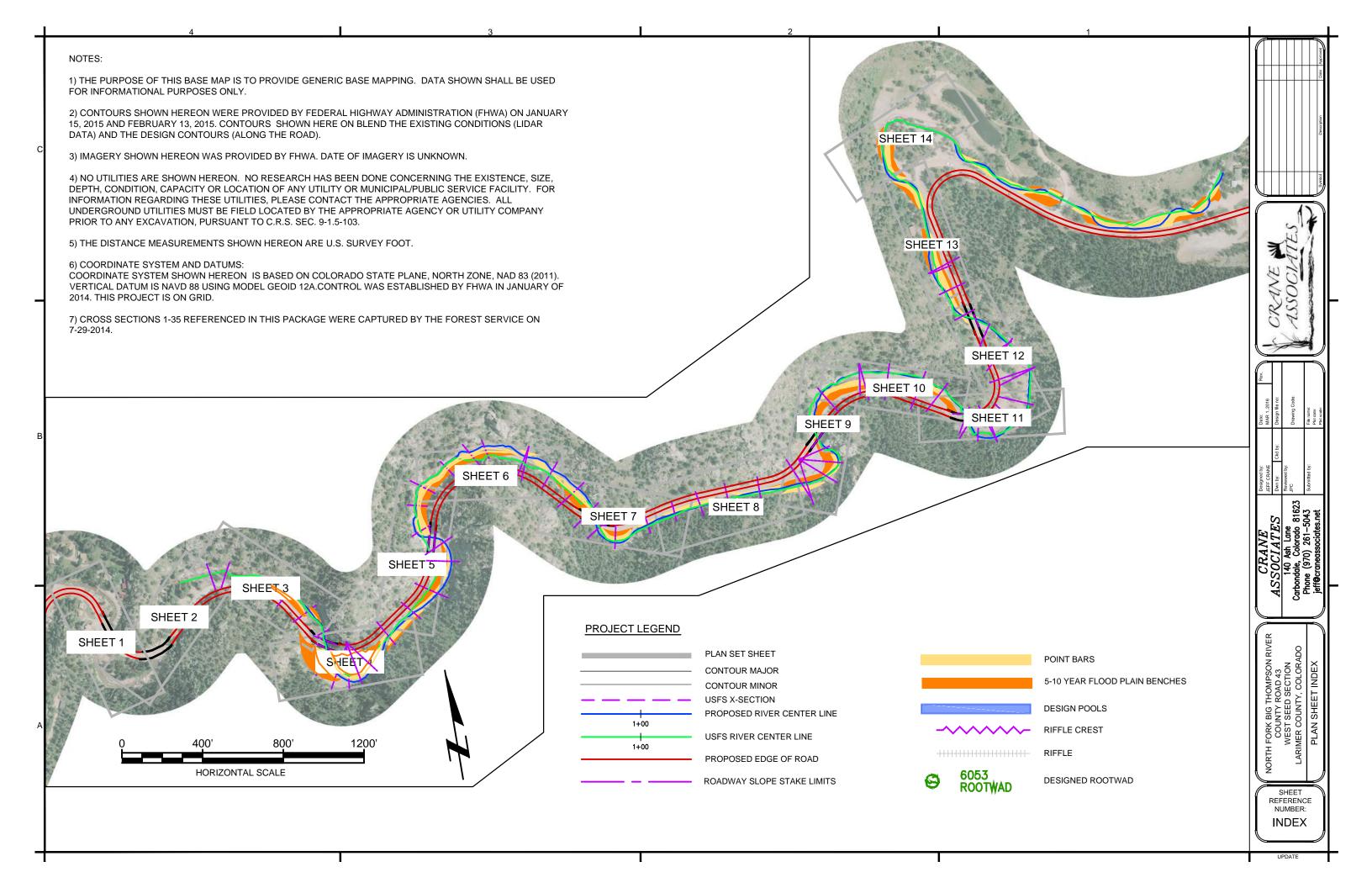
Senate Bill 14-179 provided the CWCB with funding to address this habitat loss throughout the flood-affected corridors. Wildland Restoration Volunteers, one of many grant recipients, is working with private consultants to develop locally-adapted native shrubs, trees, and grasses necessary to restore flood-impacted areas. Working with the newly-formed Southern Rockies Seed Network, a long-term impact of this program will be to build a network of collectors, growers, and distributors that provide native shrubs and trees originating from the Southern Rockies Ecoregion.

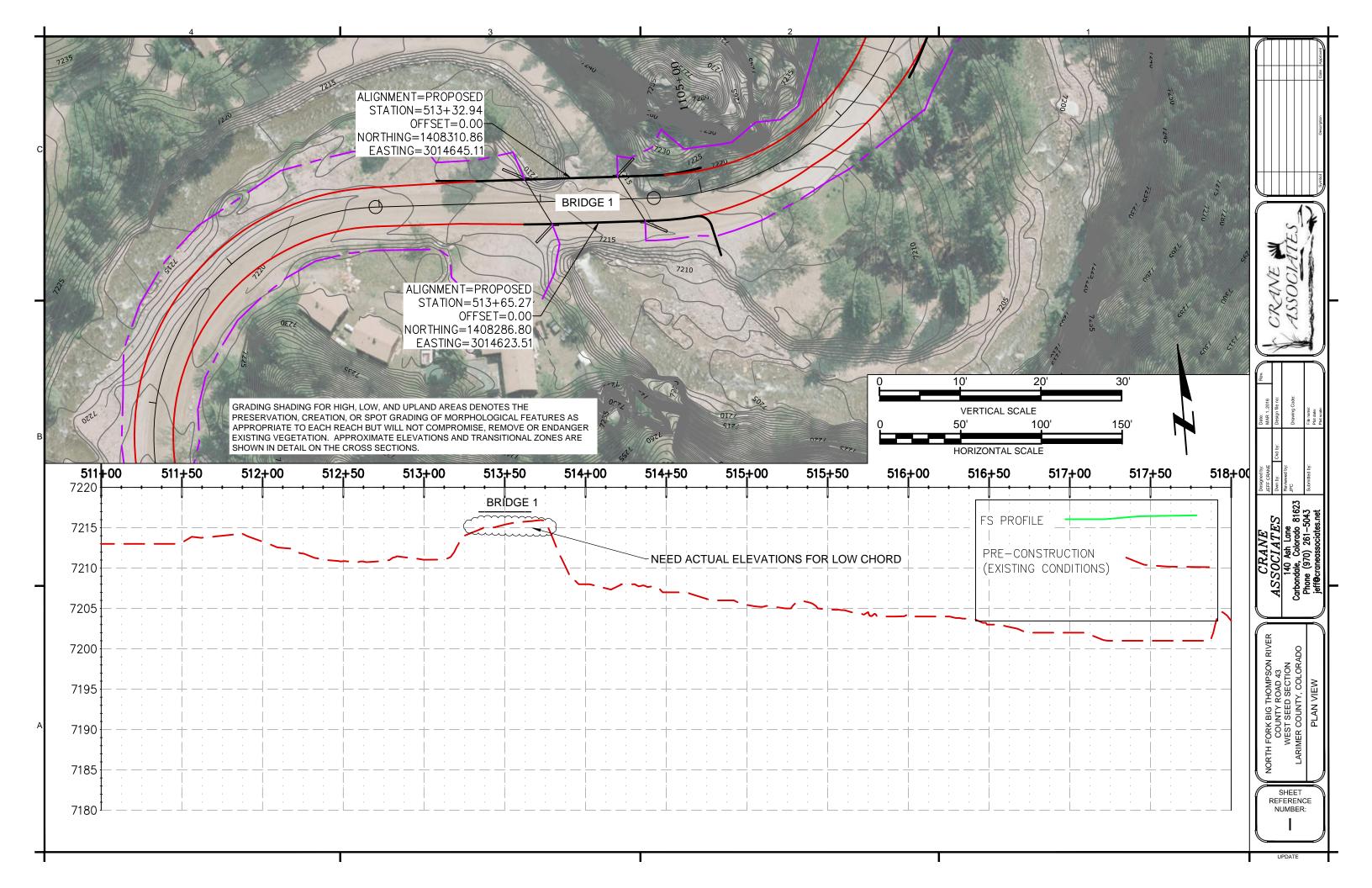
9.0 CONCLUSIONS

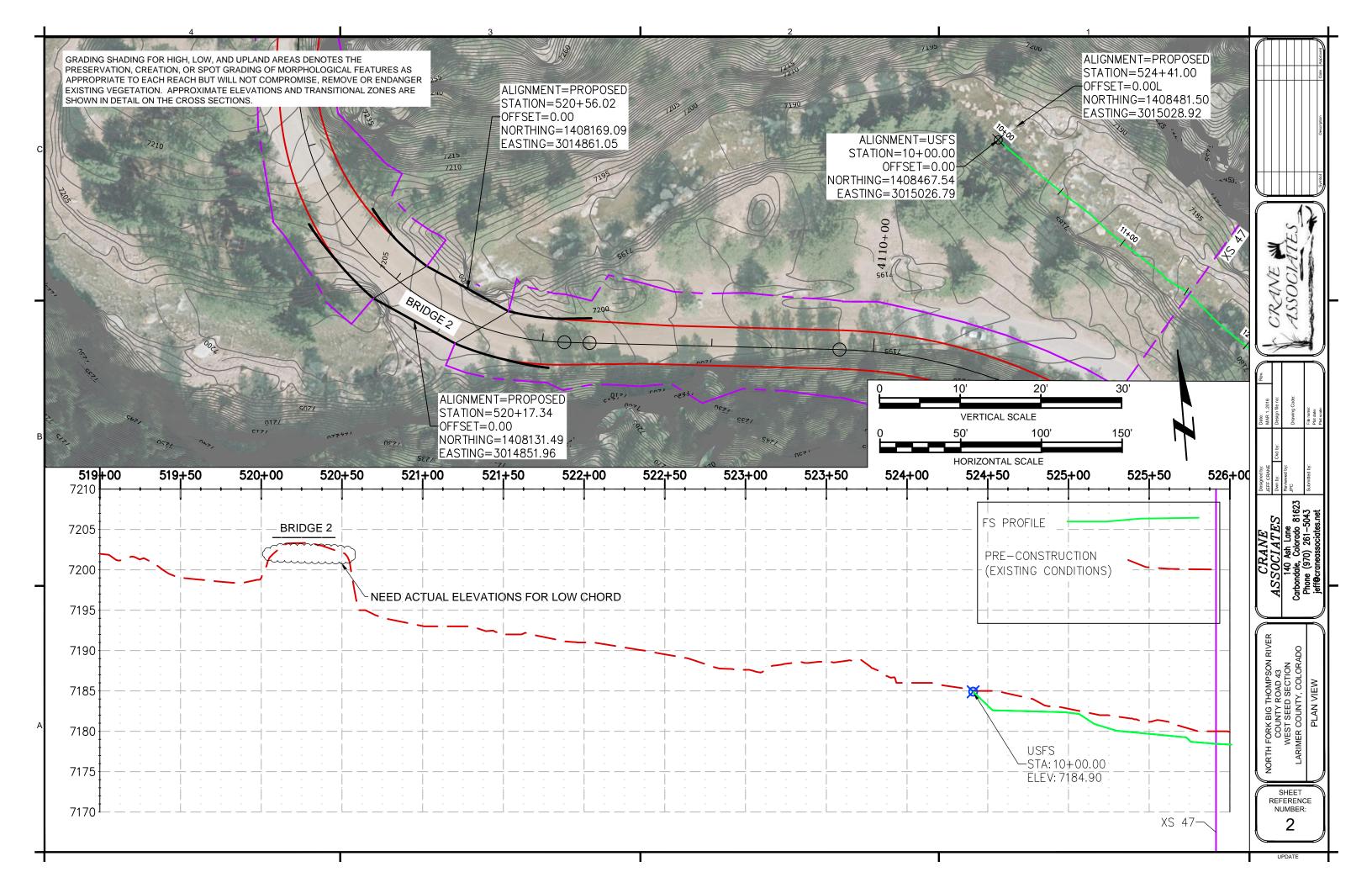
The September 2013 flood event significantly damaged large portions of Larimer CR 43 along North Fork of the Big Thompson River. Emergency roadway reconstruction activities were underway immediately following the flood, and a unique partnership quickly formed between federal and county highway agencies and state and private stream restoration experts that allowed river rehabilitation to occur concurrently as part of the permanent road reconstruction effort through the CR 43 corridor. This collaborative process proved to be a cost-effective and efficient way to protect the road from future river instability by rehabilitating the river utilizing roadway reconstruction resources to design and construct stream channel enhancements. Similar partnerships where mutual benefit can occur are recommended and should be sought out. Not only did the partnership save millions of dollars in stream restoration costs throughout this corridor, but both the stream restoration project and the roadway reconstruction project came away with lasting benefits. The collaborative process and joint projects resulted in a river/roadway corridor that is much improved and more resilient, from the standpoints of public safety, infrastructure protection, environmental and ecological health, and recreational opportunity.

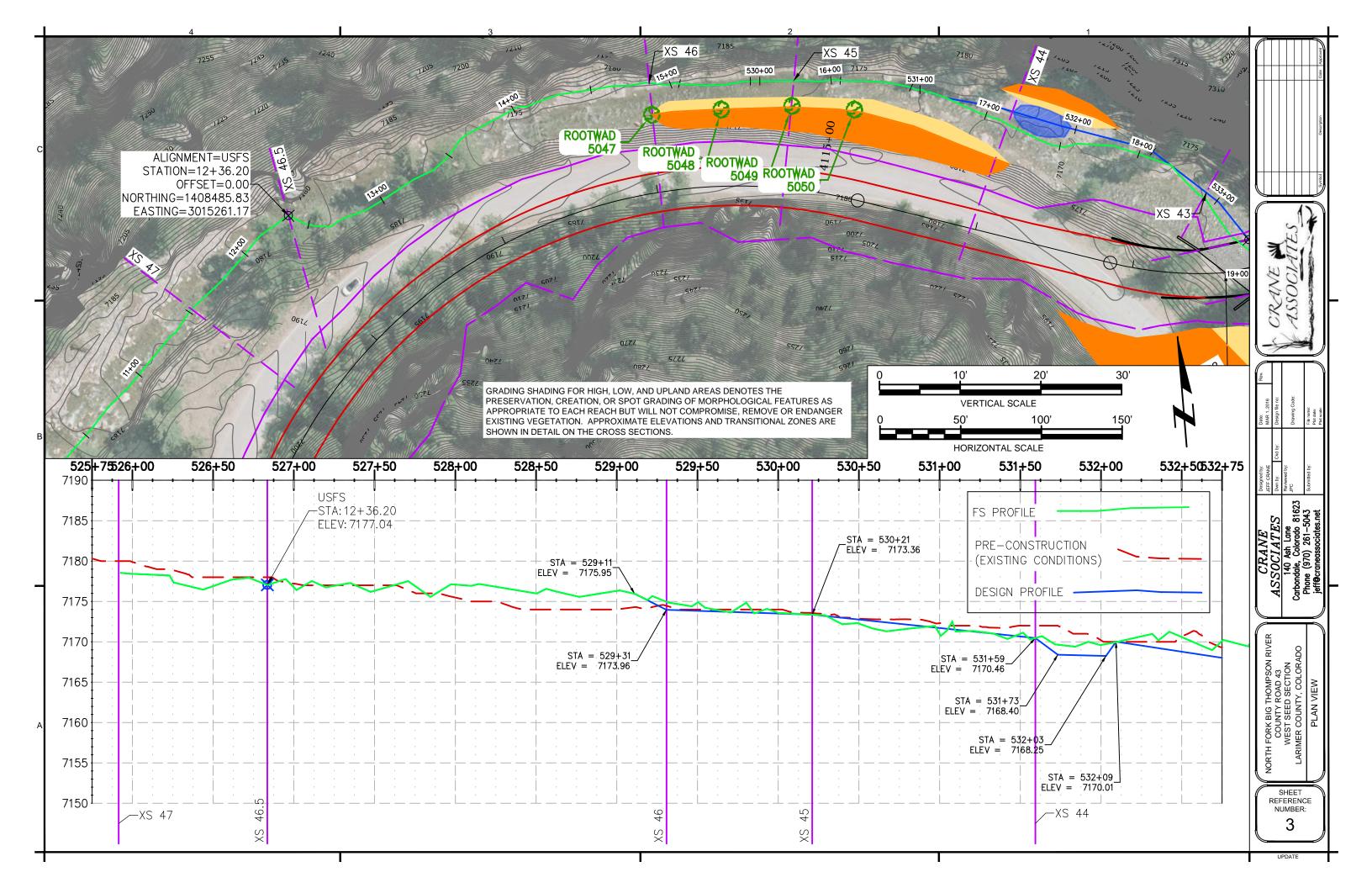
It should be noted the project management and design staff from Central Federal Lands worked pro-actively with US Forest Service staff, Larimer County Engineering and private landowners to make this project the success it was. Without their forward-thinking approach to consolidate resources and develop resiliency of the design and construction, this project may have simply been another rebuilding of the existing roadway alignment and thus losing an opportunity to build better.

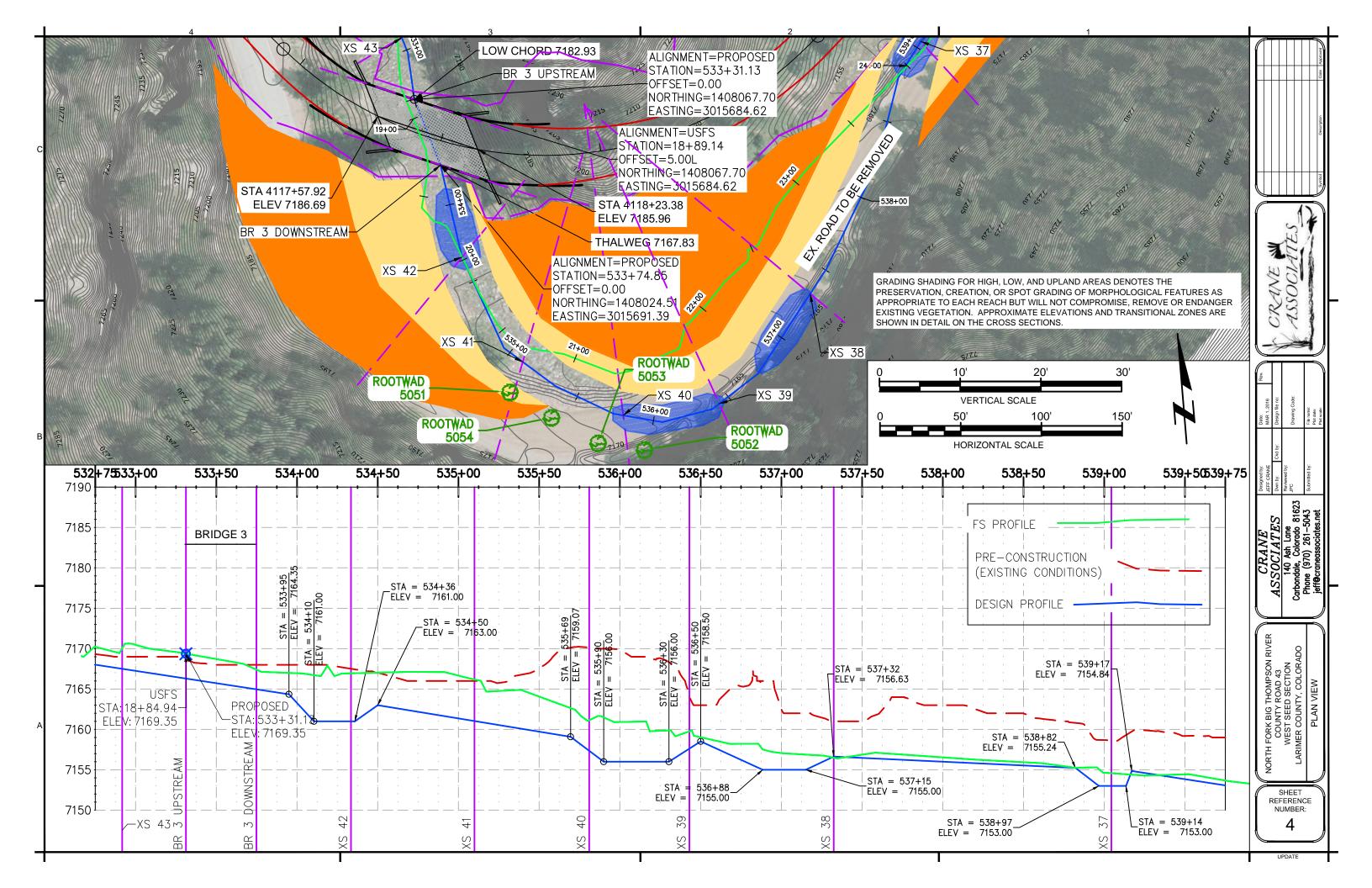
Appendix A: West Seed Plans

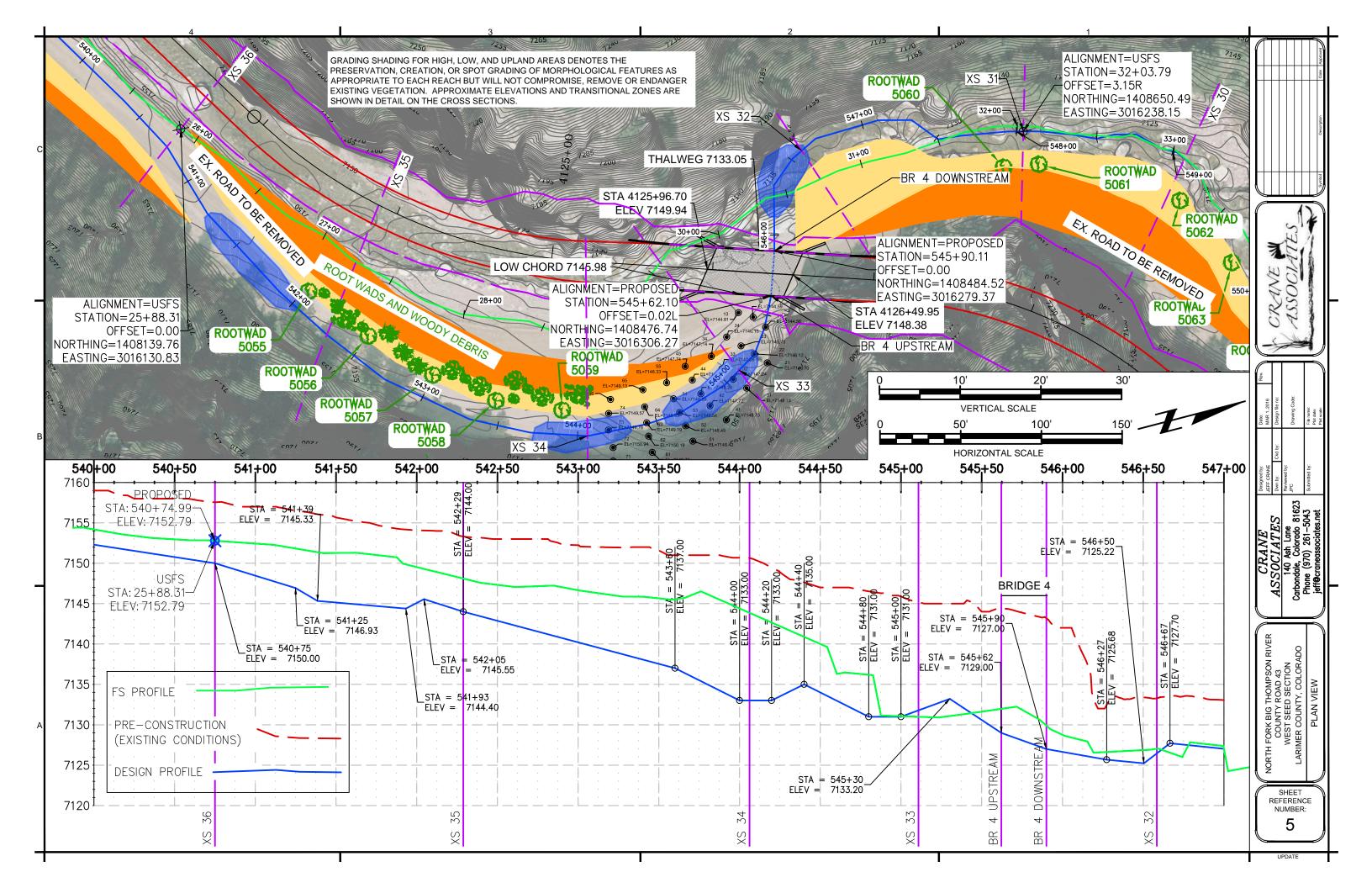


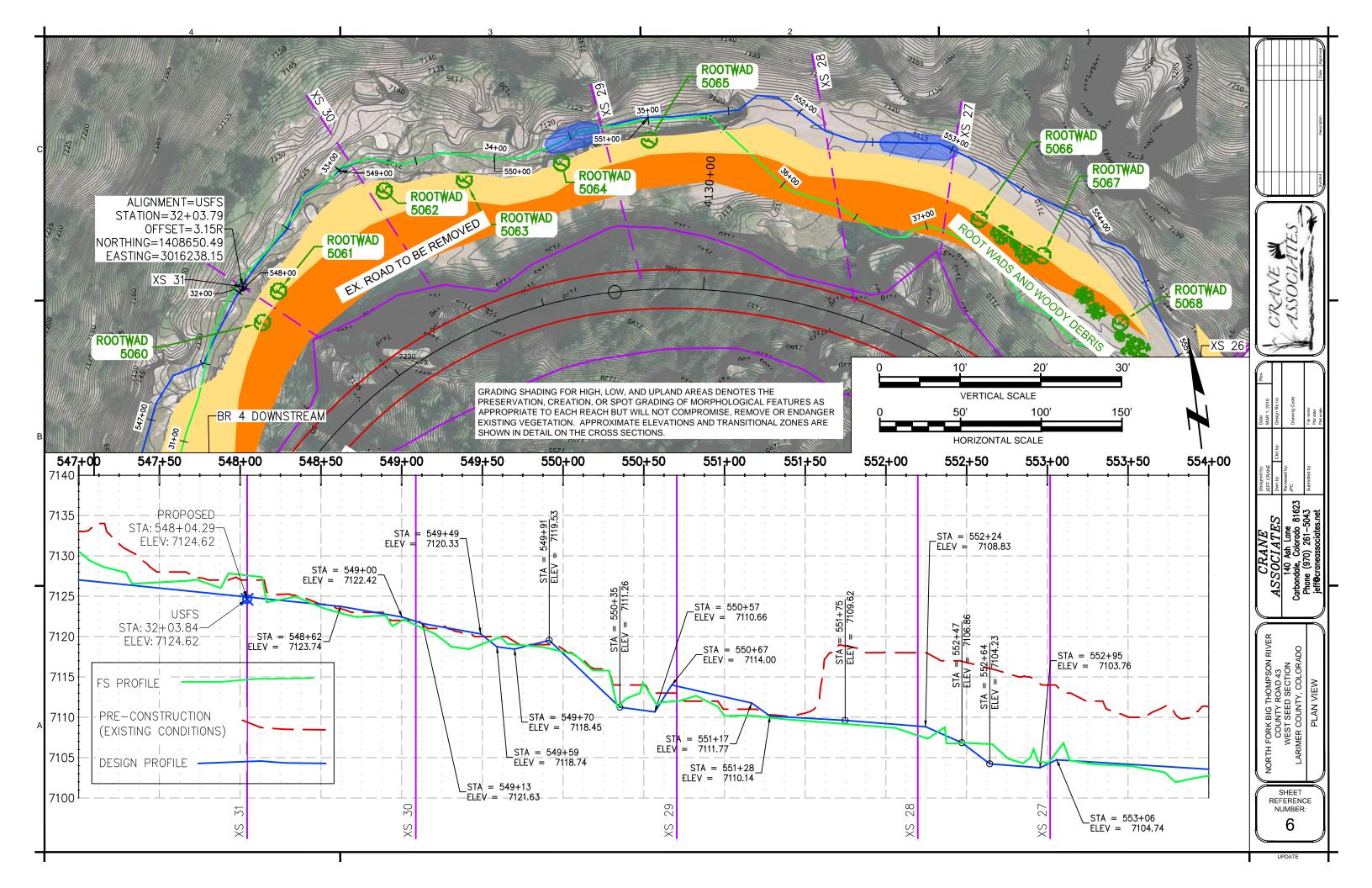


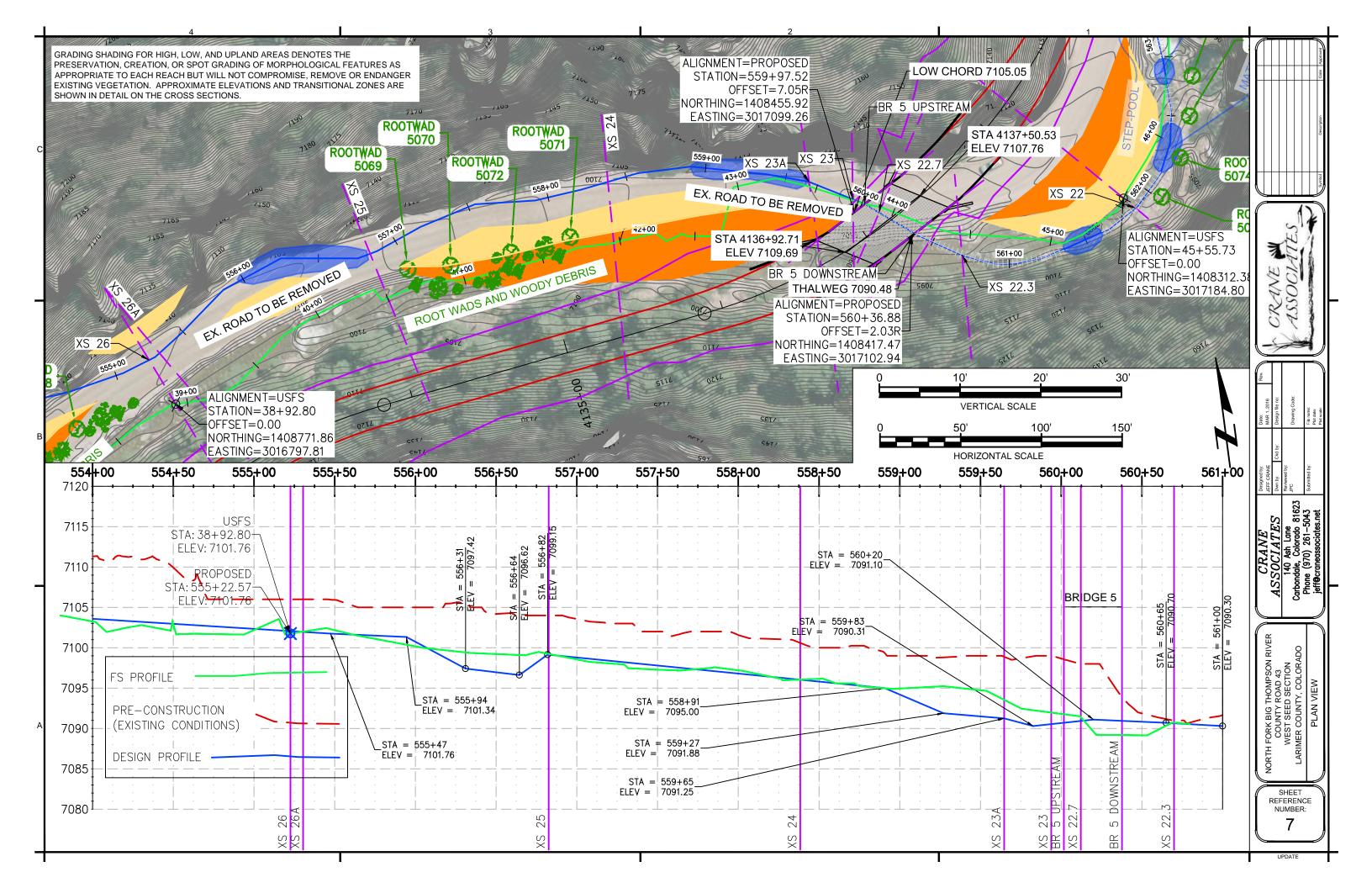


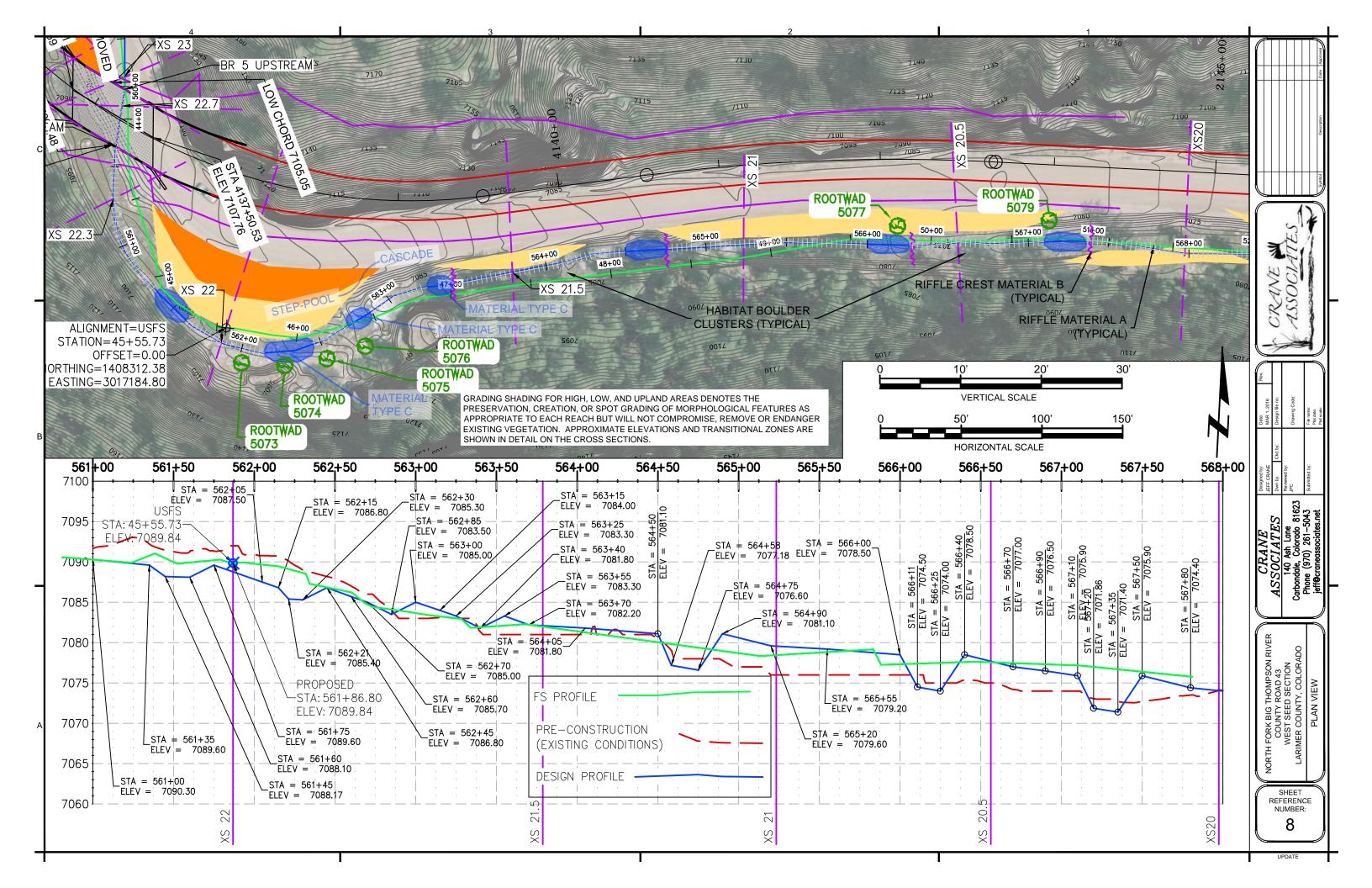


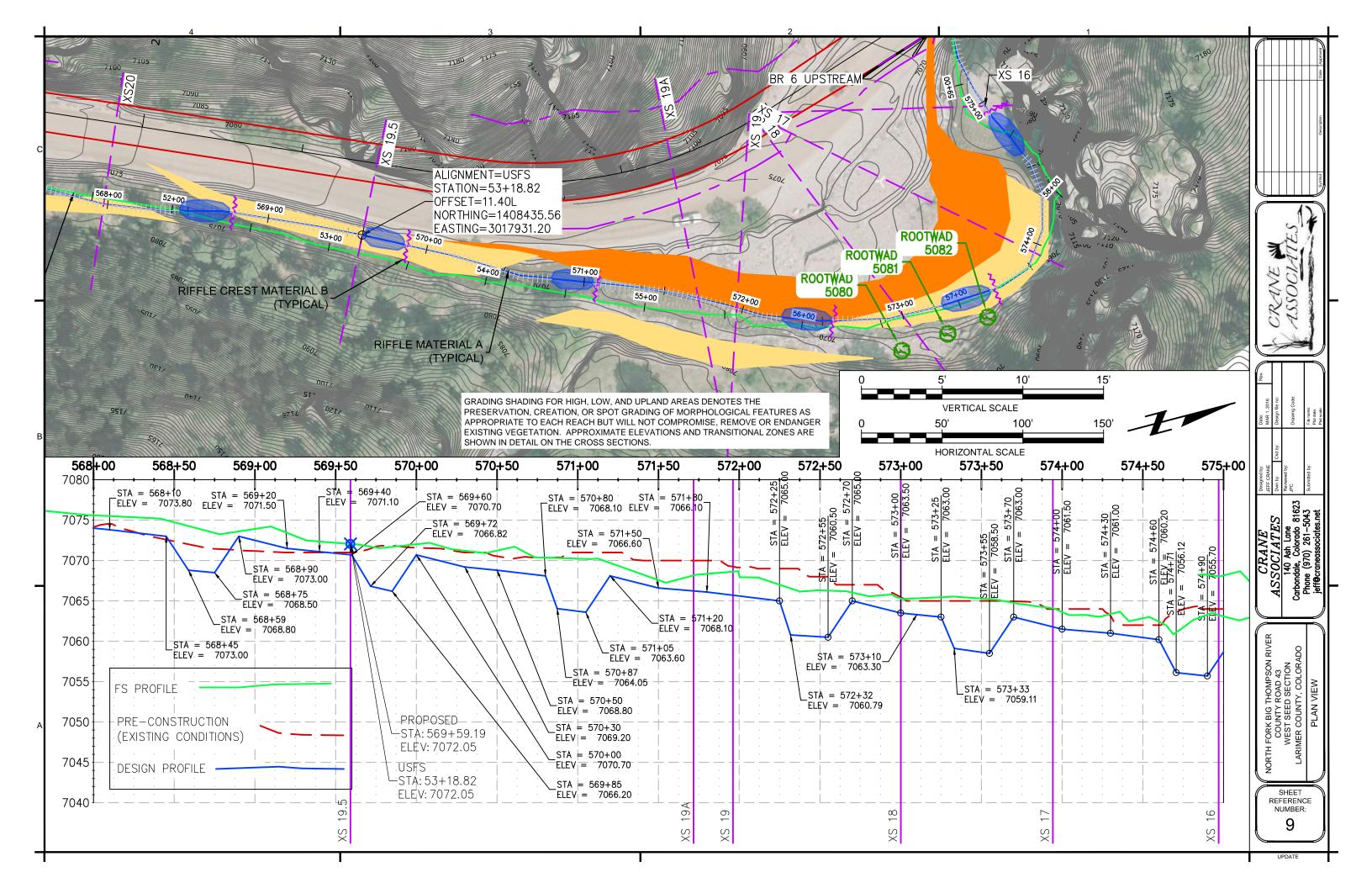


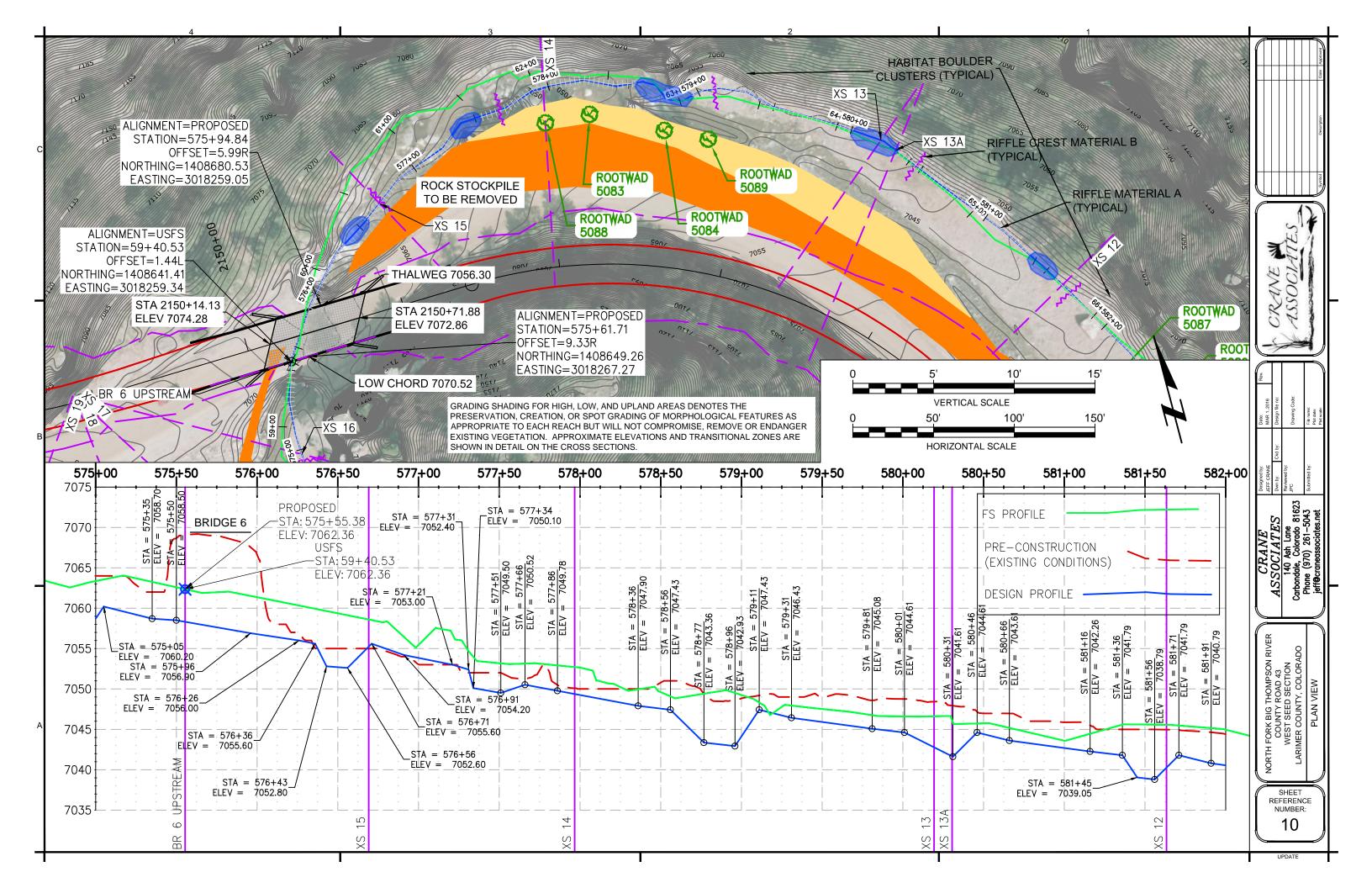


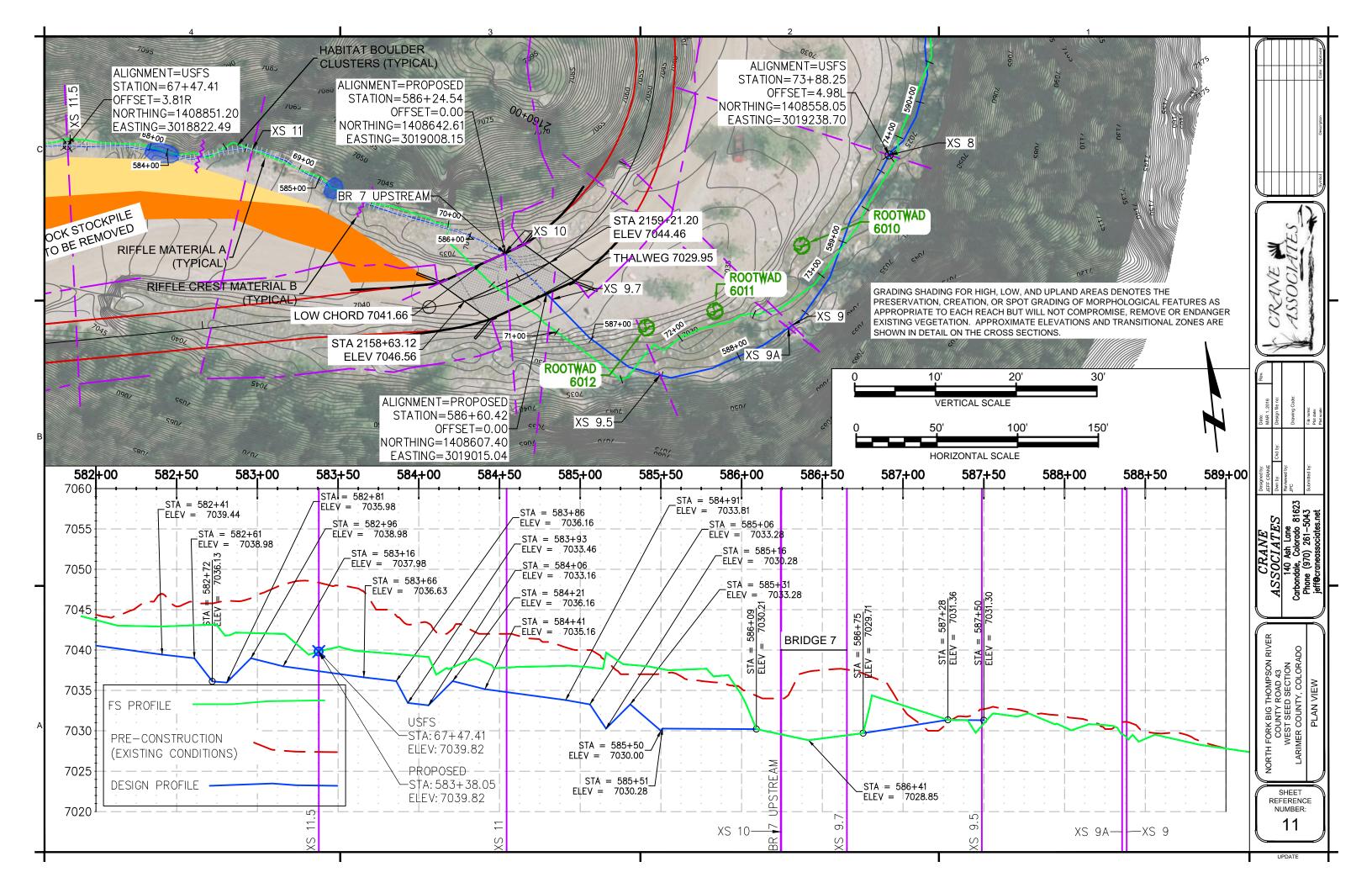




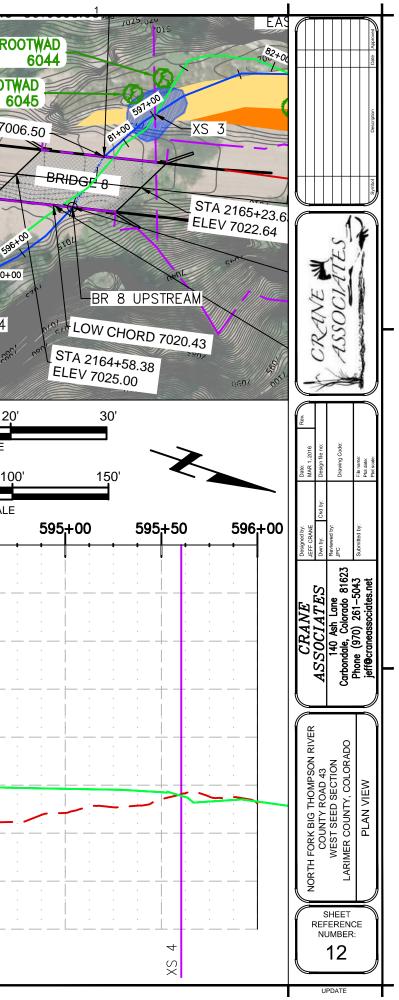


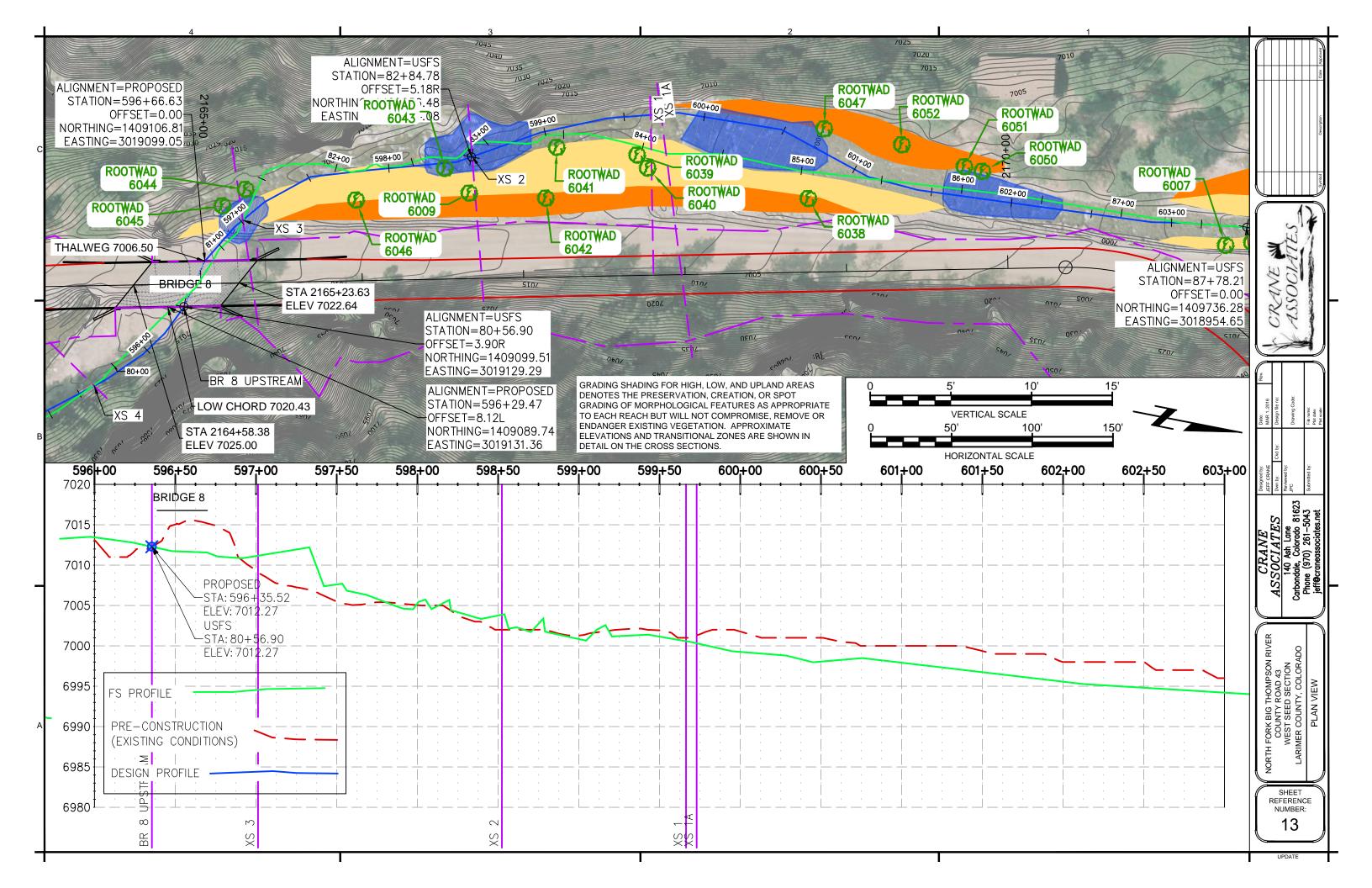


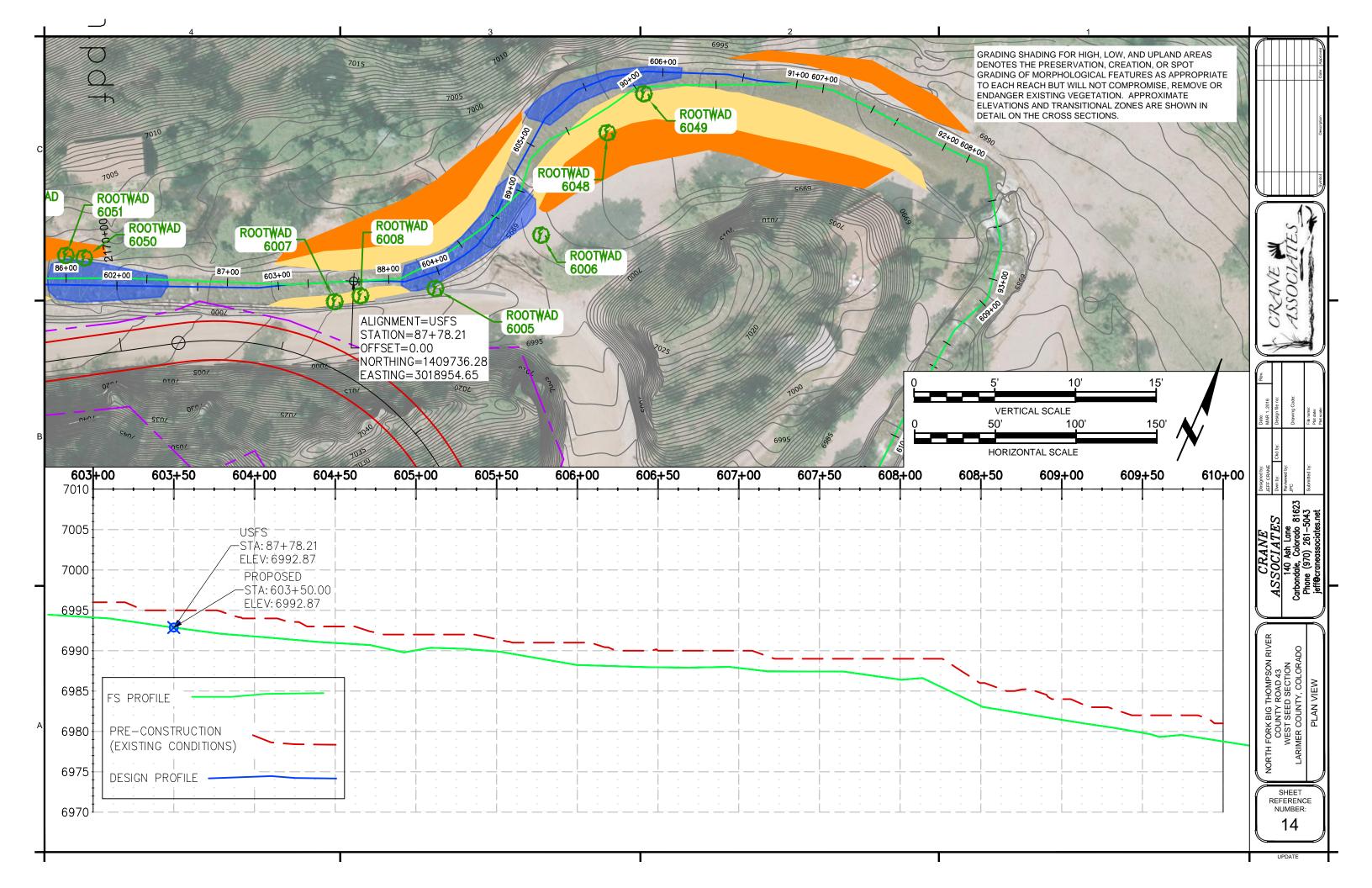




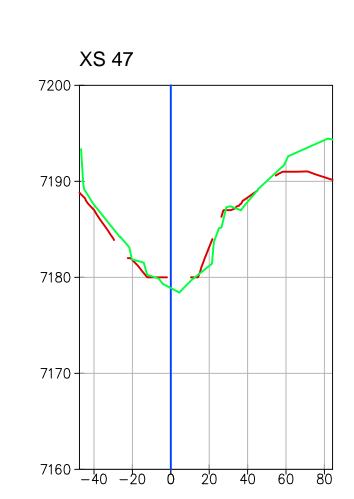
ж. ЖS	2005 14400 800-000	STATION OFFSET= NORTHIN	NT=USFS =73+88.25 =4.98L IG=1408558.0 =3019238.70		XS.6	XS 64	025 025	7045 7040 7035 7030 7025	7020		R
COL SEAMA	A cont con 00	2- 5-53: 9-5	NO CHANNEY 200 DENOTES THE	ROOTWAD 6016 ROOTWA 6017 WORK HERE 02 592 76+09 400		ROOTWAD 6015 593+00	POP POP ROOTWAL 6013	Battoo Battoo	79+00	972 595+00 5 5 A	10'
	REATION, OR SPOT	GRADING OF MORPH				77+00		c70/			
APPROPRIATE TO E	REATION, OR SPOT EACH REACH BUT V ION. APPROXIMAT ON THE CROSS SE	GRADING OF MORPH WILL NOT COMPROMIS	SE, REMOVE OR EN	IDANGER	0702 0607 591+50		592+50	593+00	593+50	0 594+00	50' HORIZONTAL S 594+5
APPROPRIATE TO E EXISTING VEGETATI SHOWN IN DETAIL C 589+00 7040 7035	REATION, OR SPOT EACH REACH BUT V ION. APPROXIMAT ON THE CROSS SE	GRADING OF MORPH WILL NOT COMPROMIS TE ELEVATIONS AND T CTIONS.	SE, REMOVE OR EN RANSITIONAL ZON	IDANGER ES ARE	0+0,	0 5507		XS 5		0 594+0(HORIZONTAL
APPROPRIATE TO E EXISTING VEGETAT SHOWN IN DETAIL C 589+00 7040	REATION, OR SPOT EACH REACH BUT V ION. APPROXIMAT ON THE CROSS SE	GRADING OF MORPH WILL NOT COMPROMIS TE ELEVATIONS AND T CTIONS. 590+00 PROPOSED STA: 589+61. ELEV 7027.81	SE, REMOVE OR EN RANSITIONAL ZON	IDANGER ES ARE	0+0,	0 5507		XS 5		0 594+00	HORIZONTAL
APPROPRIATE TO E EXISTING VEGETATI SHOWN IN DETAIL C 7040 7035 7030 7030	REATION, OR SPOT EACH REACH BUT V ION. APPROXIMAT ON THE CROSS SE	GRADING OF MORPH WILL NOT COMPROMIS TE ELEVATIONS AND T CTIONS. 590+00 PROPOSED STA: 589+61. ELEV: 7027.81	SE, REMOVE OR EN RANSITIONAL ZON	IDANGER ES ARE	0+0,	0 5507		XS 5		0 594+0(HORIZONTAL
APPROPRIATE TO E EXISTING VEGETATI SHOWN IN DETAIL C 7040 7035 7030 7025 7020 7020	REATION, OR SPOT EACH REACH BUT V ION. APPROXIMAT ON THE CROSS SE	GRADING OF MORPH WILL NOT COMPROMIS TE ELEVATIONS AND T CTIONS. 590+00 PROPOSED STA: 589+61. ELEV 7027.81	SE, REMOVE OR EN RANSITIONAL ZON	IDANGER ES ARE	0+0,	0 5507		XS 5			HORIZONTAL
APPROPRIATE TO E EXISTING VEGETATI SHOWN IN DETAIL C 7040 7035 7030 7025 7020 7020 7015 FS PF 7010 PRE-	EATION, OR SPOT ACH REACH BUT V ION. APPROXIMAT ON THE CROSS SE 589+50	GRADING OF MORPH WILL NOT COMPROMIS TE ELEVATIONS AND T CTIONS. 590+00 PROPOSED STA: 589+61. ELEV: 7027.81 USFS STA: 73+88.2 ELEV: 7027.8	SE, REMOVE OR EN RANSITIONAL ZON	IDANGER ES ARE	0+0,	0 5507		XS 5			HORIZONTAL
APPROPRIATE TO E EXISTING VEGETATI SHOWN IN DETAIL C 7040 7035 7030 7025 7020 7025 7020 7015 FS PF 7010 PRE- (EXIS	REATION, OR SPOT ACH REACH BUT V ION. APPROXIMAT ON THE CROSS SE 589+50	GRADING OF MORPH WILL NOT COMPROMIS TE ELEVATIONS AND T CTIONS. 590+00 PROPOSED STA: 589+61. ELEV: 7027.81 USFS STA: 73+88.2 ELEV: 7027.8	SE, REMOVE OR EN RANSITIONAL ZON	IDANGER ES ARE	0+0,	0 5507		XS 5			HORIZONTAL

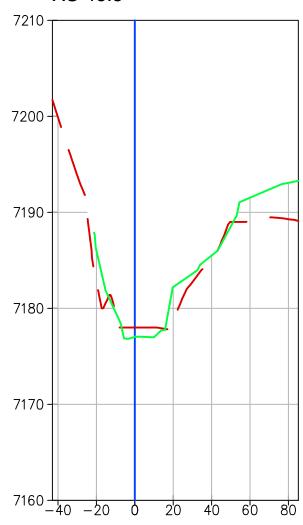


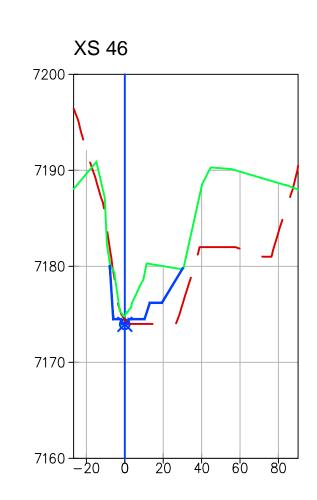


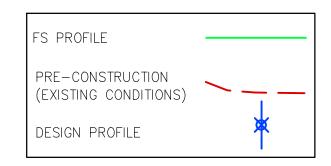


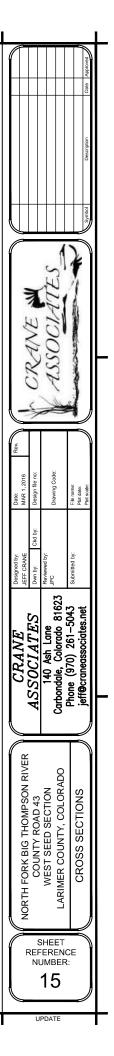
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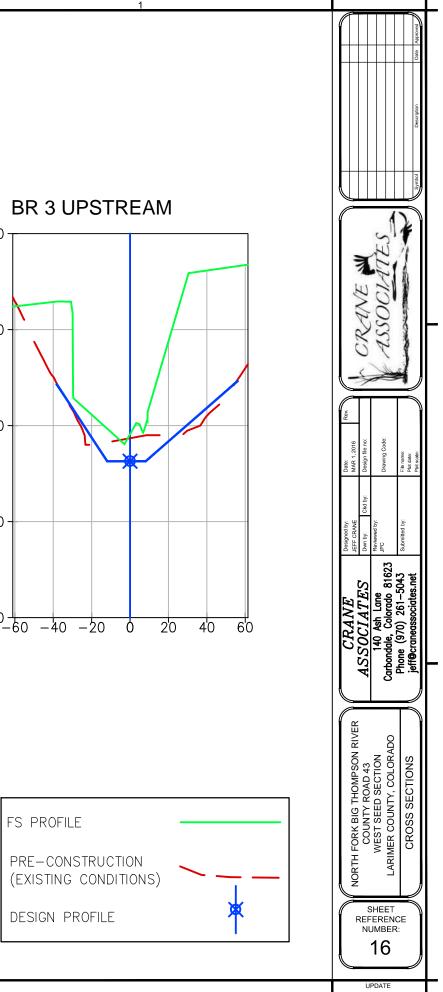


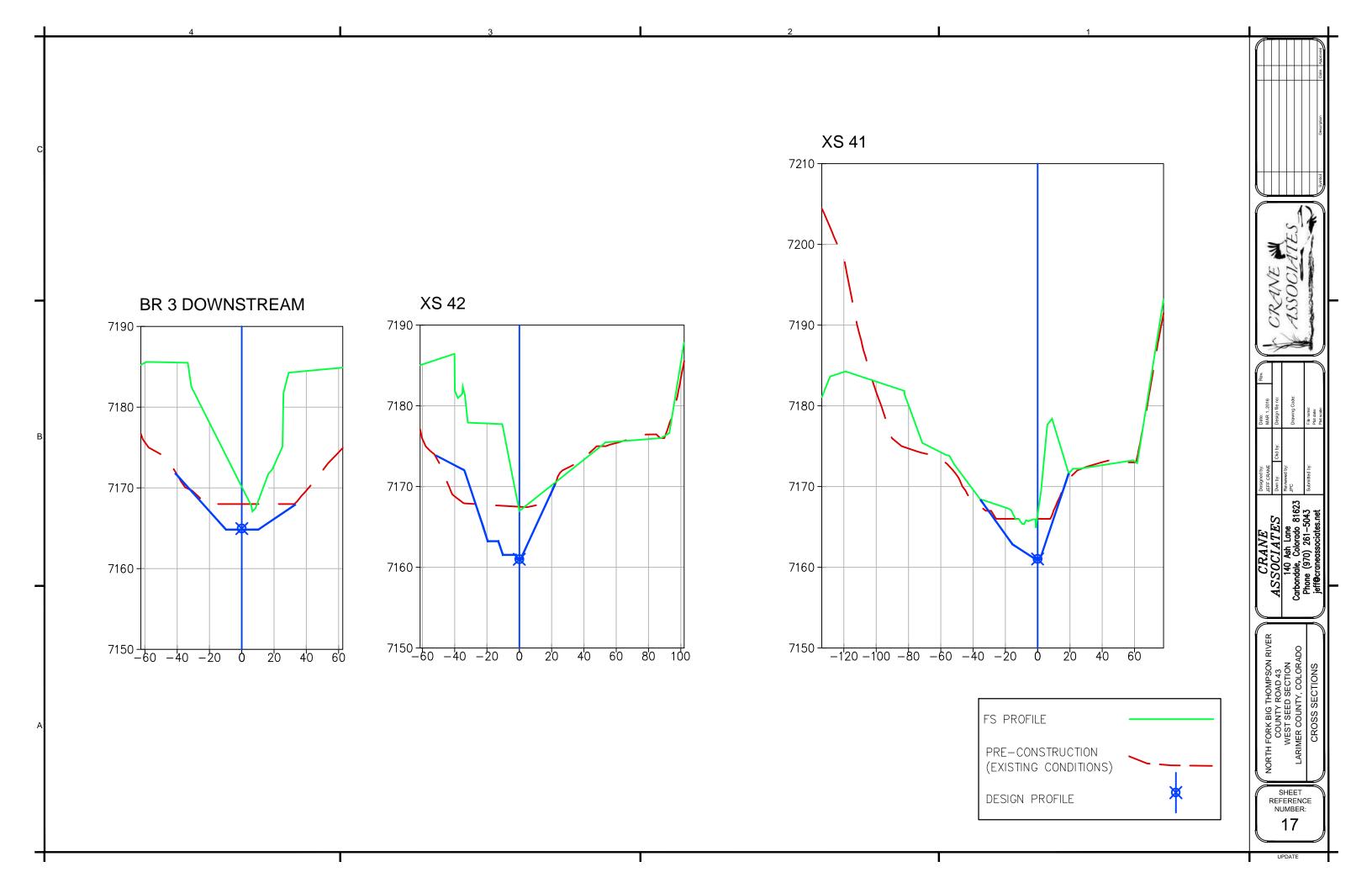


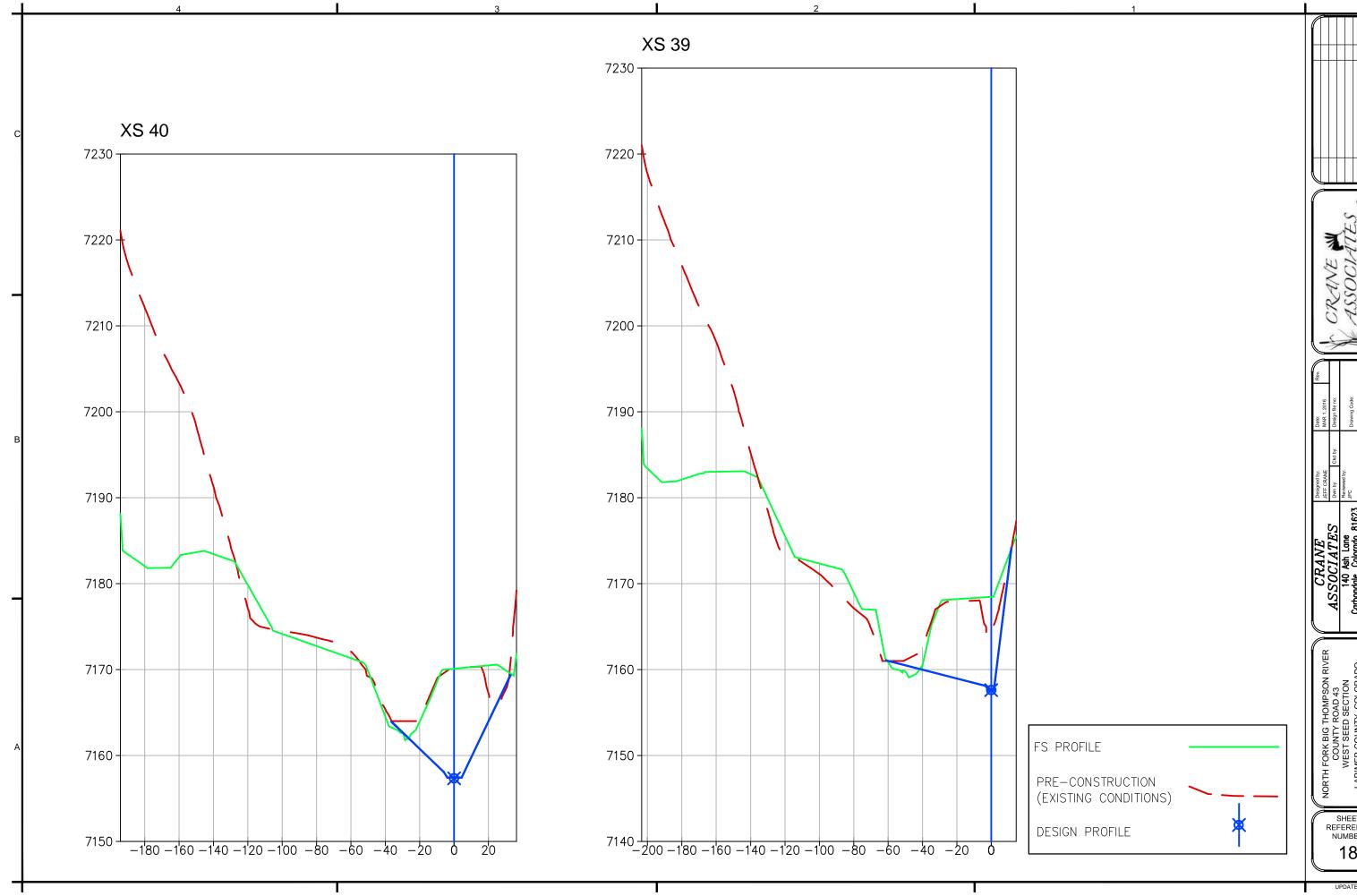


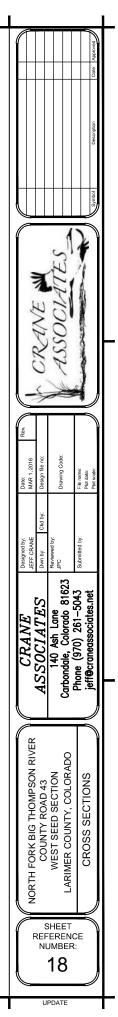


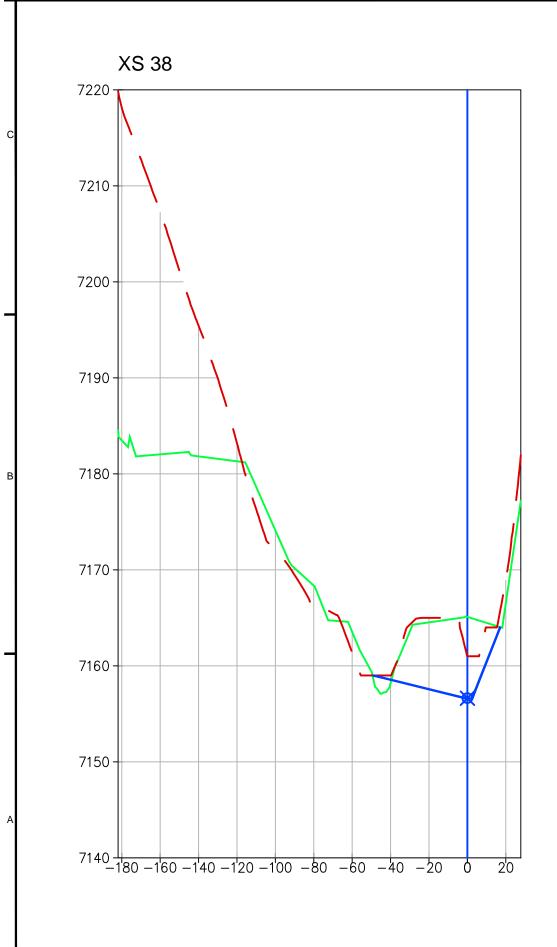
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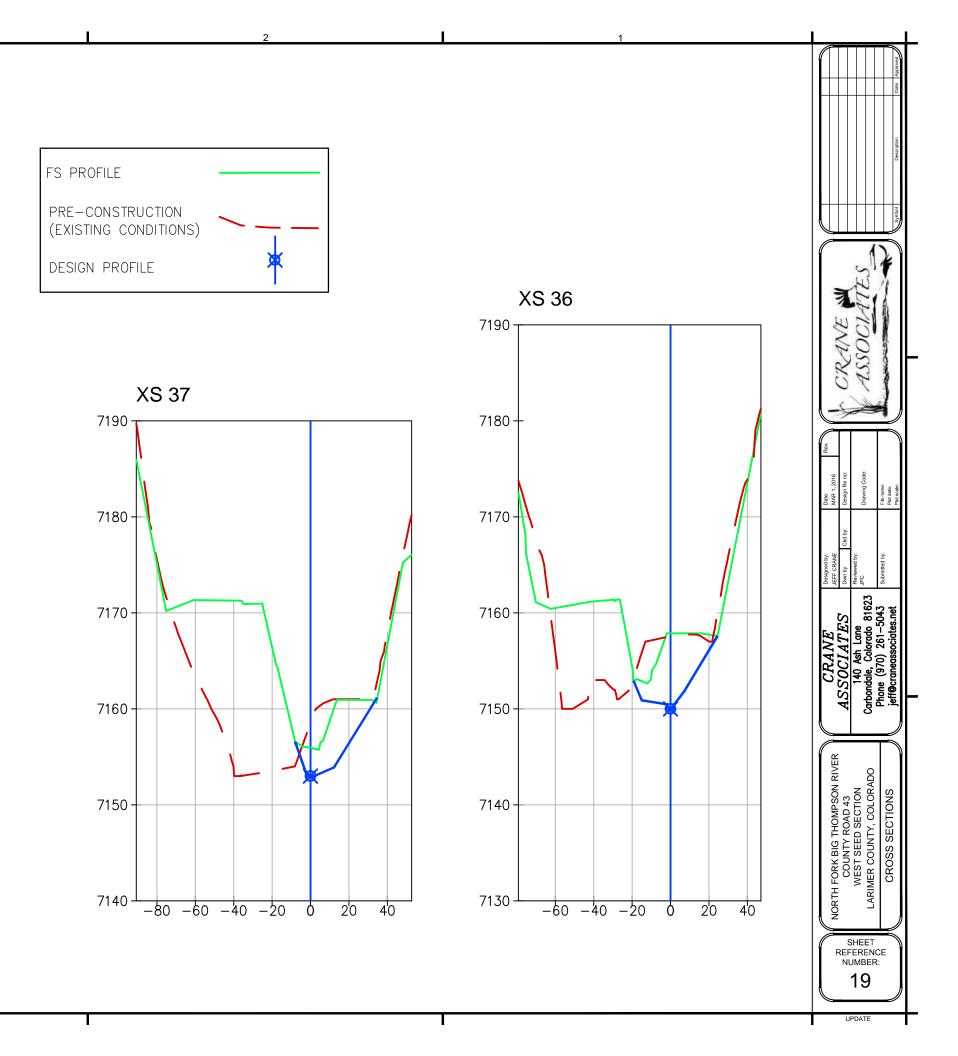


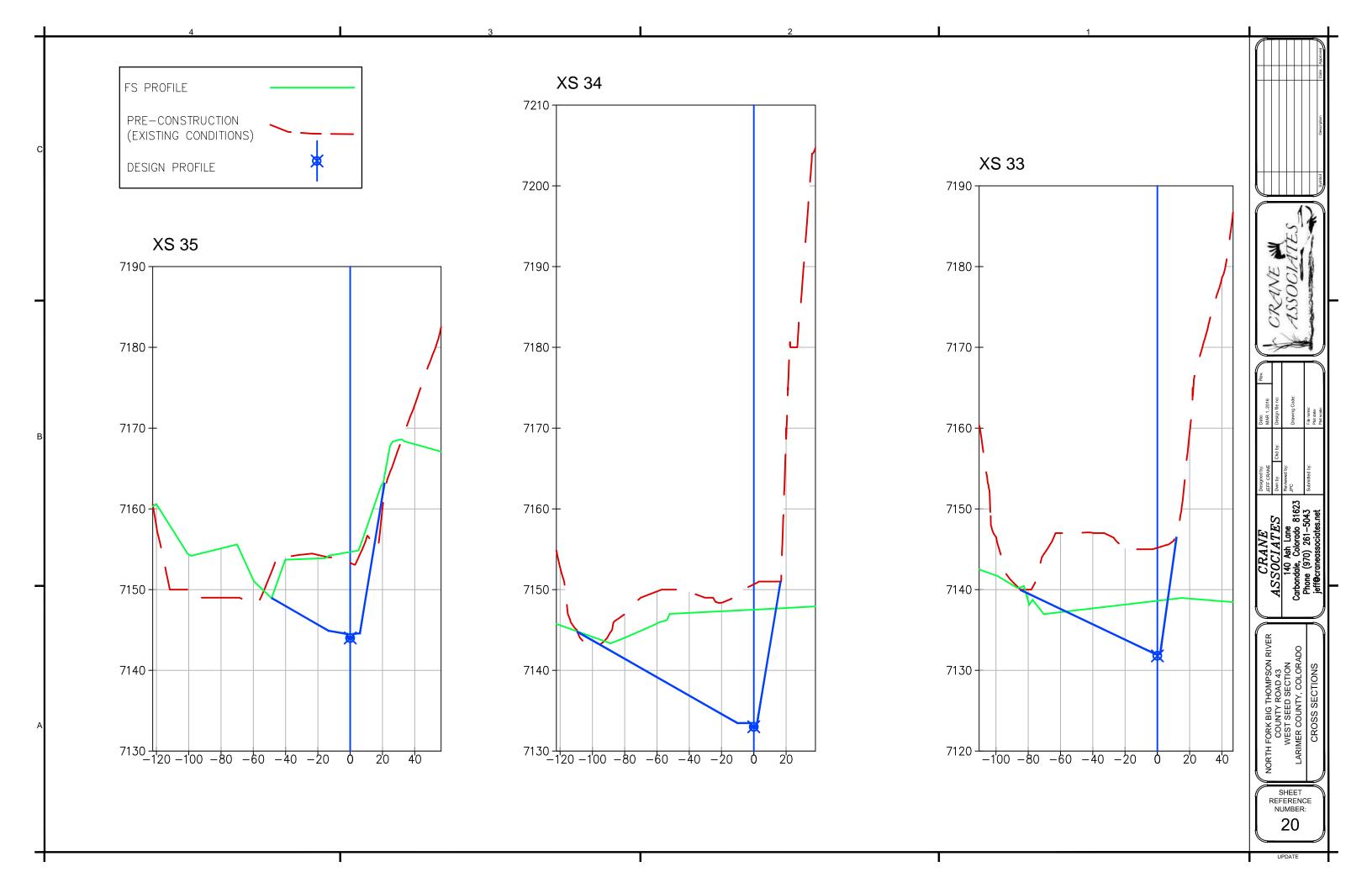


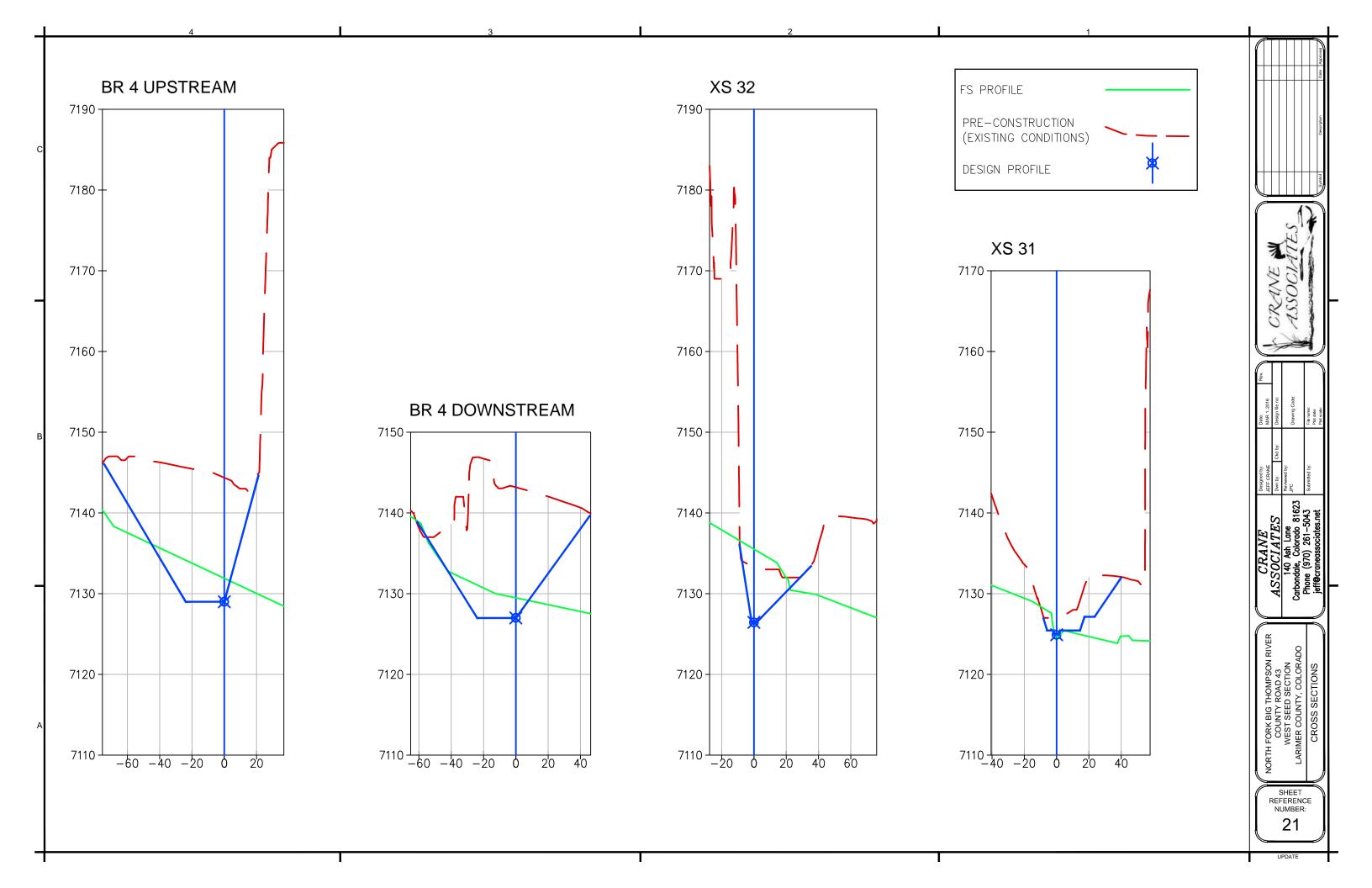


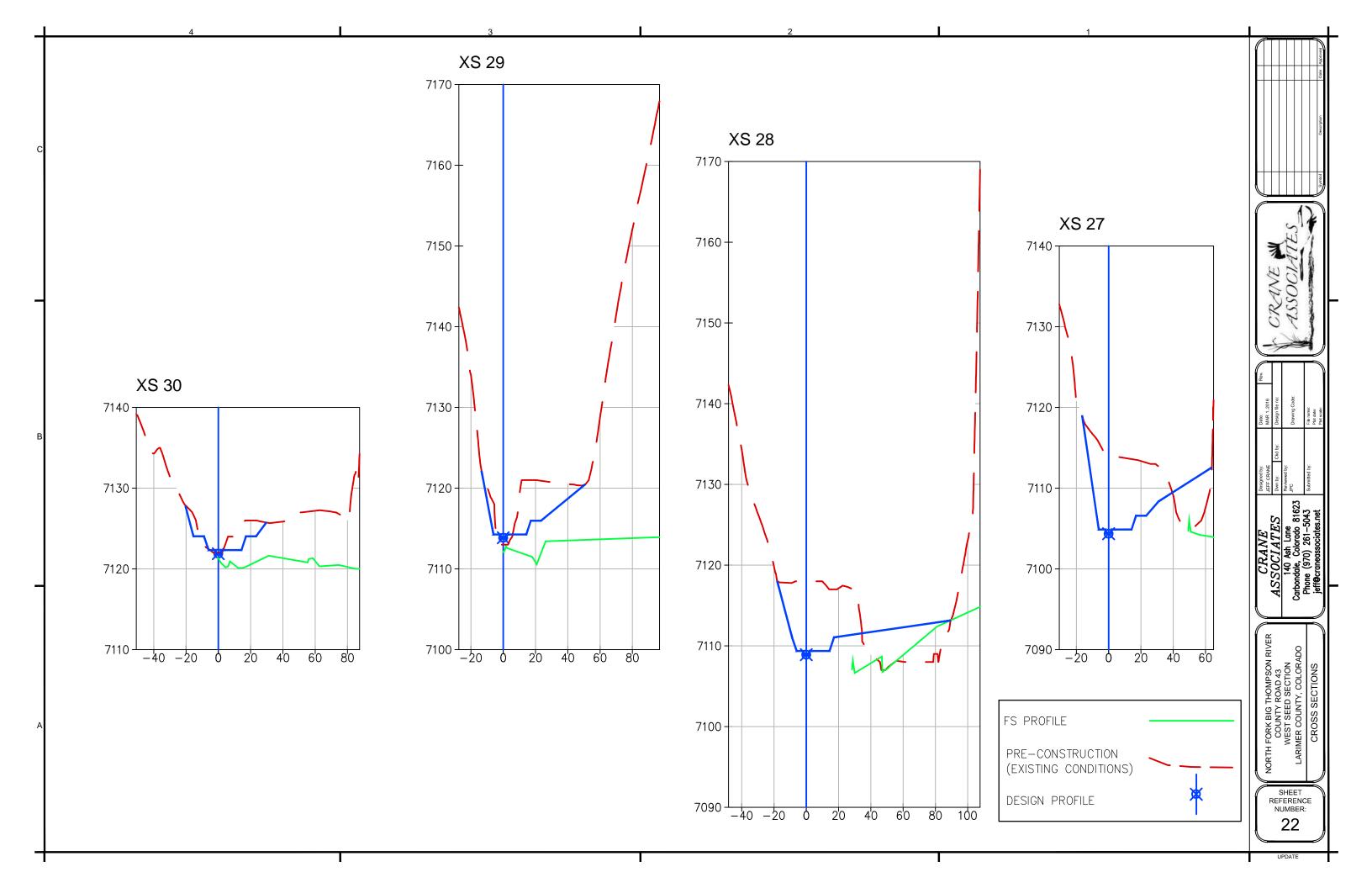


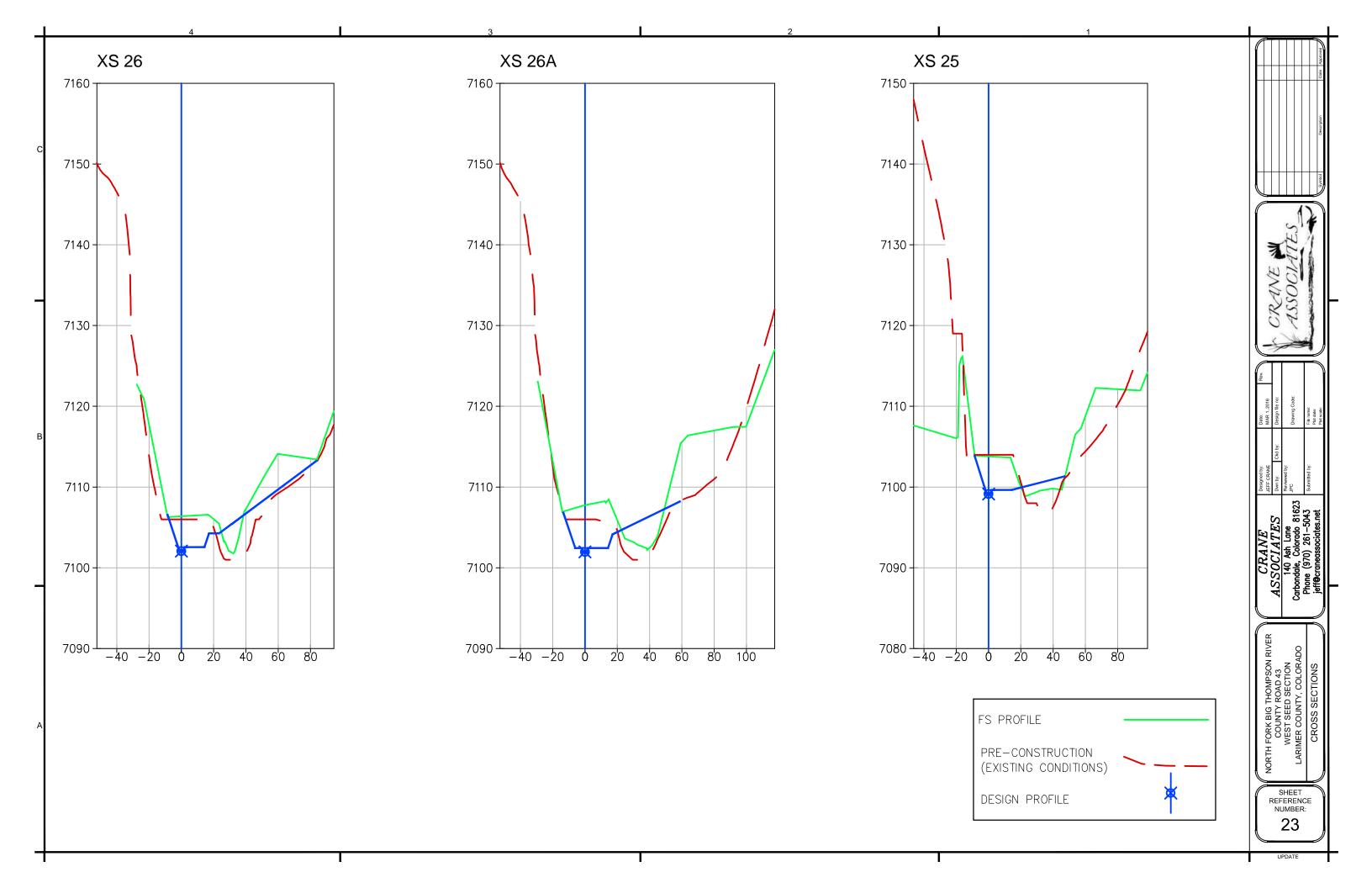


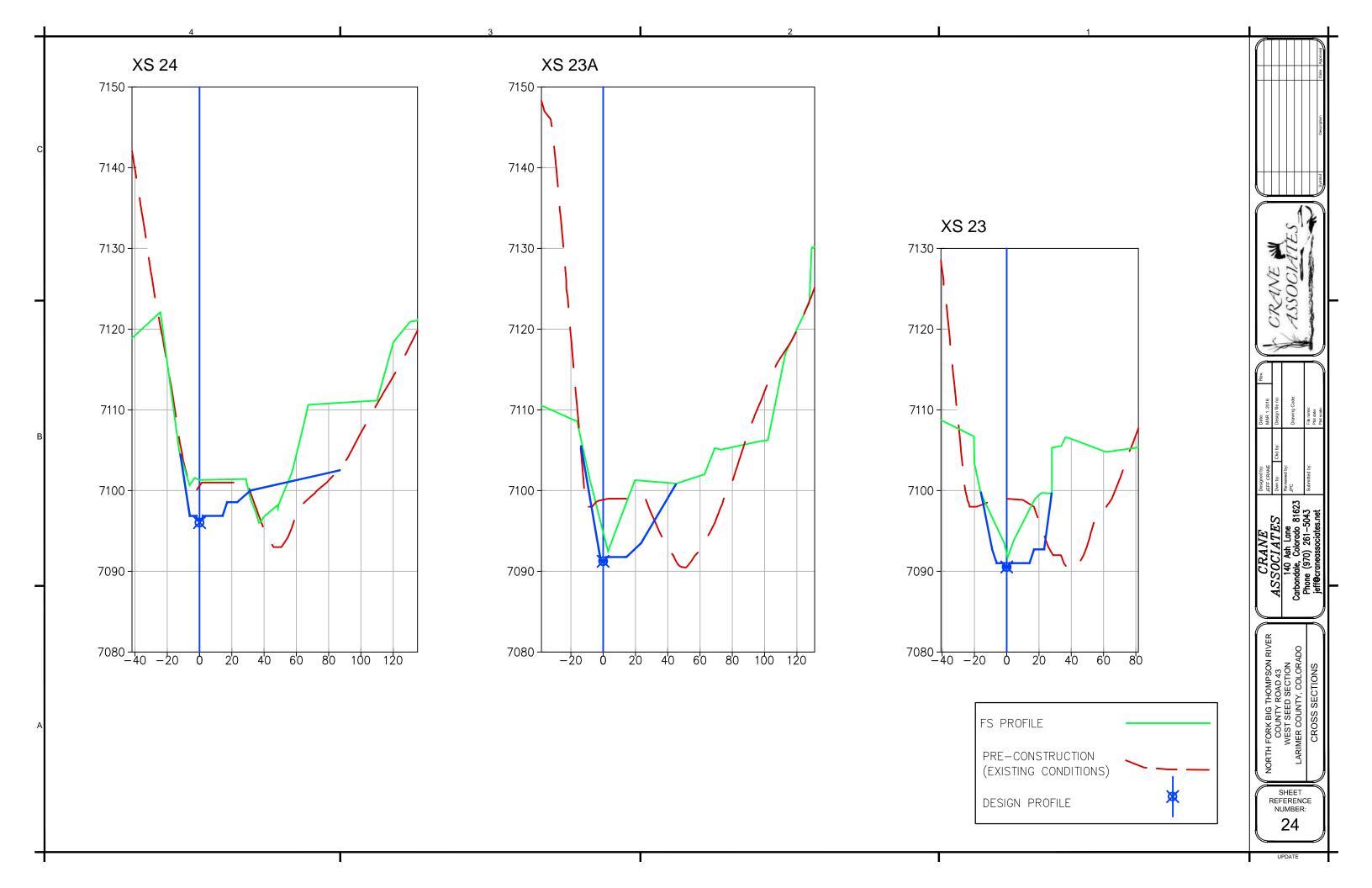


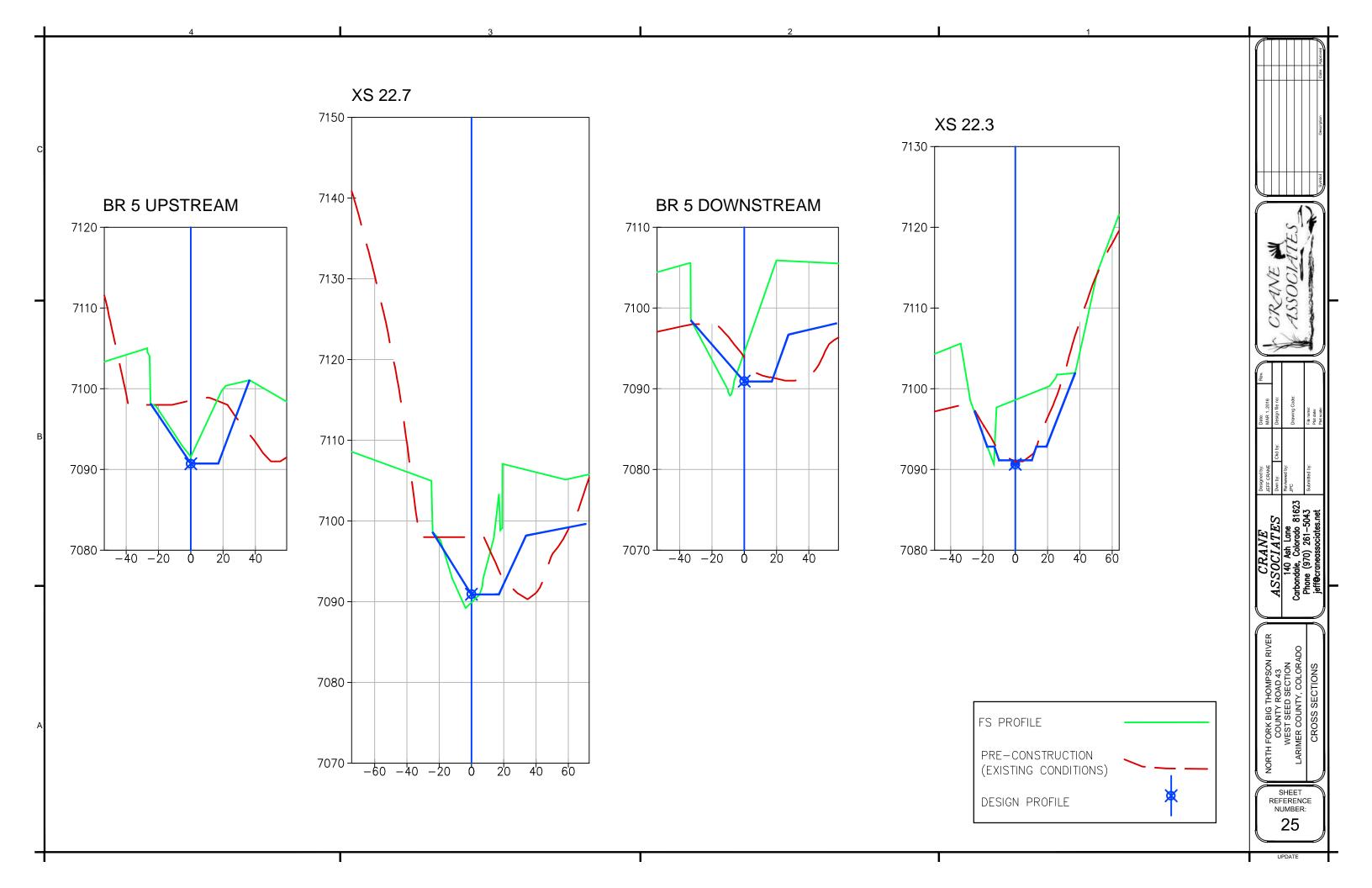


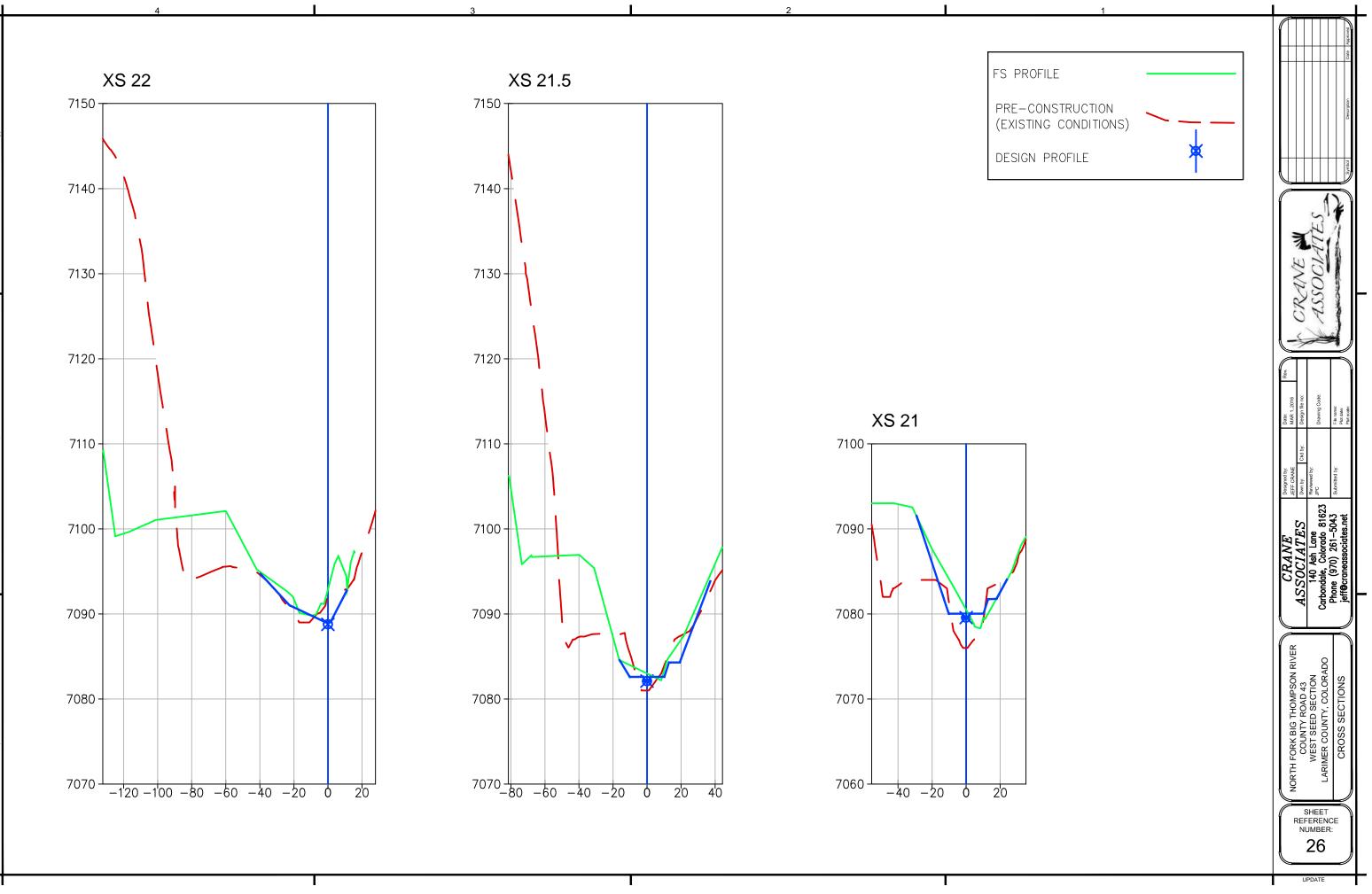


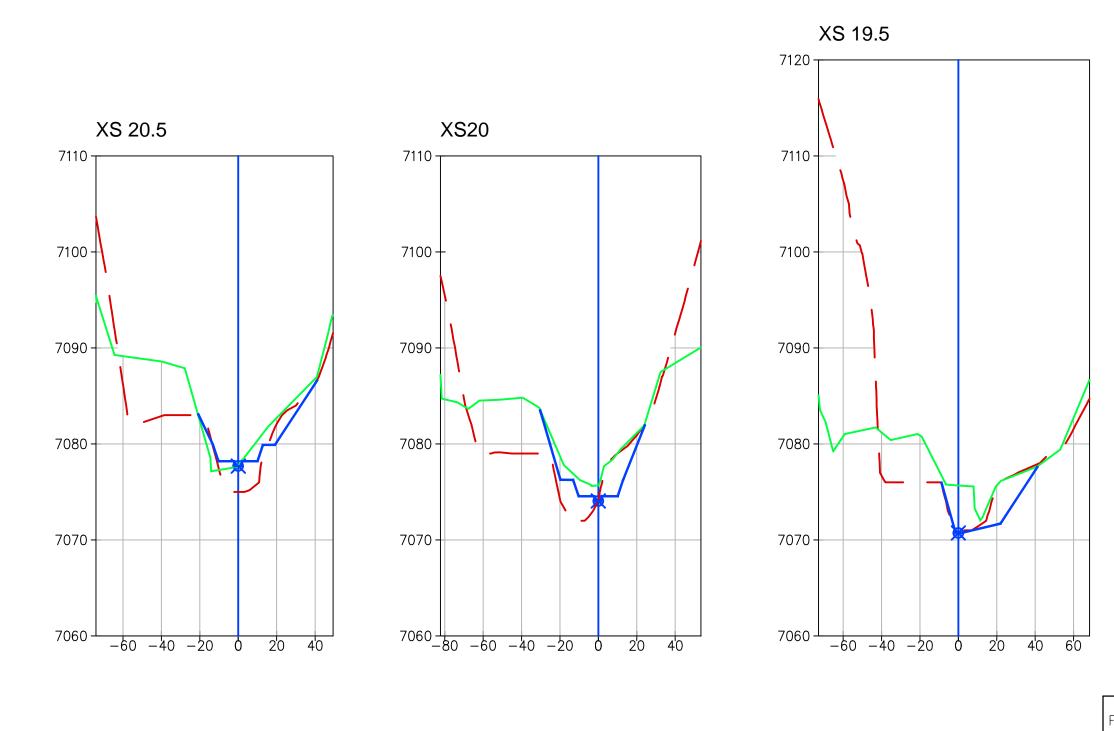


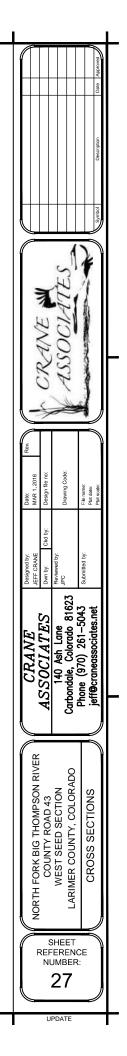


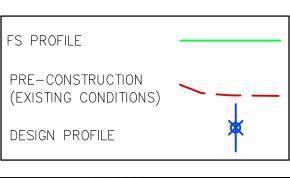


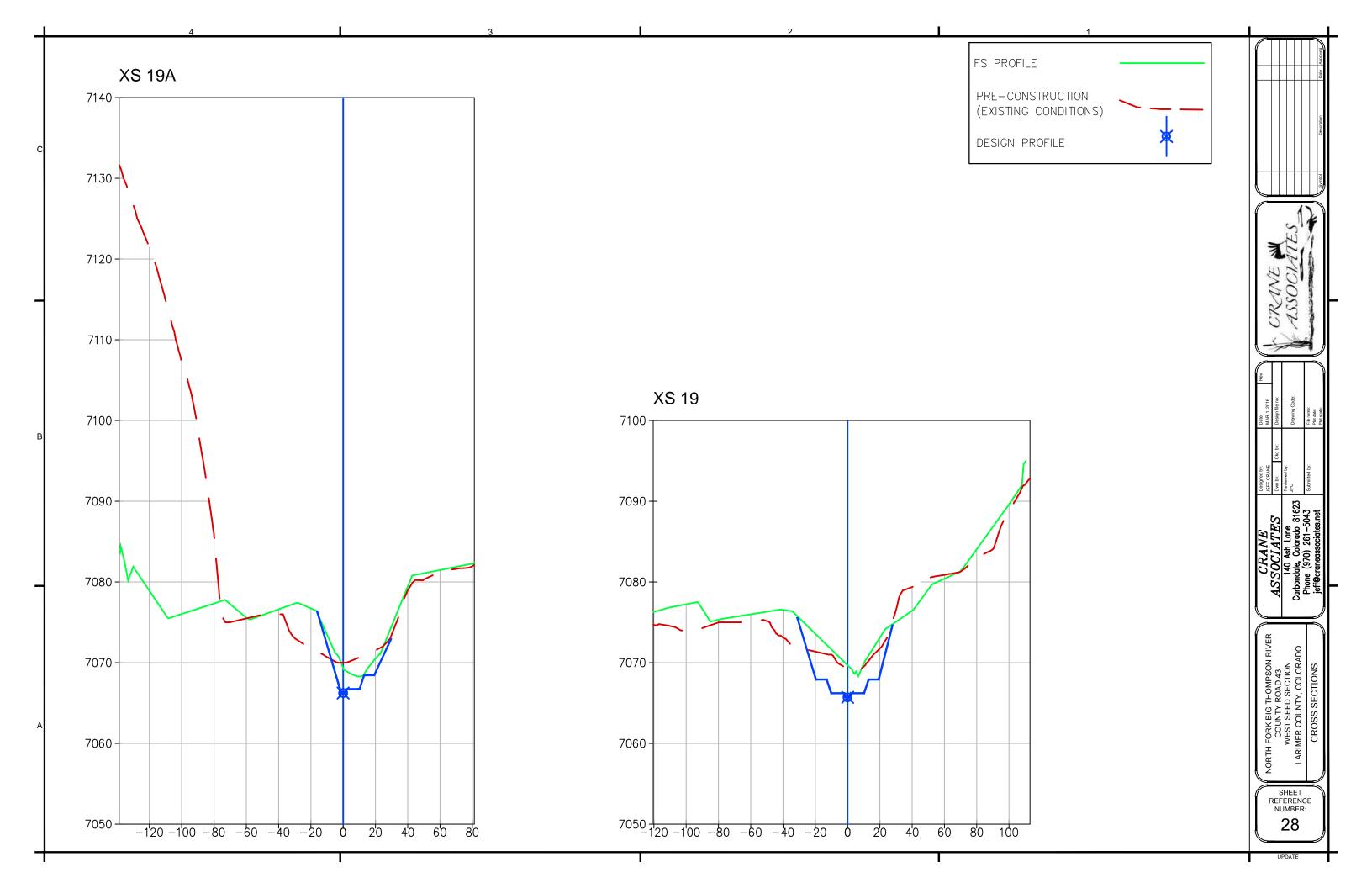


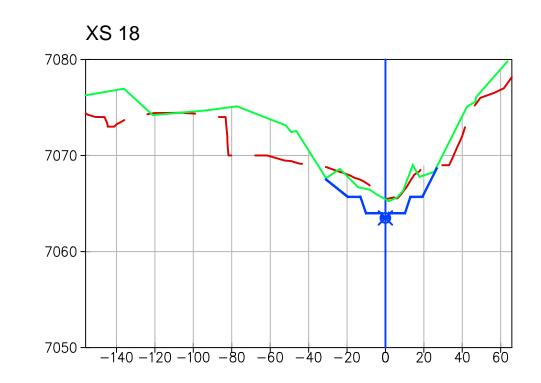




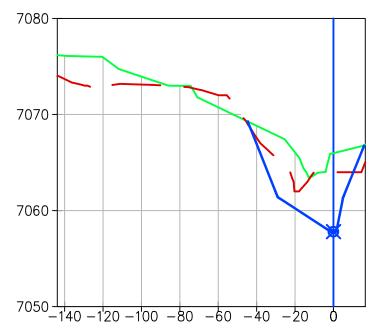




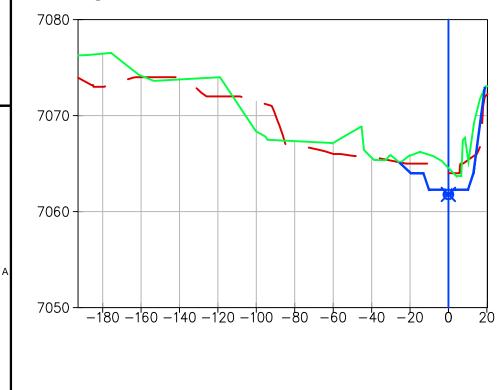


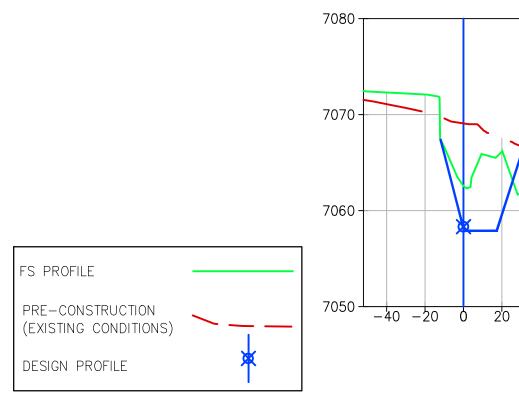


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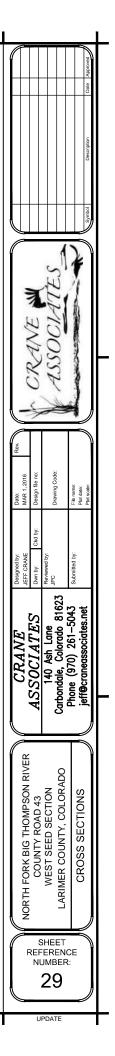
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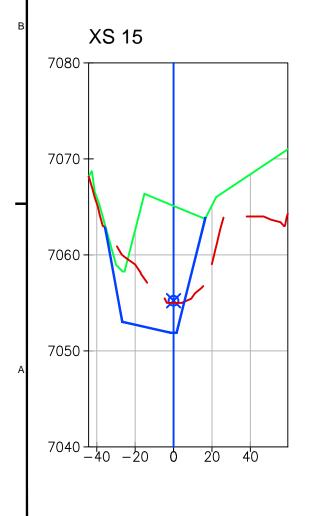


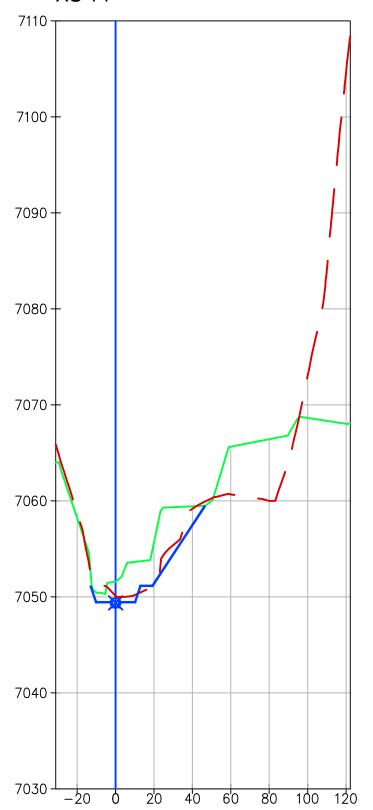
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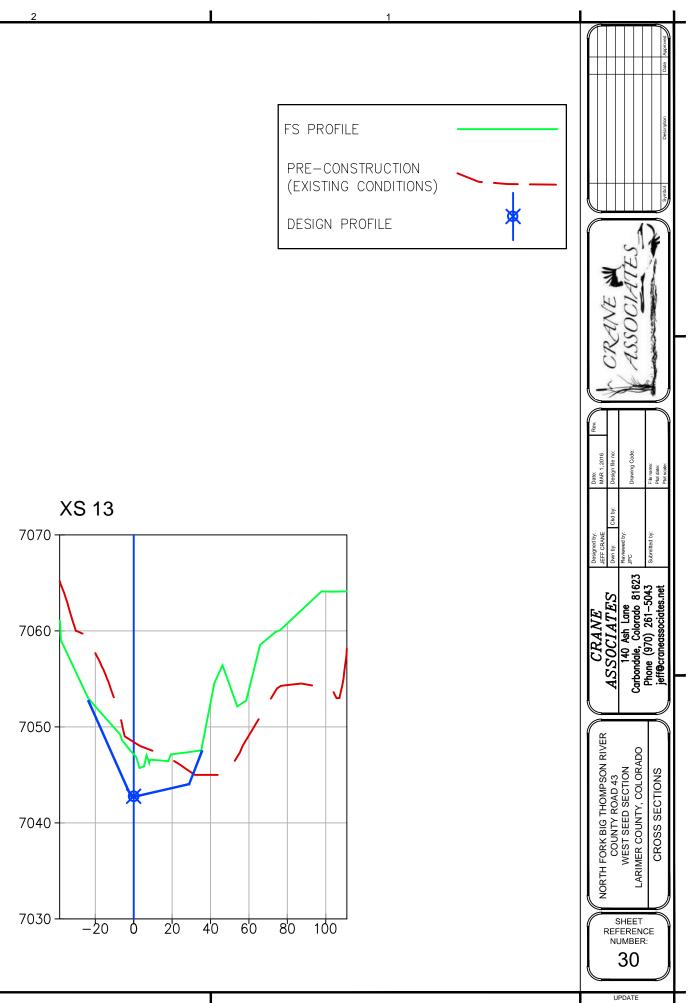
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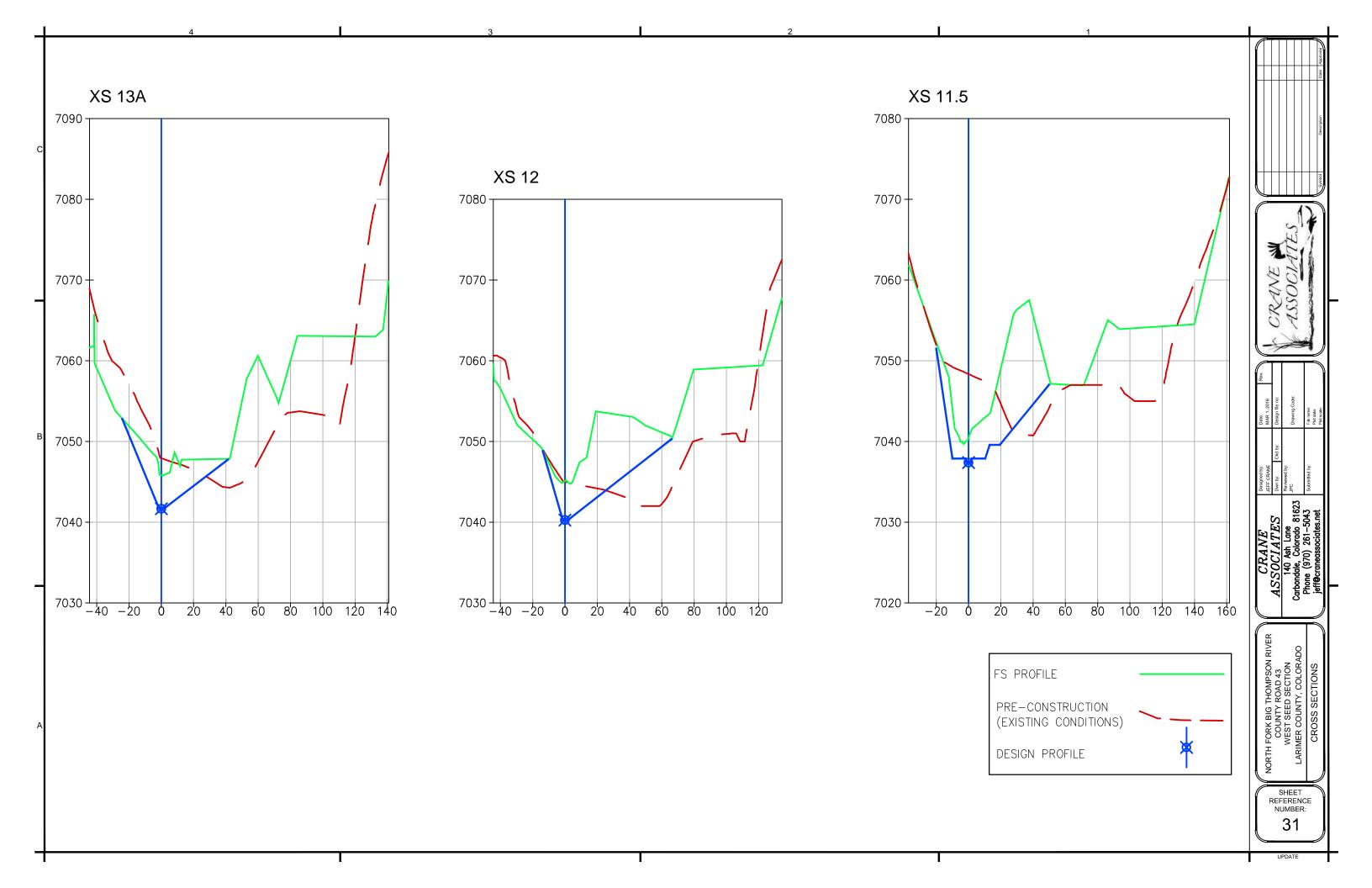


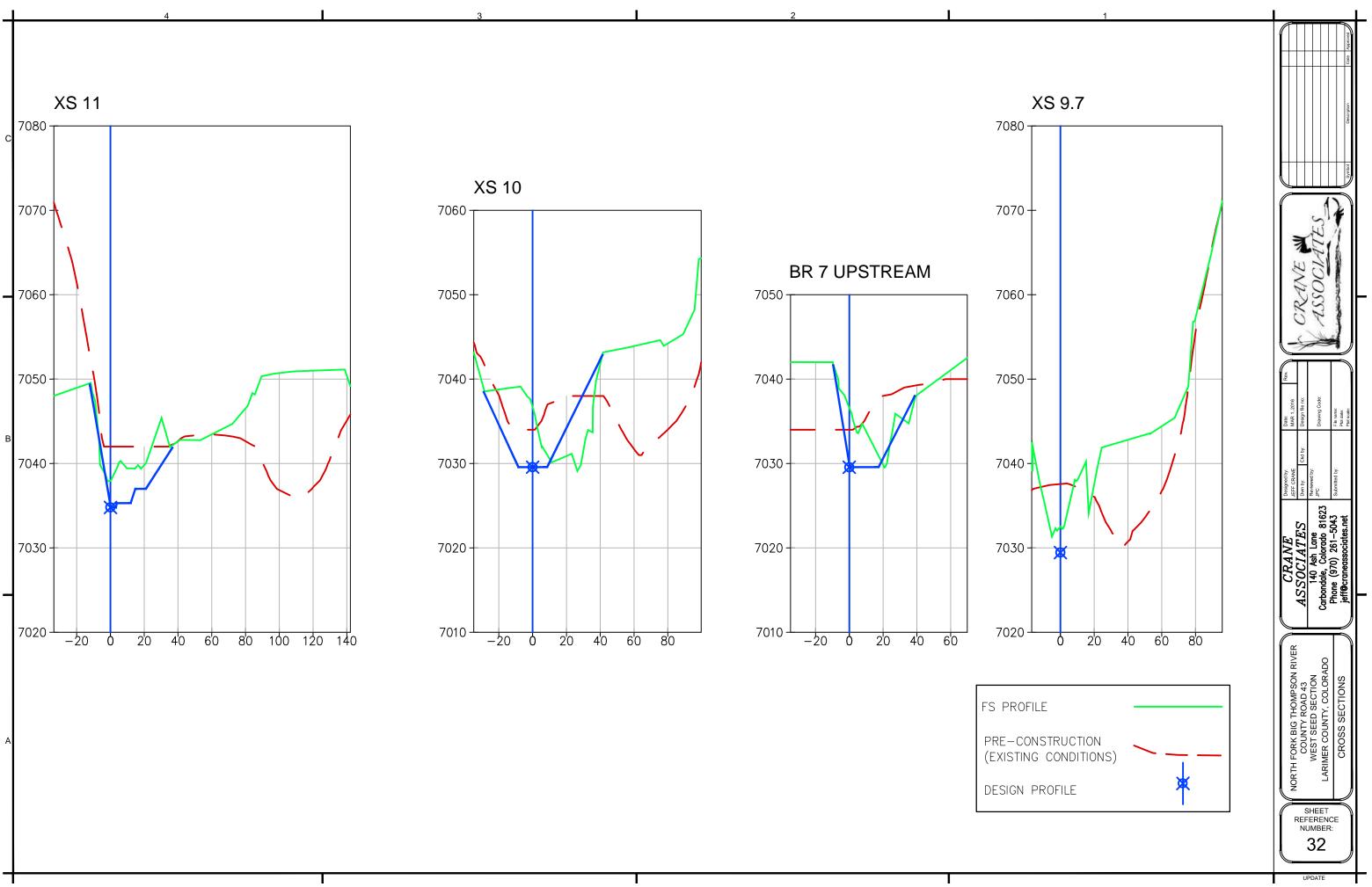
XS 14

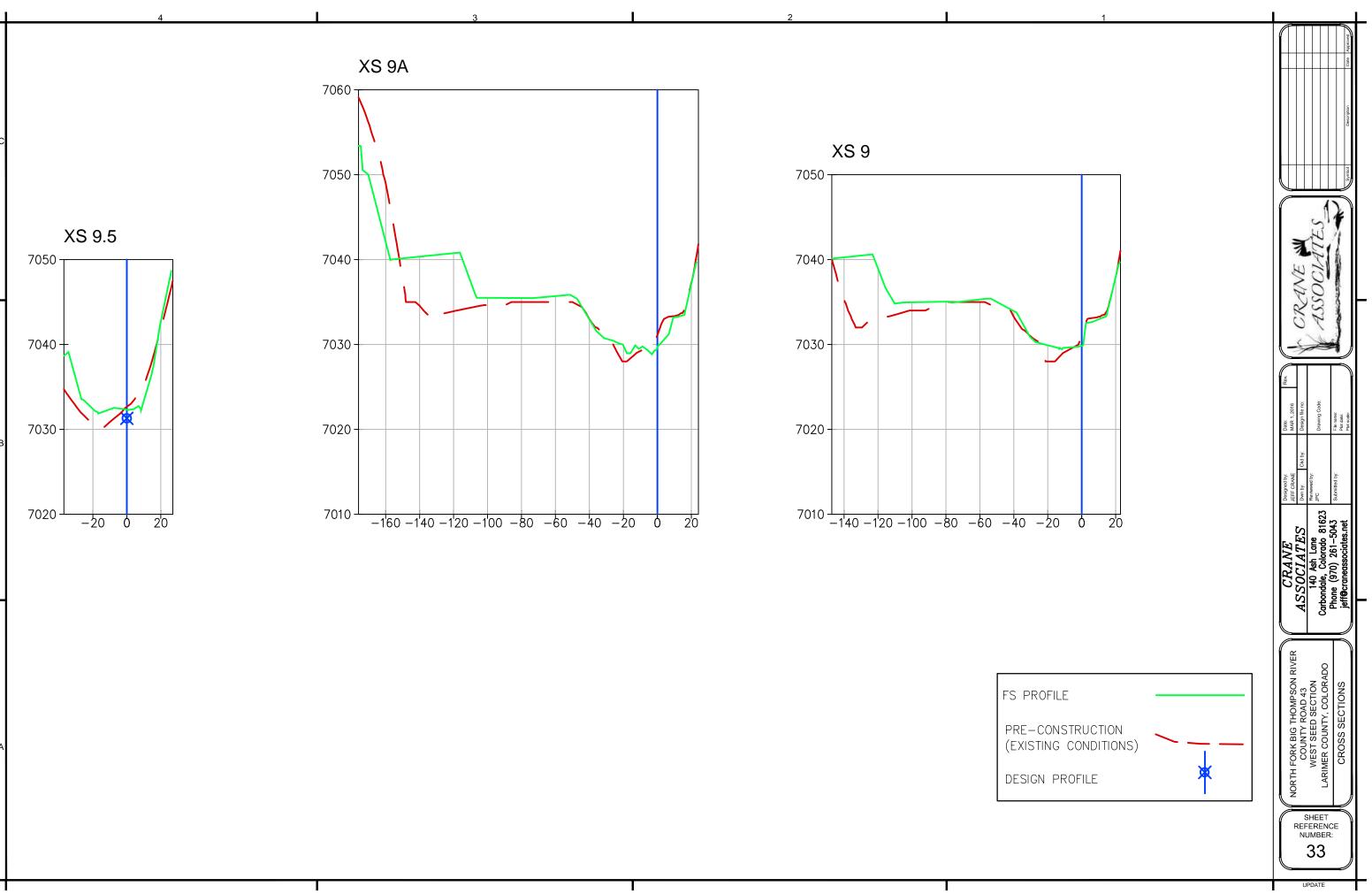


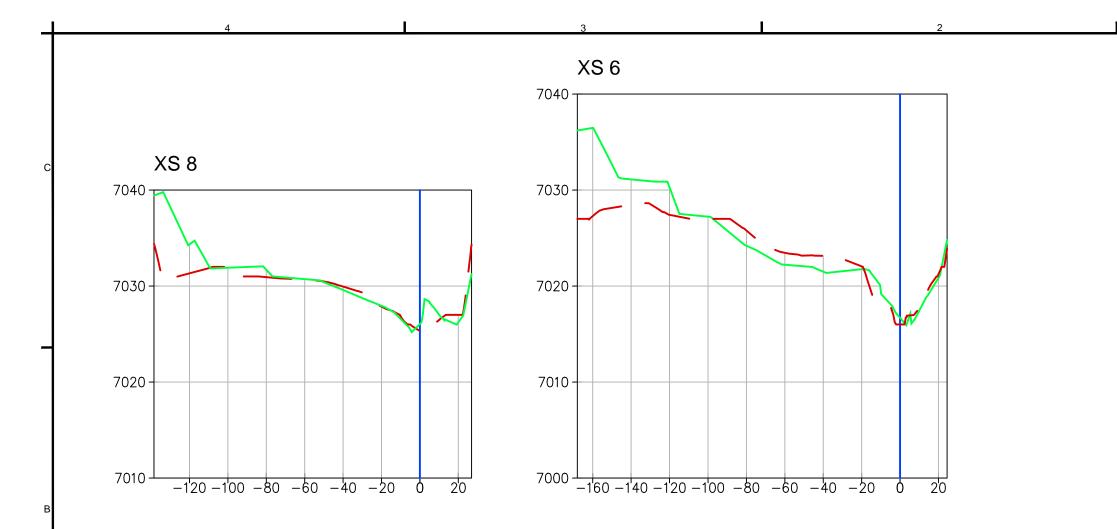


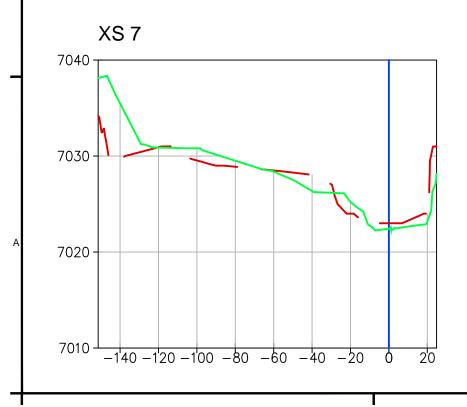


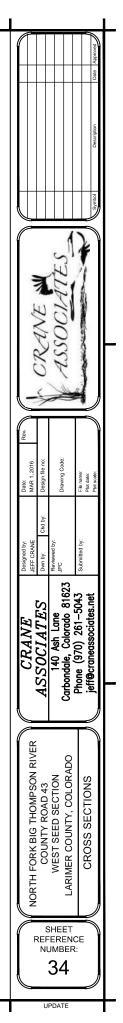


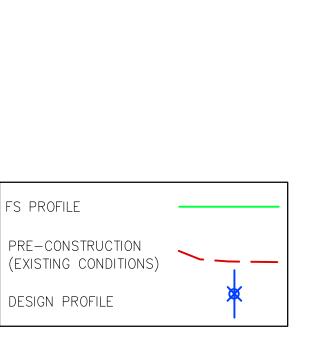


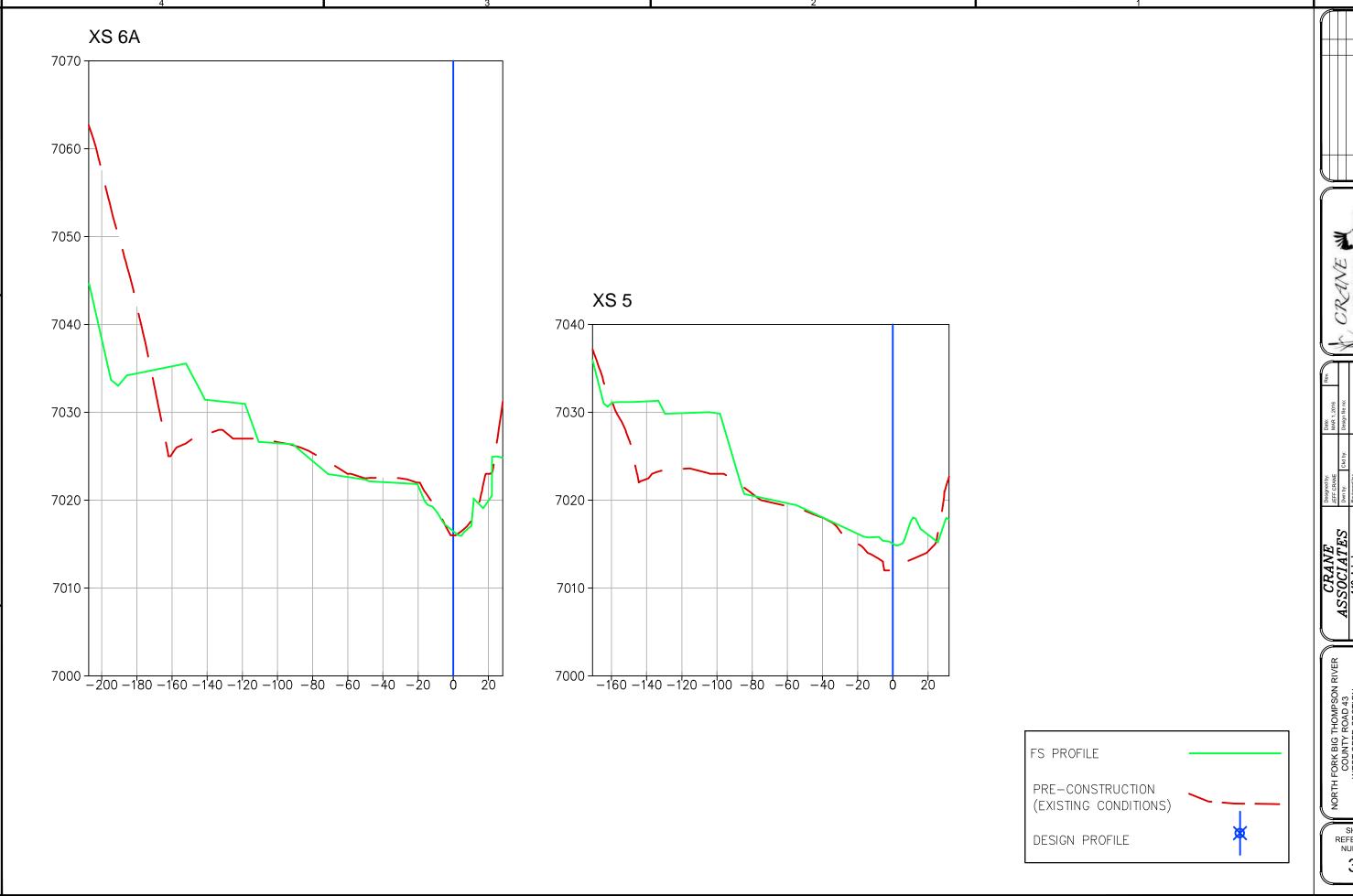


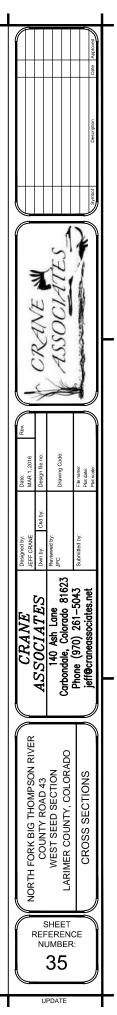


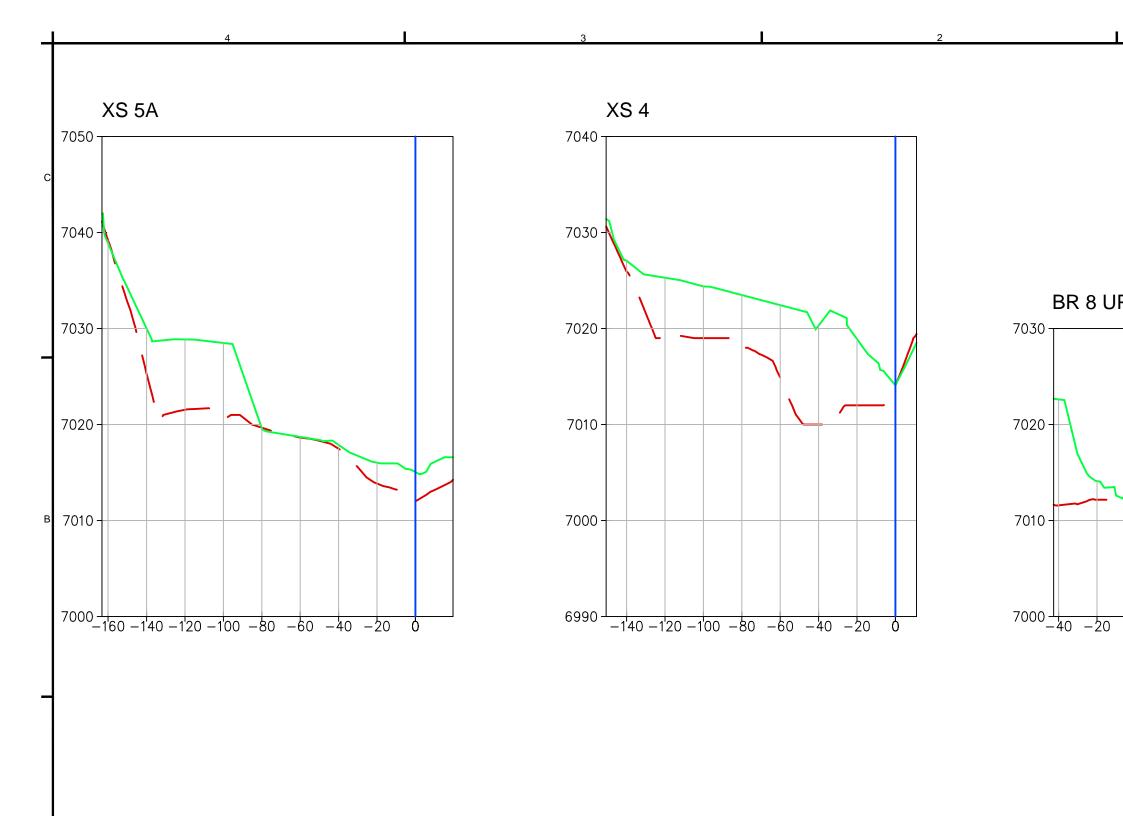




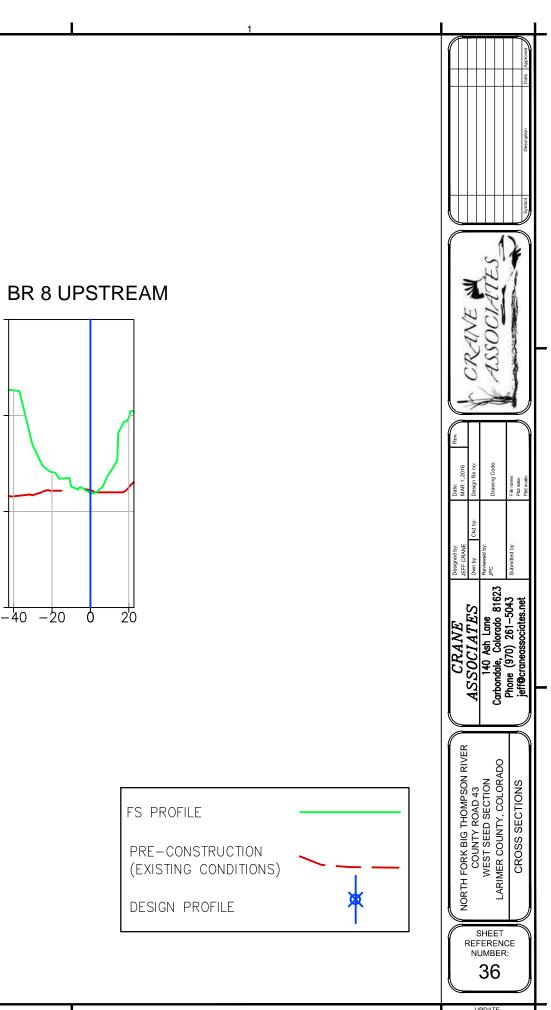




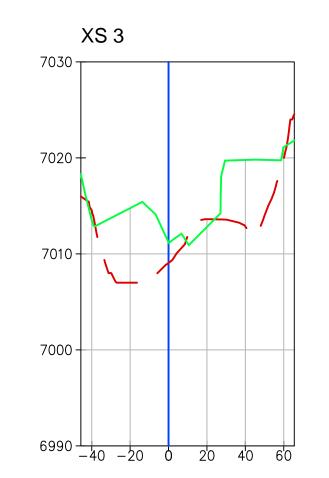




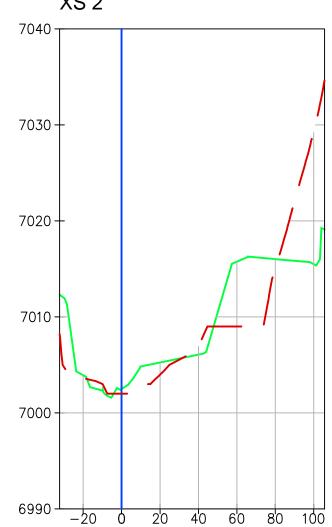
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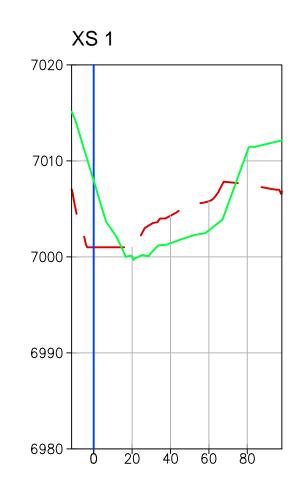


XS 2

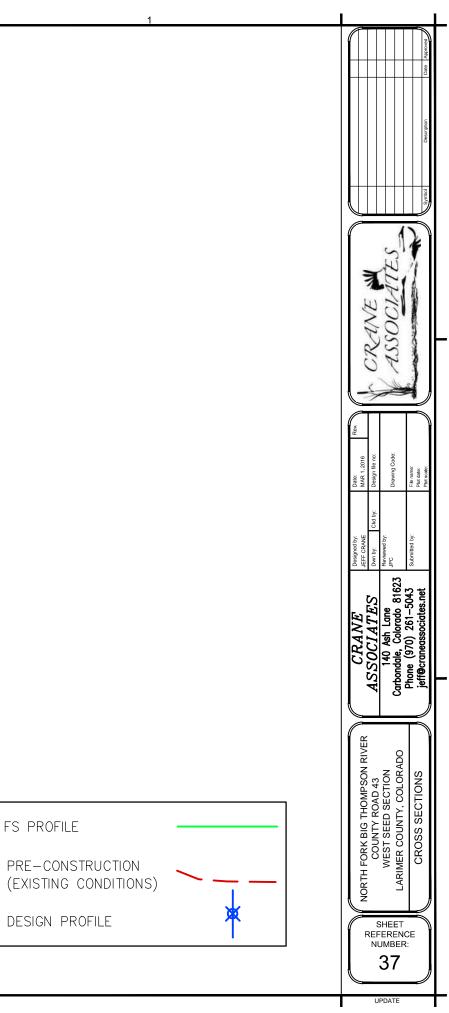


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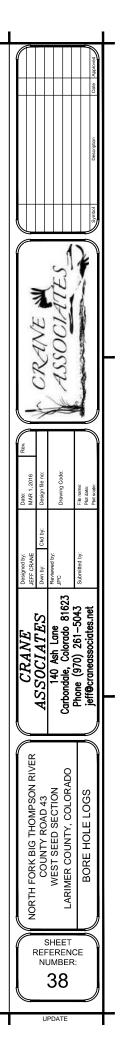


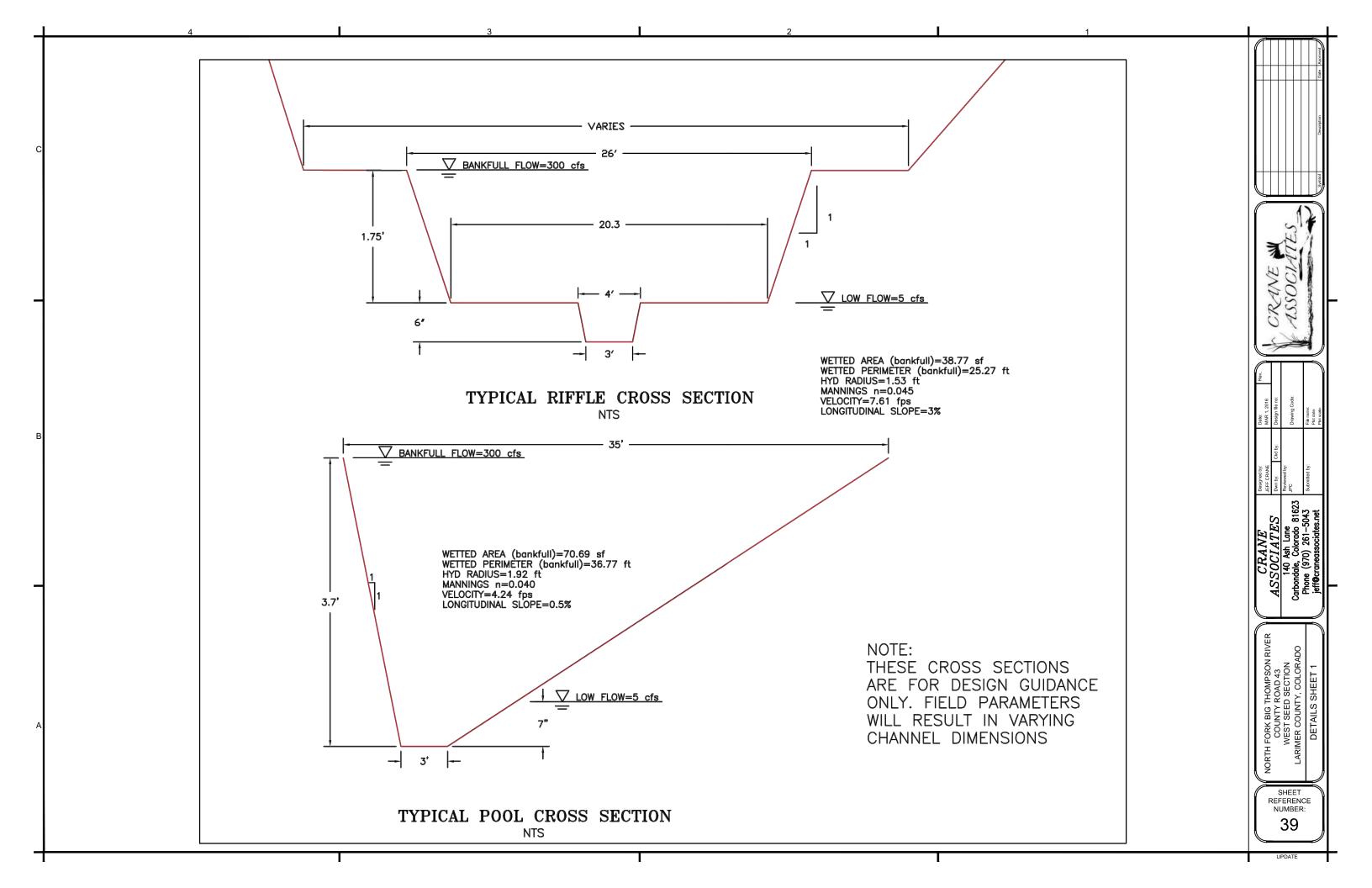


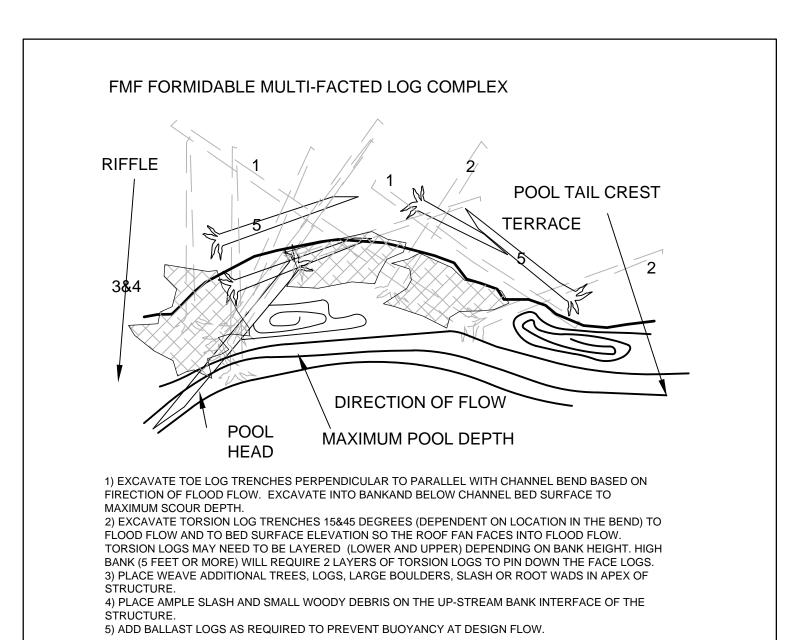
FS PROFILE



Bore Hole #	Bore Hole Log	Ground Elevations (ft)	Water Table Elevation (ft)	Bedrock Elevation (ft)	Boulder Top (ft)	Boulder Bottom (ft)
11	Water @ 10' Rock 13'-16'	7144.56	7134.56	7131.56	N/A	N/A
12	Boulder @ 5'-7' Rock 13'-16' Water @ 10	7144.59	7134.59	7131.59	7139.59	7137.59
13	Shot Rock @ 6'-16' Water @10'	7144.81	7134.81	Not Known	N/A	N/A
21	Rock @5-16', Water @10'	7146.7	7136.7	7141.7	N/A	N/A
22	Soft Through 16'	7146.12	Not Noted	N/A	N/A	N/A
23	Boulder @ 7'-10' Water@10' Soft 10'-16'	7145.73	7135.73	N/A	7138.73	7135.73
24	Water @10' Rest Soft	7146.15	7136.15	N/A	N/A	N/A
31	Rock @ 5-16' Water @10'	7148.13	7138.13	N/A	N/A	N/A
32	Water @5' Boulder @12'-14' Rest Soft	7147.24	7142.24	N/A	7135.24	7133.24
33	Water @5' All Soft	7147.05	7142.05	N/A	N/A	N/A
34	Boulder @3'-6' Water @6' Rest Soft	7147.14	7141.14	N/A	7144.14	7141.14
41	Rock @5-16', Water @10'	7148.73	7138.73	7143.73	N/A	N/A
42	Boulder @5'-7' Water @7' Rock 11'-16'	7147.72	7140.72	N/A	7142.72	7140.72
43	Water @5' All Soft	7148.26	7143.26	N/A	N/A	N/A
44	Water @10' All Soft	7147.69	7137.69	N/A	N/A	N/A
45	Boulder @ 6'-9' Water @ 9' Rest Soft	7147.74	7138.74	N/A	7141.74	7138.74
51	Rock/Water @ 5'-14', 14'-16' Soft	7148.42	7143.42	N/A	7138.42	7134.42
52	Water @ 5', Rock 12'-16'	7149.45	7144.45	7137.45	N/A	N/A
53	Water @ 5', Rest Soft	7148.54	7143.54	N/A	N/A	N/A
54	Water @ 6' Rest Soft	7148.39	7142.39	N/A	N/A	N/A
55	Water @ 6' Rest Soft	7148.33	7142.33	N/A	N/A	N/A
61	Water @ 5' Rock @ 6'-16'	7149.72	7144.72	7143.72	N/A	N/A
62	Water @ 5', Rest Soft	7150.19	7145.19	N/A	N/A	N/A
63	Water @5', Rest Soft	7149.19	7144.19	N/A	N/A	N/A
64	Water @5', Rest Soft	7148.93	7143.93	N/A	N/A	N/A
65	Water @5', Rest Soft	7149.13	7144.13	N/A	N/A	N/A
71	Water @7', Rock @ 7'-16'	7150.9	7143.9	7143.9	N/A	N/A
	Water @6', Rock @12'-16'	7150.94				N/A
	Water @5', Rest Soft	7149.76	7144.76		N/A	N/A
74	Water @5', Rest Soft	7149.57				N/A
75	Water @5', Rest Soft	7149.46				N/A







Component Riffle height above channel bed design Reach upstream bed elevation define Reach downstram bed elevation define **Reach length** measu Average width of flow model Reach channel slope calcula Slope of upstream riffle face design Slope of downstream riffle face design Distance of heel to crest calcula Distance of crest to toe calcula Height of bed at the crest above toe calcula Total drop calcula Pool length calcula Interval between crests calcula

Material Type A

Riffle and Low Bar Material shall have, at minimum, the following compisition: D16 20 mm D50 45 mm D84 100 mm D90 145 mm

Material Type B

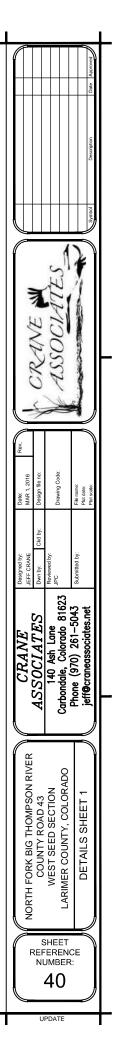
Riffle Crest Material shall									
have,	have, at minimum, the								
follow	following compisition:								
D16	D16 30 mm								
D50	mm								
D84 150 mm									
D90	D90 220 mm								

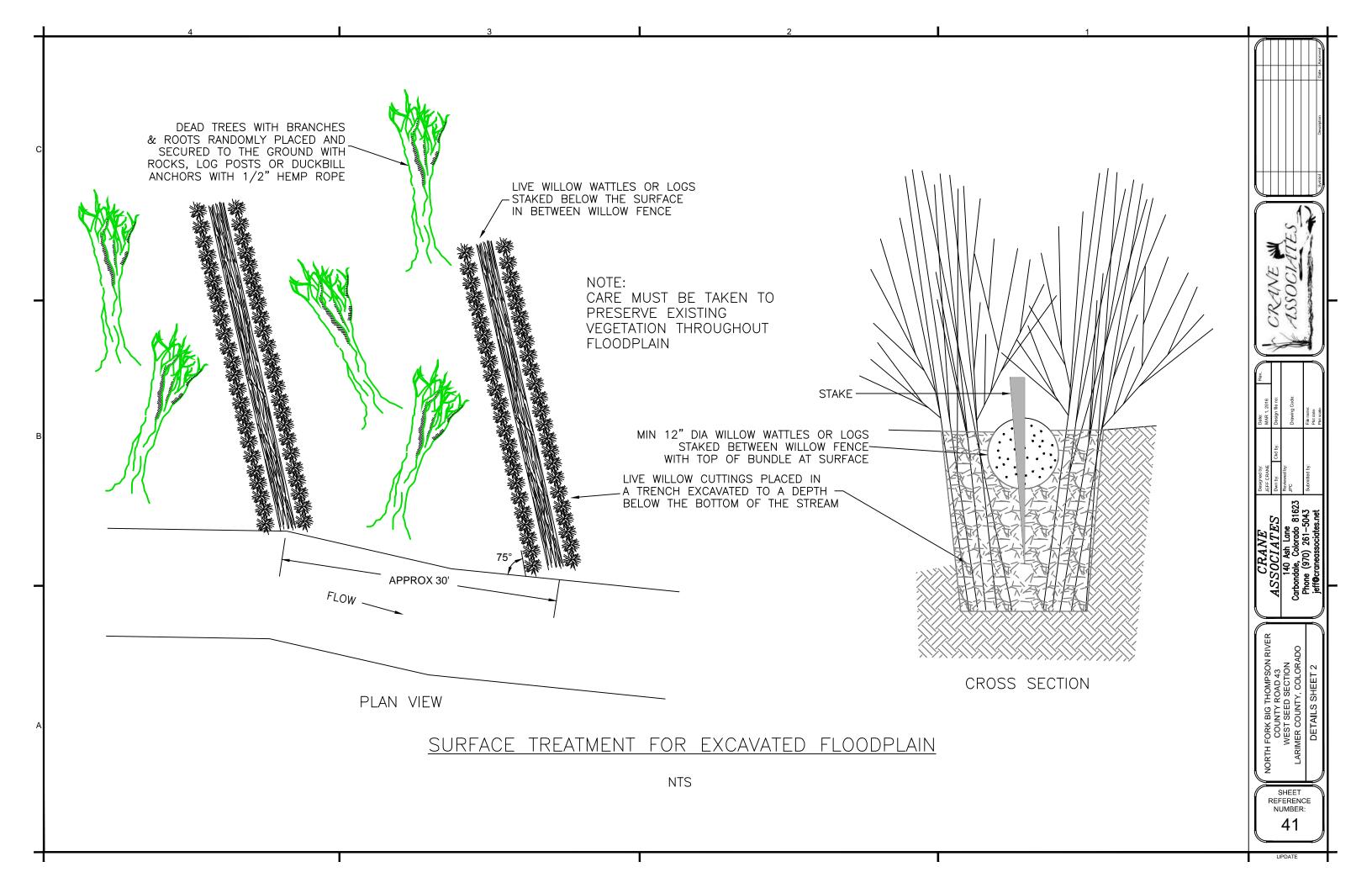
Material Type C

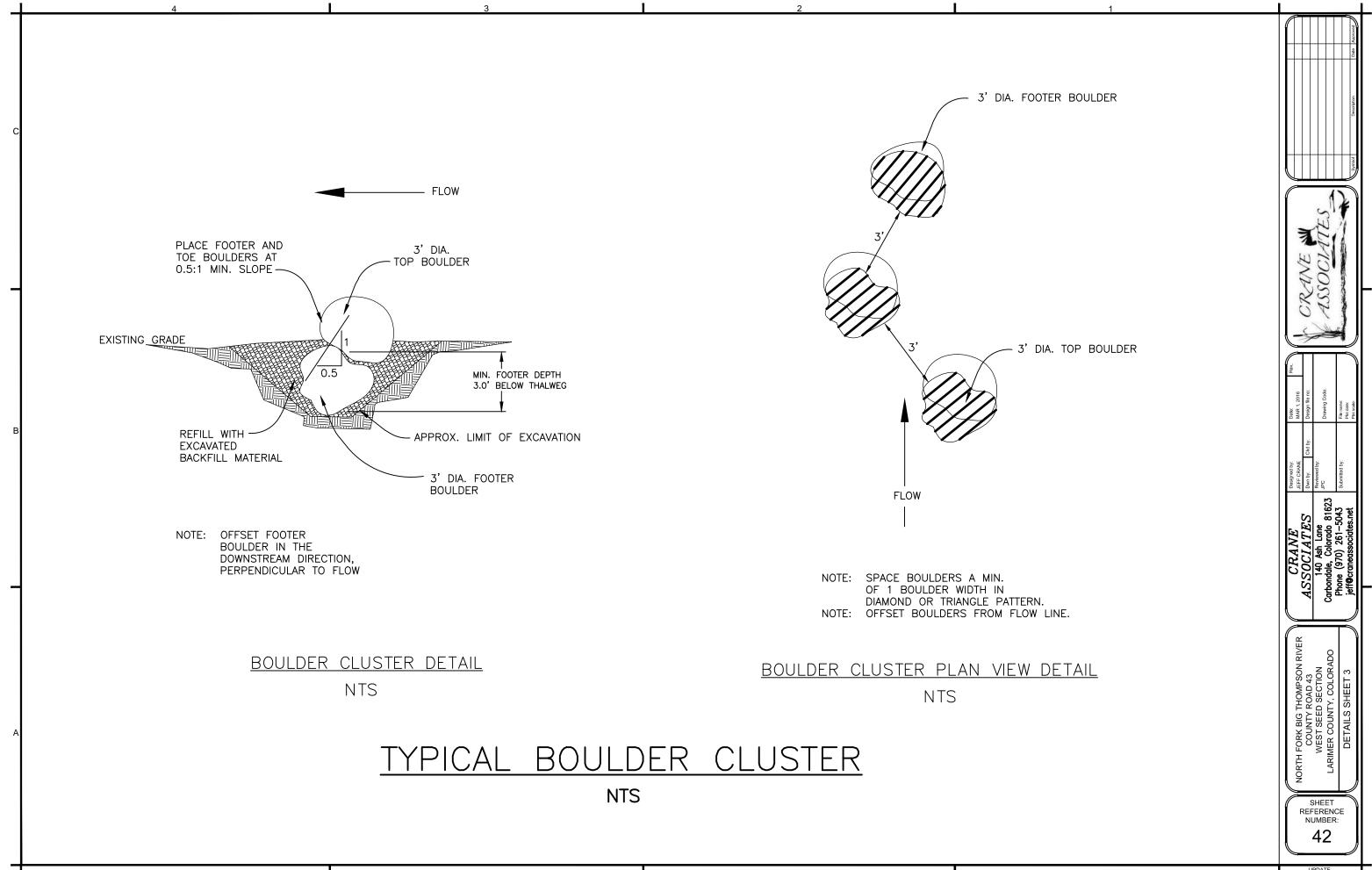
Step Pool Material shall have, at minimum, the following							
с	compisition:						
D16 30 mm							
D50	128	mm					
D84	mm						
D90 256+ mm							

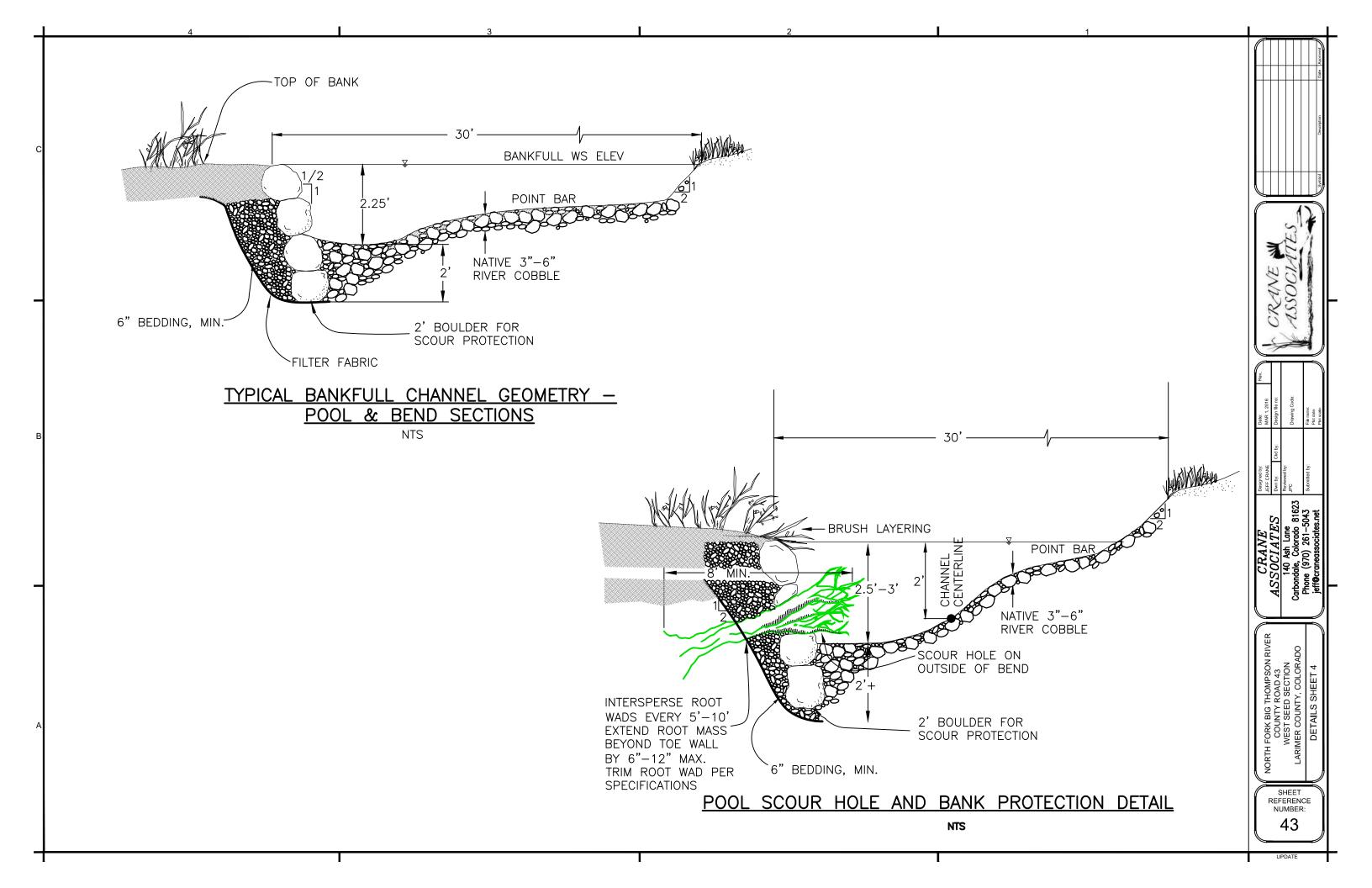
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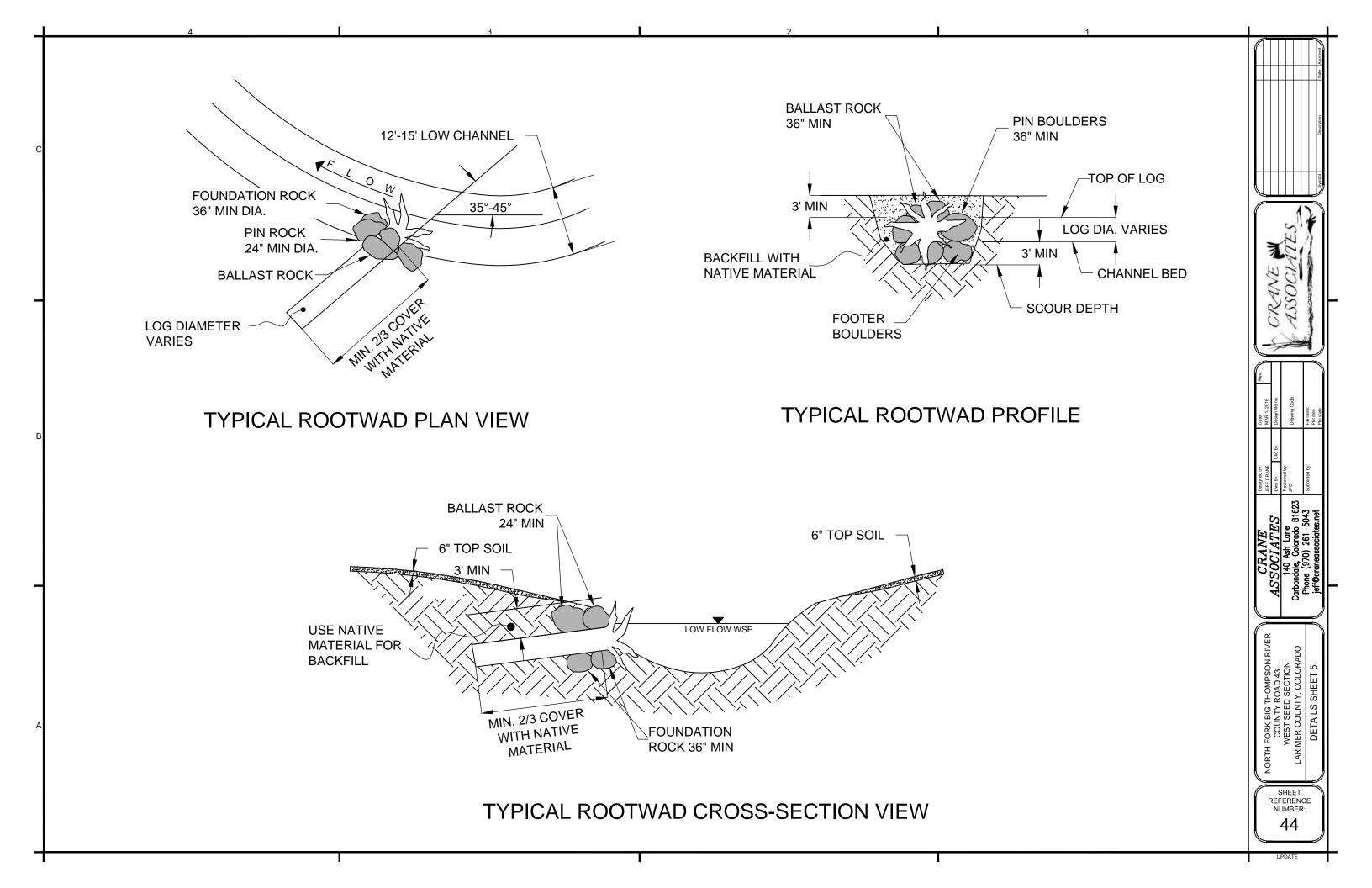
Reach 9	Riffle Characteristics		
	Description	Value	Unit
lbed	design	1	feet
tion	defined by bridge invert	7006.5	feet
ration	defined by bridge invert	6995.5	feet
	measured	575	feet
	model output/measured	30	feet
	calculated	0.02	ft/ft
e	design	0.3	ft/ft
face	design	0.05	ft/ft
	calculated	3	feet
	calculated	30	feet
bove toe	calculated	0.75	feet
	calculated	1.5	feet
	calculated	45	feet
	calculated	80	feet

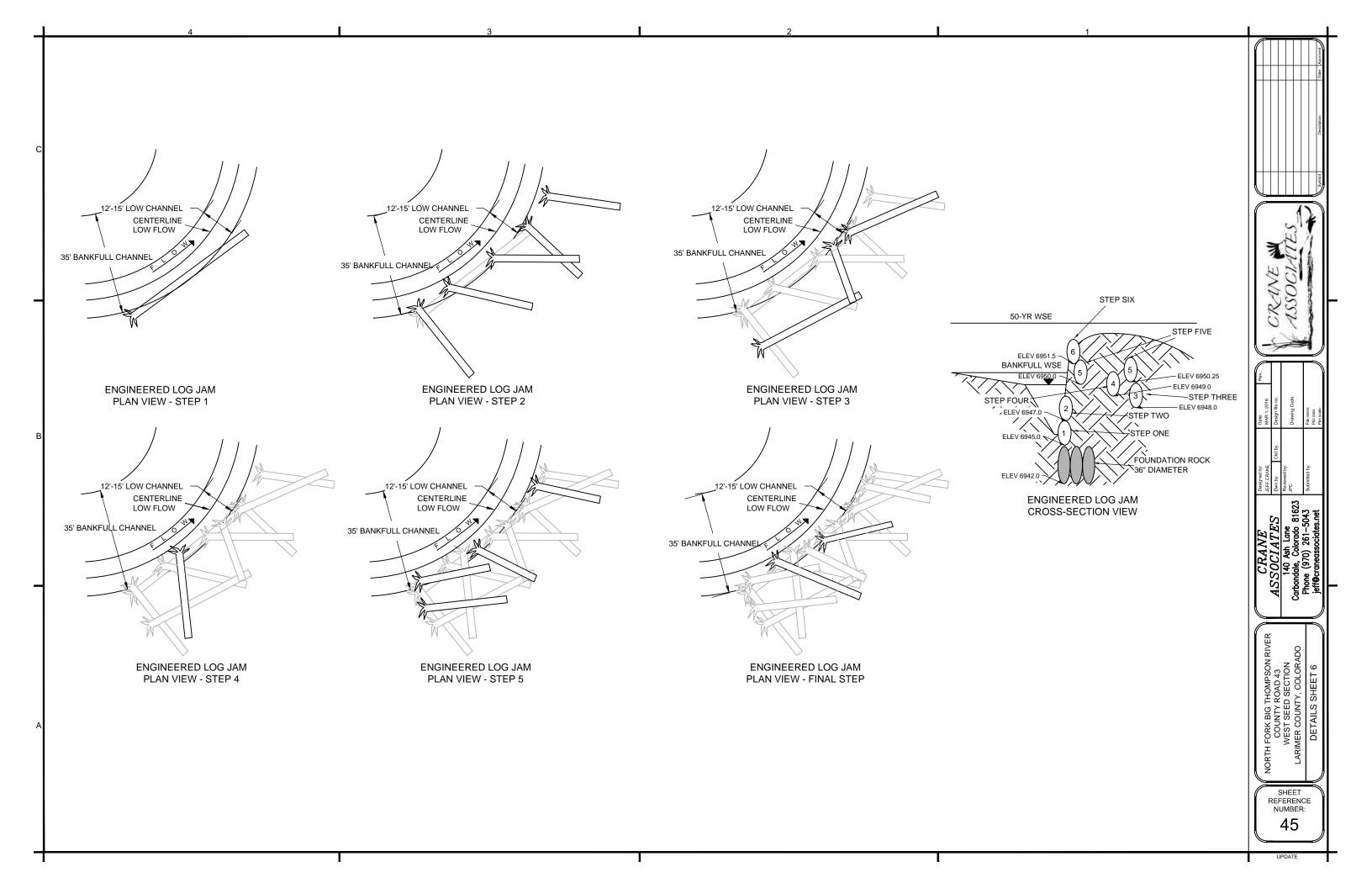












					Water							Water							Water	
Model River				Channel	Surface	EstimatedMa	Model River				Channel	Surface	EstimatedMa	Model River				Channel	Surface	EstimatedMa
Sta	Cross Section	Flow Profile	Flow	Invert	Elevation	x. Depth	Sta	Cross Section	Flow Profile	Flow	Invert	Elevation	x. Depth	Sta	Cross Section	Flow Profile	Flow	Invert	Elevation	x. Depth
(ft)	(ft)		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft)		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft)		(cfs)	(ft)	(ft)	(ft)
7673.817	USFS XS 29	Low Flow 30 cfs	30	7111.5	7113.2	1.6	S141.93	USFS XS 15	Low Flow 30 cfs	30	7055.0	7055.6	0.6	3007.753	2167+00	Low Flow 30 cfs	30	6999.5	7000.8	1.3
7673.817	USFS XS 29	StreamStats 2yr	300	7111.5	7115.4	3.9	S141.93	USFS XS 15	StreamStats 2yr	300	70SS.0	7057.2	2.2	3007.753	2167+00	StreamStats 2yr	300	6999.5	7002.6	3.0
7462.438	USFS XS 27	Low Flow 30 cfs	30	7101.7	7102.4	0.7	4669.68S	USFS XS12	Low Flow 30 cfs	30	7041.S	7043.0	1.5	2928.346	USFS XS1	Low Flow 30 cfs	30	6998.0	6999.0	1.0
7462.438	USFS XS 27	StreamStats 2yr	300	7101.7	710S.3	3.6	4669.68S	USFS XS12	StreamStats 2yr	300	7041.5	7045.2	3.7	2928.346	USFS XS1	StreamStats 2yr	300	6998.0	7001.S	3.5
7262.389	USFS XS26	Low Flow 30 cfs	30	7099.0	7101.7	2.7	4420.553	USFS XS11	Low Flow 30 cfs	30	7040.0	7042.2	2.2	2856.611	XS 2168+S0	Low Flow 30 cfs	30	6995.3	6997.S	2.1
7262.389	USFS XS26	StreamStats 2yr	300	7099.0	7104.2	5.2	4420.553	USFS XS11	StreamStats 2yr	300	7040.0	7043.1	3.1	2856.611	XS 2168+50	StreamStats 2yr	300	6995.3	7000.7	S.4
7115.395	USFS XS2S	Low Flow 30 cfs	30	7095.5	7097.4	1.9	4063.026	USFS XS 9	Low Flow 30 cfs	30	7028.0	7029.2	1.2	2583.848	XS 2171+00	Low Flow 30 cfs	30	6993.0	6994.4	1.3
7115.395	USFS XS2S	StreamStats 2yr	300	7095.5	7099.7	4.2	4063.026	USFS XS 9	StreamStats 2yr	300	7028.0	7031.0	3.0	2583.848	XS 2171+00	StreamStats 2yr	300	6993.0	6995.9	2.9
6968.628	USFS XS 24	Low Flow 30 cfs	30	7091.5	7092.4	0.9	3938.423	USFS XS8	Low Flow 30 cfs	30	7026.0	7026.9	0.9	2356.821	XS 2171+S0	Low Flow 30 cfs	30	6988.0	6988.9	0.9
6968.628	USFS XS 24	StreamStats 2yr	300	7091.5	7094.S	3.0	3938.423	USFS XS8	StreamStats 2yr	300	7026.0	7028.1	2.1	2356.821	XS 2171+S0	StreamStats 2yr	300	6988.0	6990.8	2.8
6861.255	USFS XS 23	Low Flow 30 cfs	30	7087.8	7090.2	2.4	3819.788	USFS XS7	Low Flow 30 cfs	30	7023.0	7023.4	0.4	1513.956	2174+S0	Low Flow 30 cfs	30	6969.3	6970.1	0.7
6861.255	USFS XS 23	StreamStats 2yr	300	7087.8	7093.2	5.4	3819.788	USFS XS7	StreamStats 2yr	300	7023.0	7024.6	1.6	1513.956	2174+50	StreamStats 2yr	300	6969.3	6971.S	2.1
6643.565	USFS XS 22	Low Flow 30 cfs	30	7088.0	7089.1	1.1	3649.238	USFS XS6	Low Flow 30 cfs	30	7016.0	7017.3	1.3	1227.34	2177+50	Low Flow 30 cfs	30	6961.0	6962.2	1.2
6643.565	USFS XS 22	StreamStats 2yr	300	7088.0	7090.9	2.9	3649.238	USFS XS6	StreamStats 2yr	300	7016.0	7019.6	3.6	1227.34	2177+50	StreamStats 2yr	300	6961.0	6963.9	2.9
6314.975	USFS XS 21	Low Flow 30 cfs	30	7076.0	7077.6	1.6	3469.493	USFS XS5	Low Flow 30 cfs	30	7011.0	7012.4	1.4	950.0442	XS 2180+50	Low Flow 30 cfs	30	6957.3	6958.0	0.7
6314.975	USFS XS 21	StreamStats 2yr	300	7076.0	7079.8	3.8	3469.493	USFS XS5	StreamStats 2yr	300	7011.0	7014.1	3.1	950.0442	XS 2180+S0	StreamStats 2yr	300	6957.3	6959.S	2.2
5945.151	USFS XS20	Low Flow 30 cfs	30	7071.0	7072.7	1.7	3425.121	2163+50	Low Flow 30 cfs	30	7011.3	7012.1	0.8	666.6795	XS 2183+50	Low Flow 30 cfs	30	6951.3	6952.9	1.6
5945.151	USFS XS20	StreamStats 2yr	300	7071.0	7075.2	4.2	3425.121	2163+50	StreamStats 2yr	300	7011.3	7013.5	2.2	666.6795	XS 2183+S0	StreamStats 2yr	300	6951.3	6954.9	3.6
5660.946	USFS XS19	Low Flow 30 cfs	30	7068.0	7069.0	1.0	3344.161	USFS XS4	Low Flow 30 cfs	30	7007.8	7008.6	0.8	369.5939	XS 2186+S0	Low Flow 30 cfs	30	6948.5	6949.S	1.0
5660.946	USFS XS19	StreamStats 2yr	300	7068.0	7070.4	2.4	3344.161	USFS XS4	StreamStats 2yr	300	7007.8	7010.3	2.5	369.5939	XS 2186+S0	StreamStats 2yr	300	6948.5	6950.8	2.3
5533.594	USFS XS18	Low Flow 30 cfs	30	7064.5	7066.0	1.5	3184.341	USFS XS3	Low Flow 30 cfs	30	7003.0	7004.3	1.3							
5533.594	USFS XS18	StreamStats 2yr	300	7064.5	7068.0	3.5	3184.341	USFS XS3	StreamStats 2yr	300	7003.0	7006.4	3.4							
5305.82	USFS XS 16	Low Flow 30 cfs	30	7060.3	7061.4	1.1	3038.00S	USFS XS2	Low Flow 30 cfs	30	7000.0	7001.2	1.2							
5305.82	USFS XS 16	StreamStats 2yr	300	7060.3	7063.3	3.0	3038.005	USFS XS2	StreamStats 2yr	300	7000.0	7003.2	3.1							

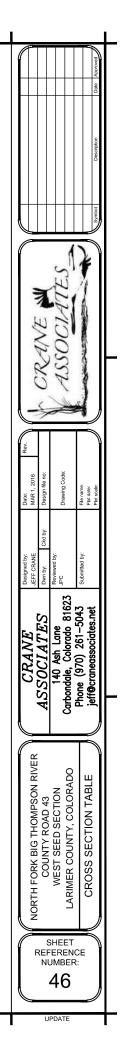
Site 5 Riffle Characteristics (3.0%)						
Component	Description	Value	Unit			
Riffle height above channel bed	design	2	feet			
Reach upstream bed elevation	defined by bridge invert	7133.05	feet			
Reach downstram bed elevation	defined by bridge invert	7090.62	feet			
Reach length	measured	1300	feet			
Average width of flow	model output/measured	30	feet			
Reach channel slope	calculated	0.03	ft/ft			
Slope of upstream riffle face	design	0.3	ft/ft			
Slope of downstream riffle face	design	0.05	ft/ft			
Distance of heel to crest	calculated	6	feet			
Distance of crest to toe	calculated	115	feet			
Height of bed at the crest above toe	calculated	3.5	feet			
Total drop	calculated	5.5	feet			
Pool length	calculated	60	feet			
Interval between crests	calculated	113	feet			

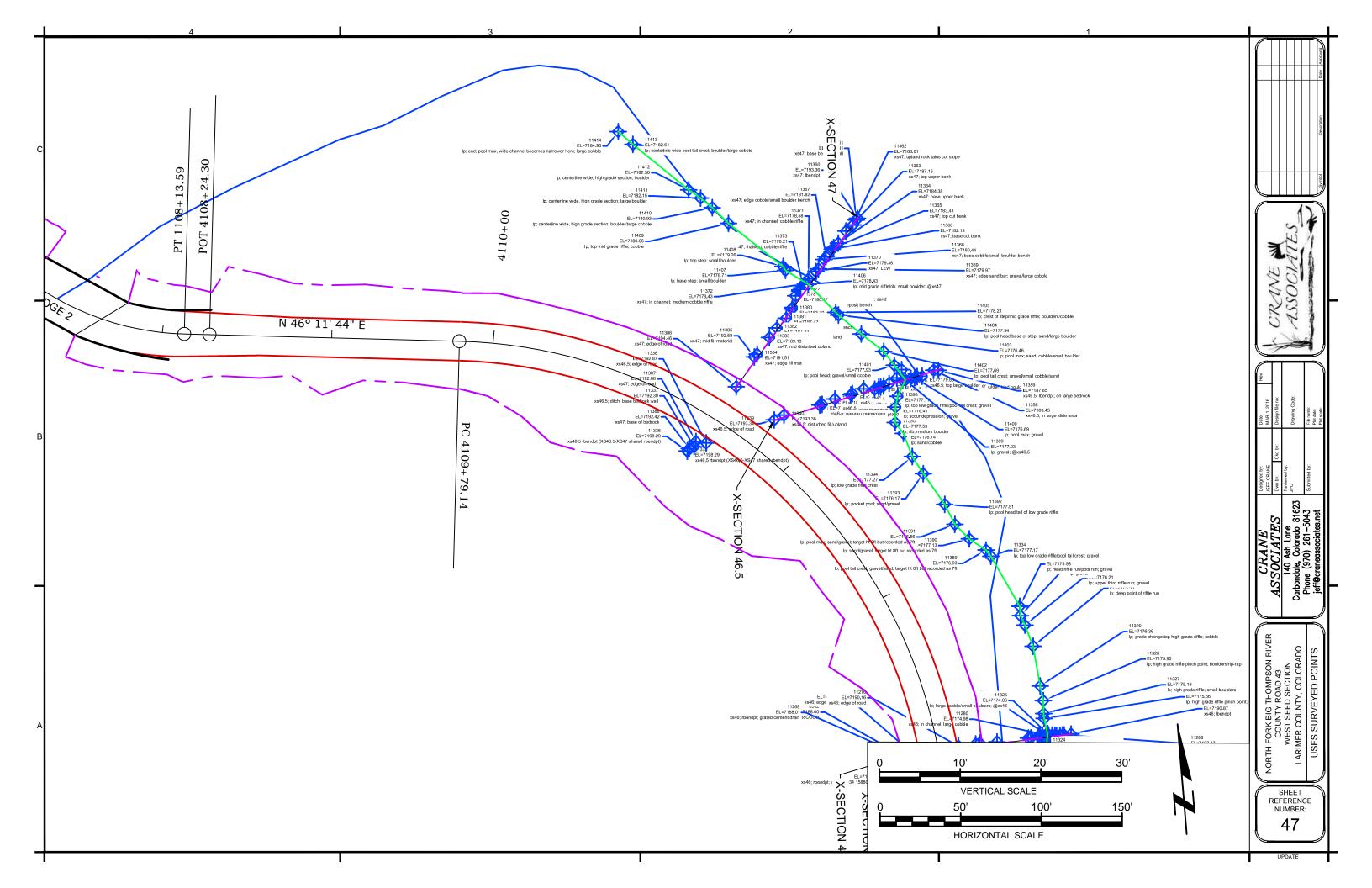
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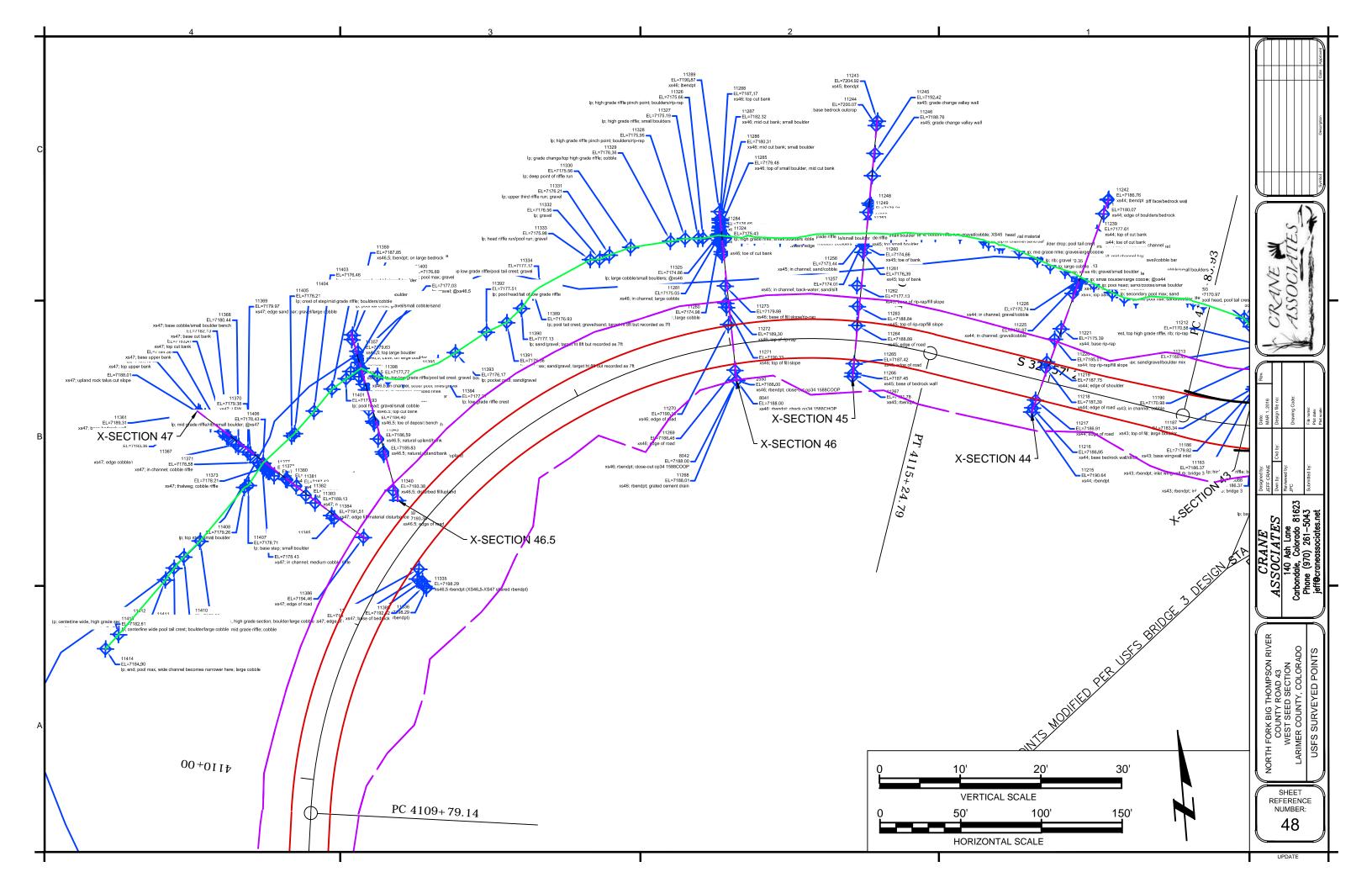
5ite 6 Riffle	5ite 6 Riffle Characteristics (2.0%)						
Component	Description	Value	Unit				
Riffle height above channel bed	design	1.5	feet				
Reach upstream bed elevation	defined by bridge invert	7090.48	feet				
Reach downstram bed elevation	defined by bridge invert	7056.3	feet				
Reach length	measured	1700	feet				
Average width of flow	model output/measured	25	feet				
Reach channel slope	calculated	0.02	ft/ft				
Slope of upstream riffle face	design	0.3	ft/ft				
Slope of downstream riffle face	design	0.05	ft/ft				
Distance of heel to crest	calculated	5	feet				
Distance of crest to toe	calculated	54	feet				
Height of bed at the crest above toe	calculated	1	feet				
Total drop	calculated	2.5	feet				
Pool length	calculated	80	feet				
Interval between crests	calculated	133	feet				

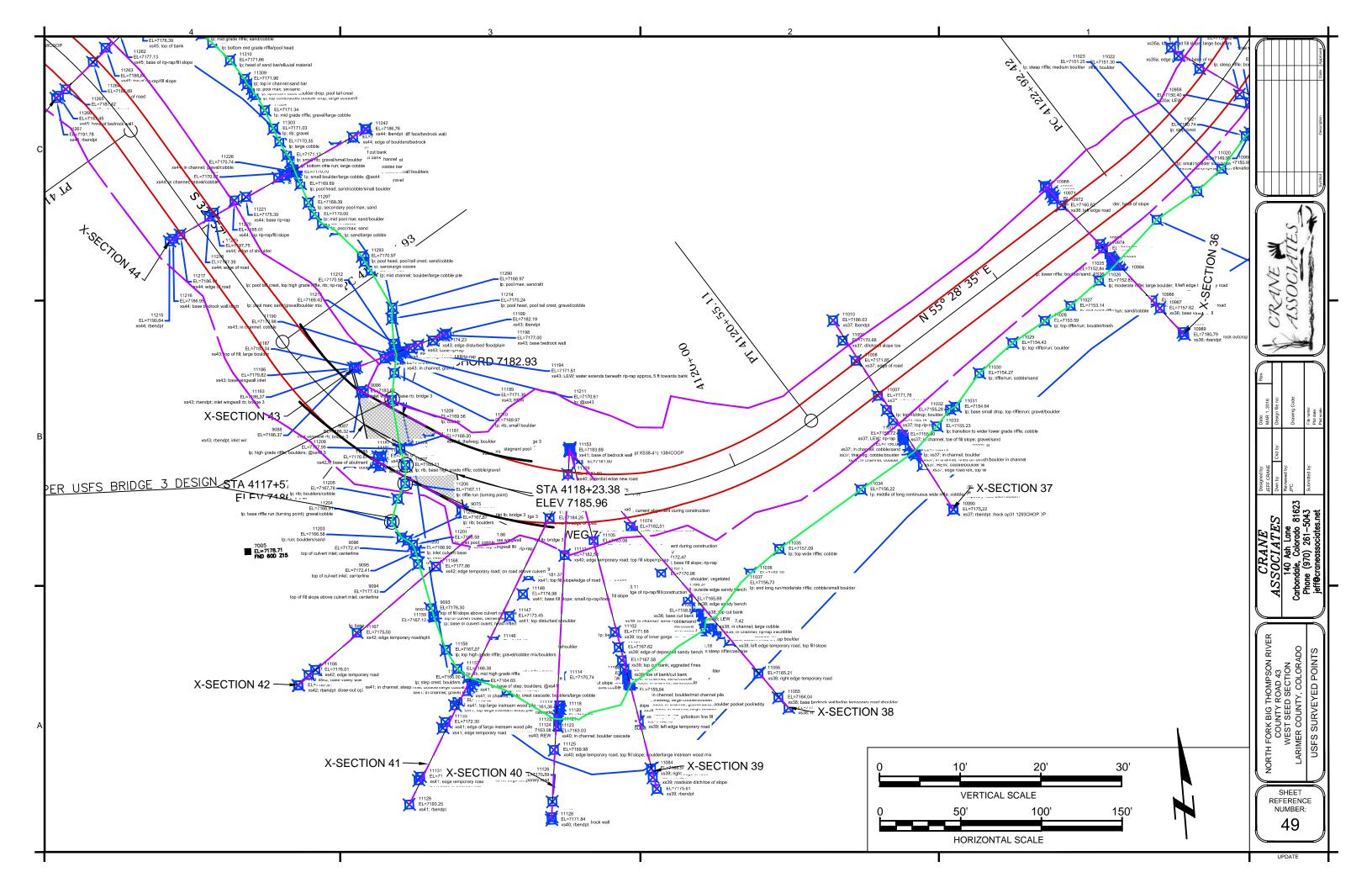
Component Riffle height above chanr Reach upstream bed elev Reach downstram bed elev Reach length Average width of flow Reach channel slope Slope of upstream riffle f Slope of downstream riffle Distance of heel to crest Distance of crest to toe Height of bed at the crest Total drop Pool length Interval between crests

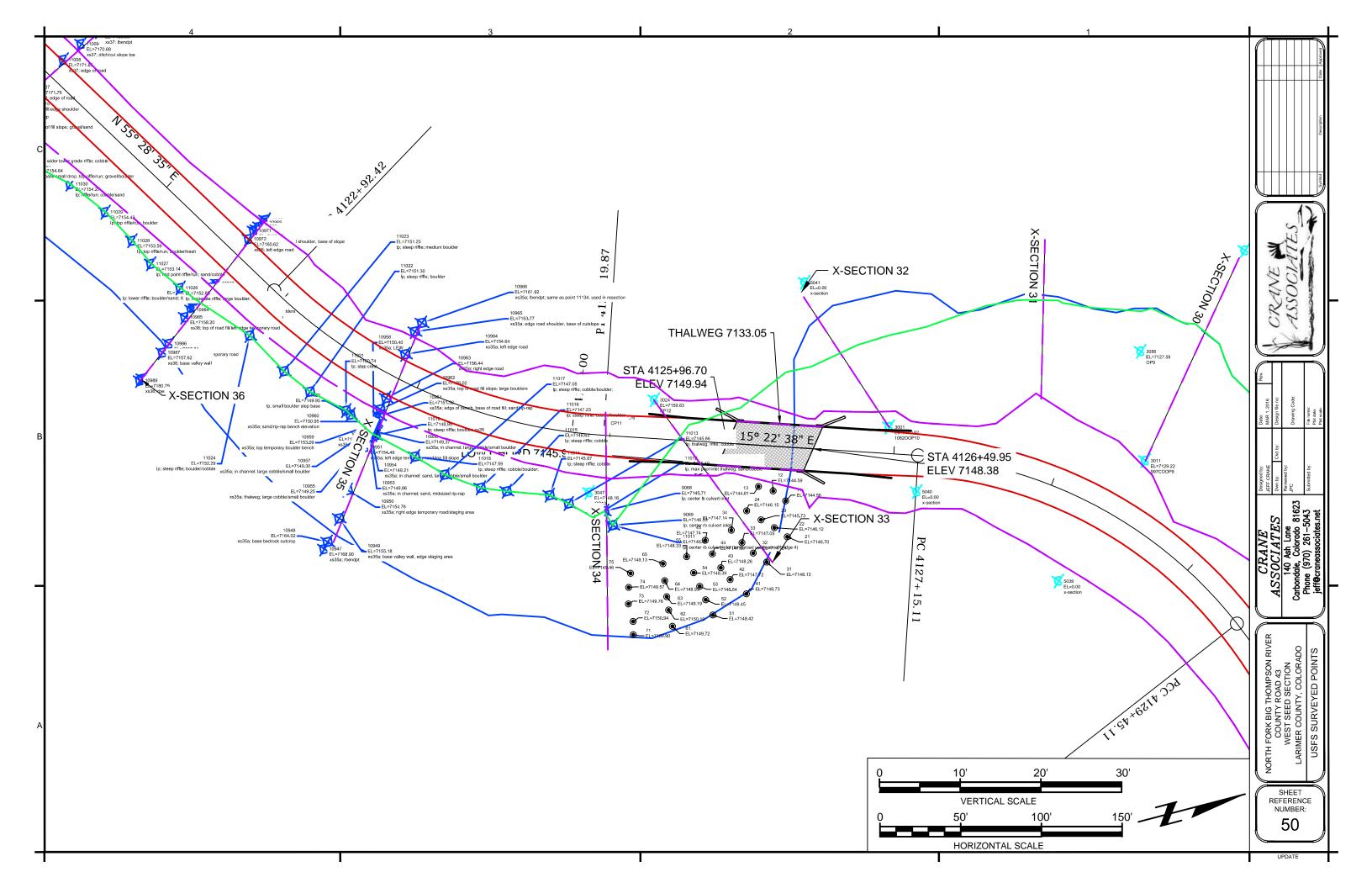
Site 8 Riffle	e Characteristics (2.5%)		
t	Description	Value	Unit
nnel bed	design	1.5	feet
evation	defined by bridge invert	7006.4	feet
elevation	defined by bridge invert	7029.95	feet
	measured	1000	feet
	model output/measured	32	feet
	calculated	0.025	ft/ft
e face	design	0.3	ft/ft
ffle face	design	0.05	ft/ft
t	calculated	4.5	feet
	calculated	55	feet
st above toe	calculated	1.25	feet
	calculated	2.75	feet
	calculated	60	feet
5	calculated	70	feet

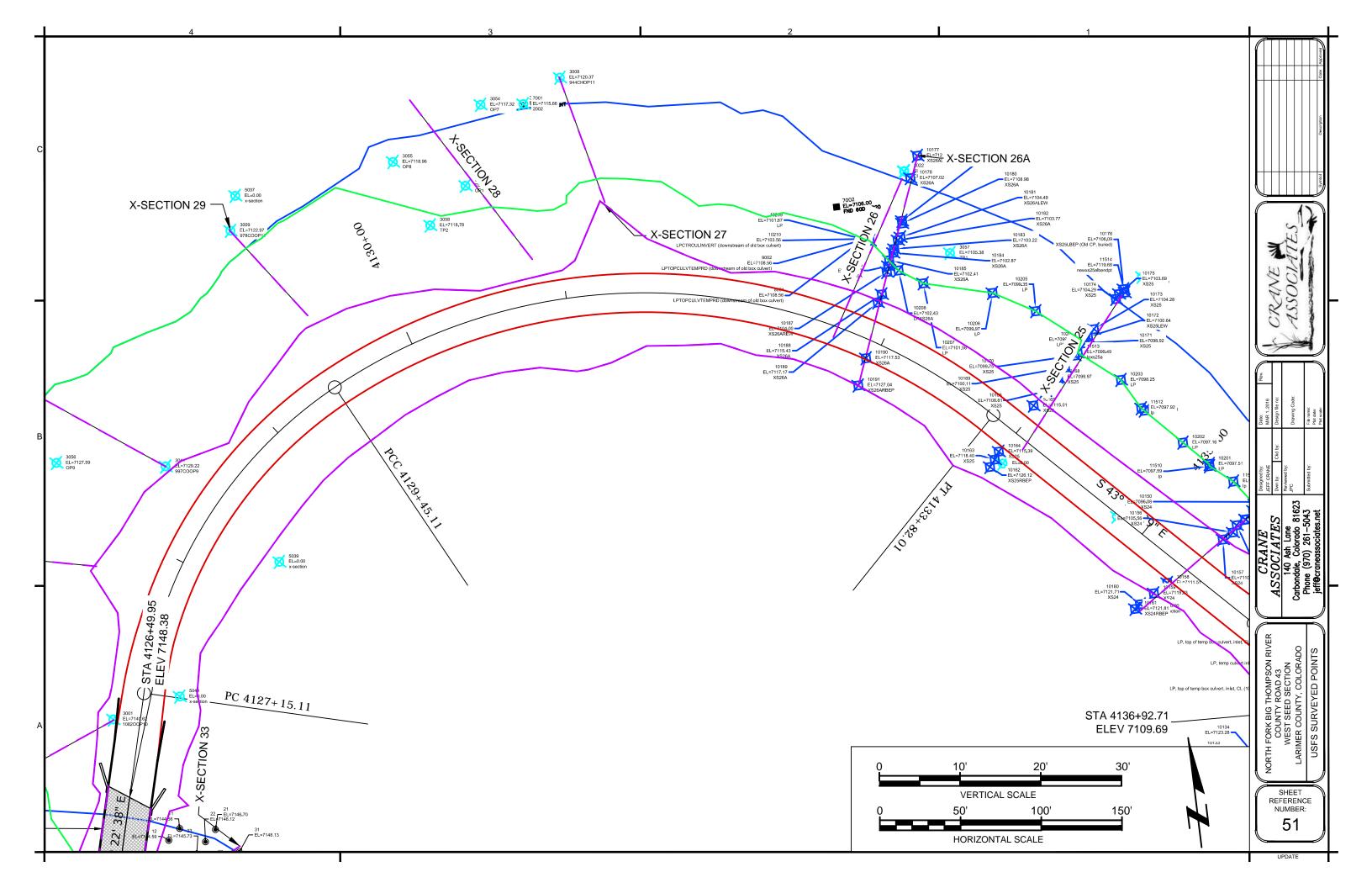


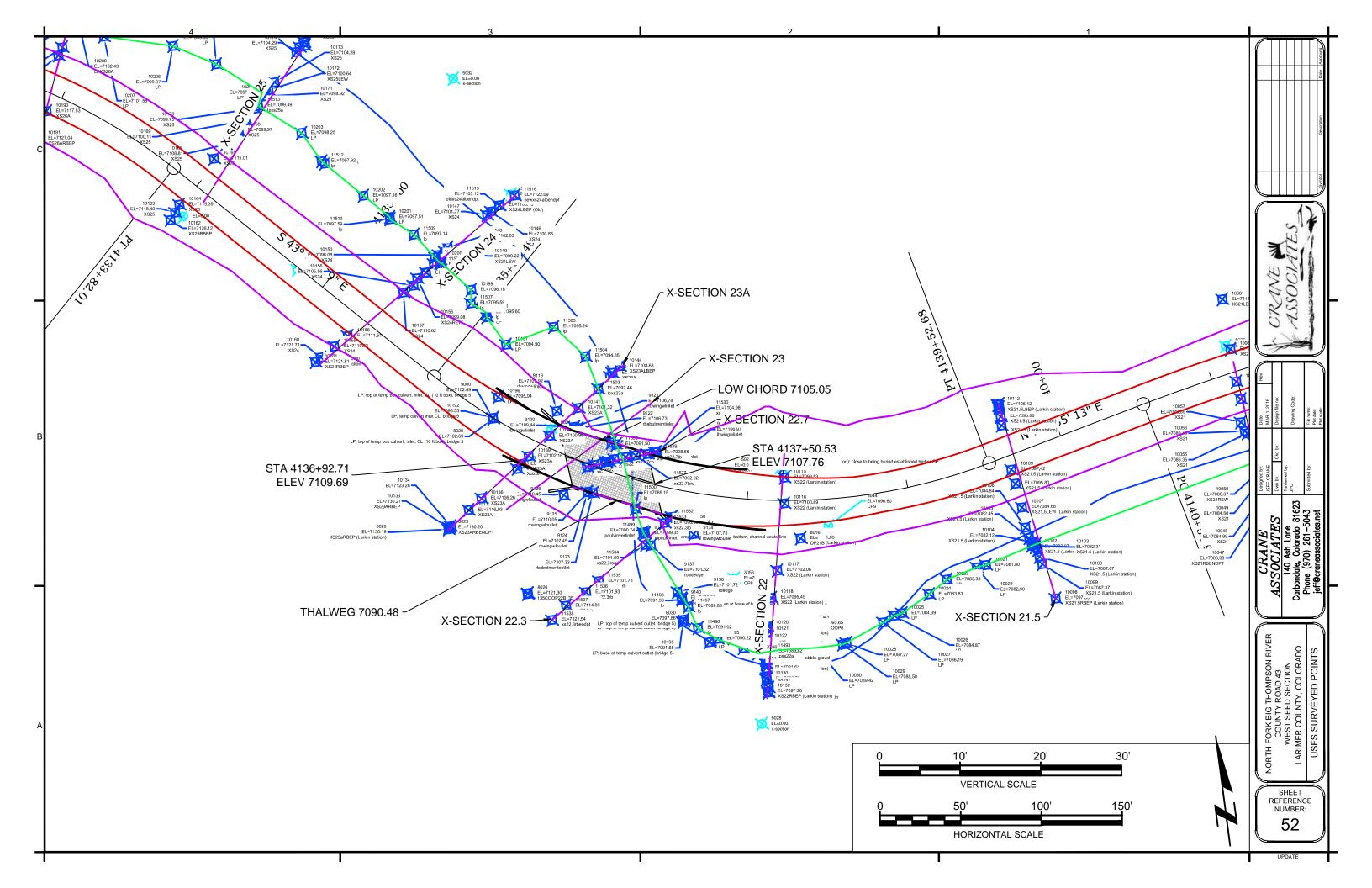


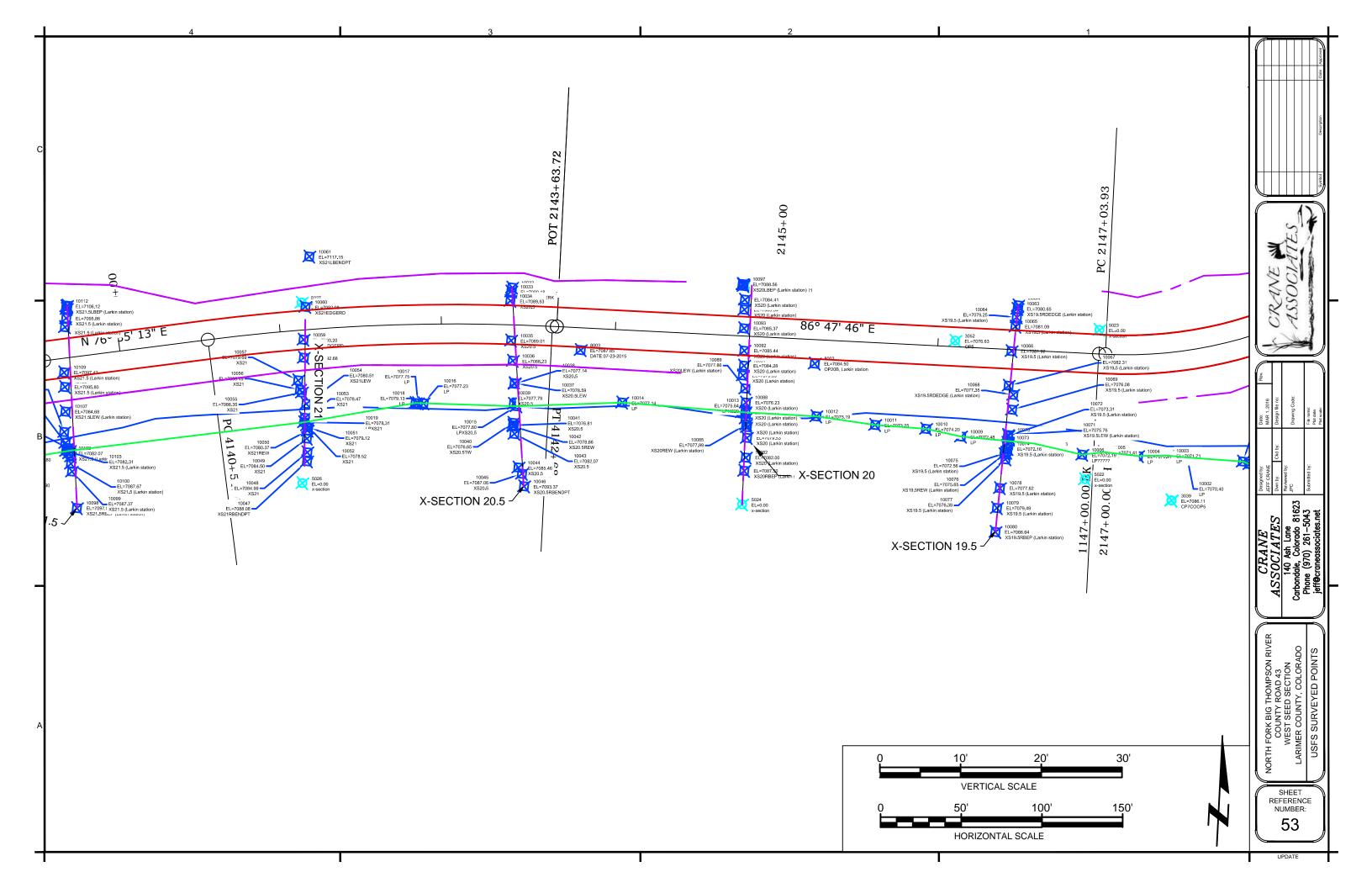


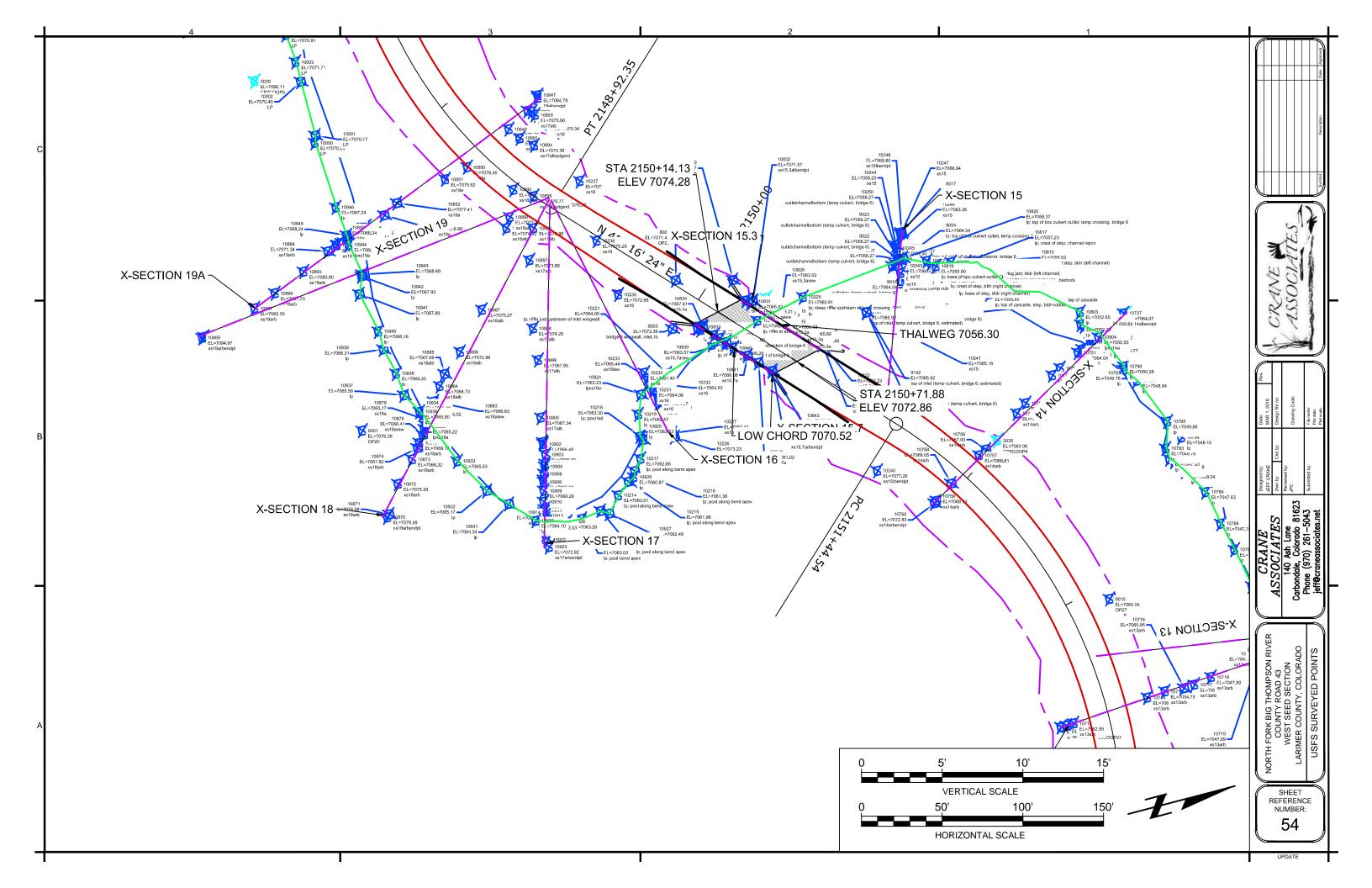


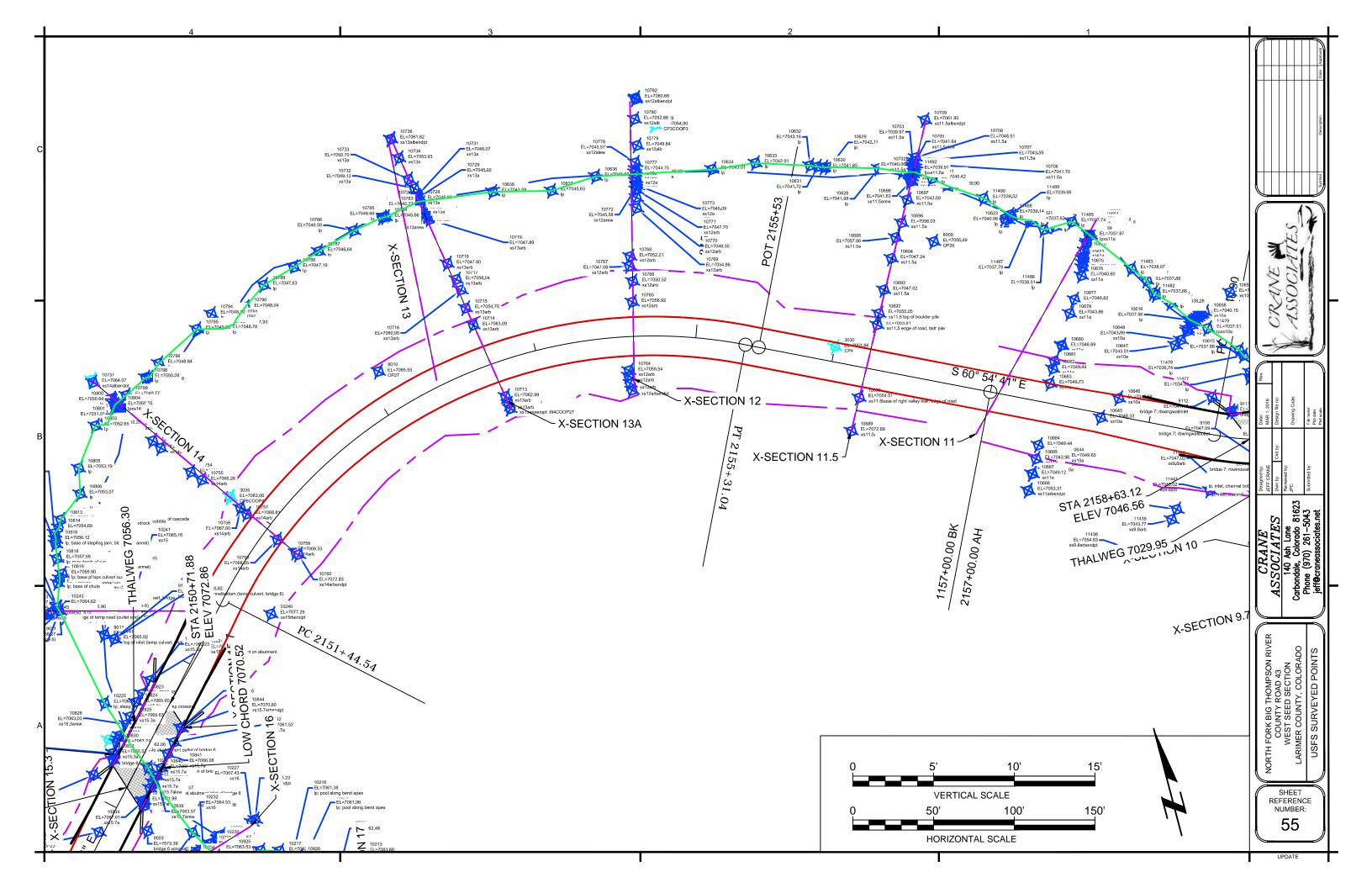


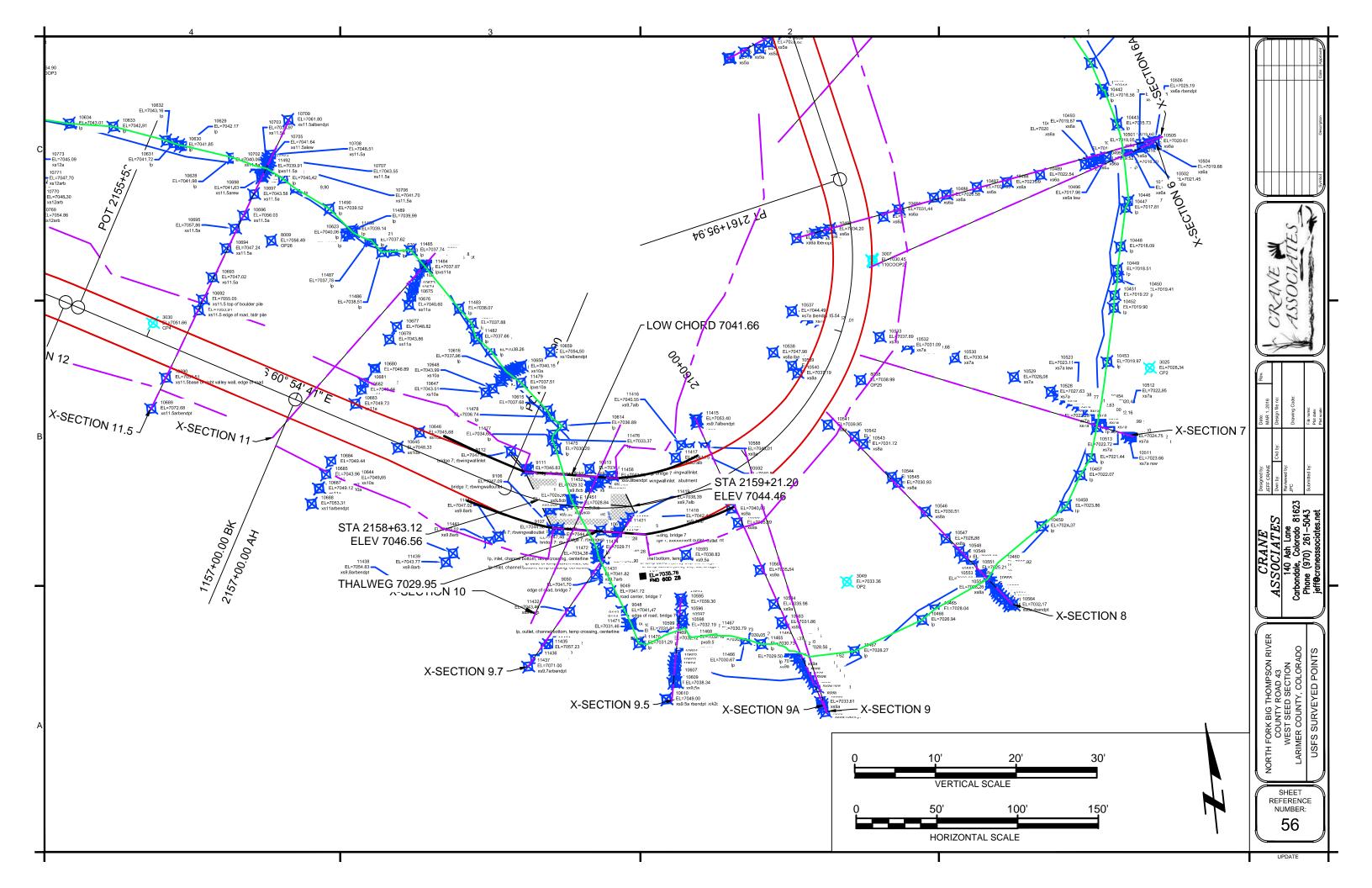


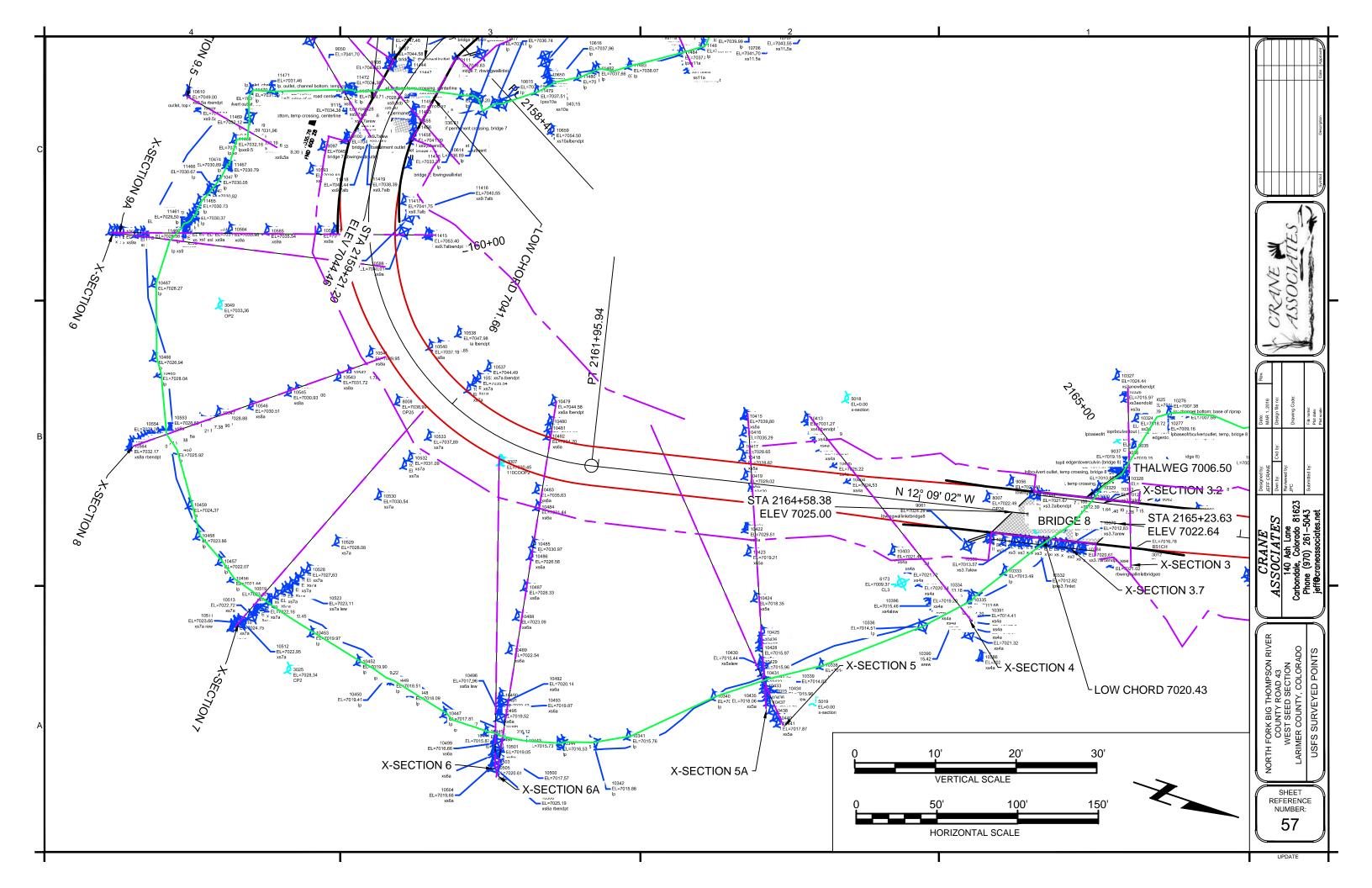


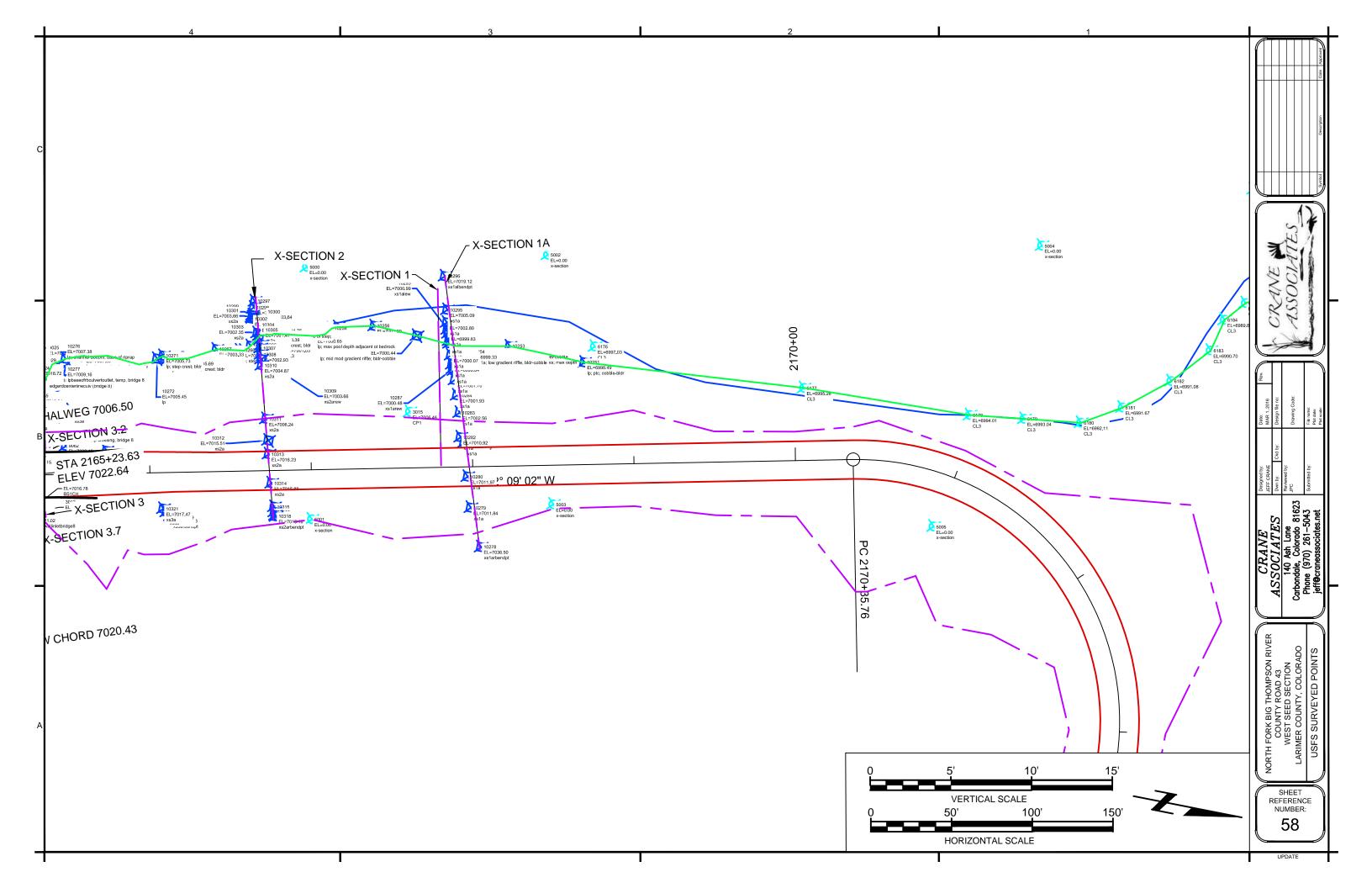


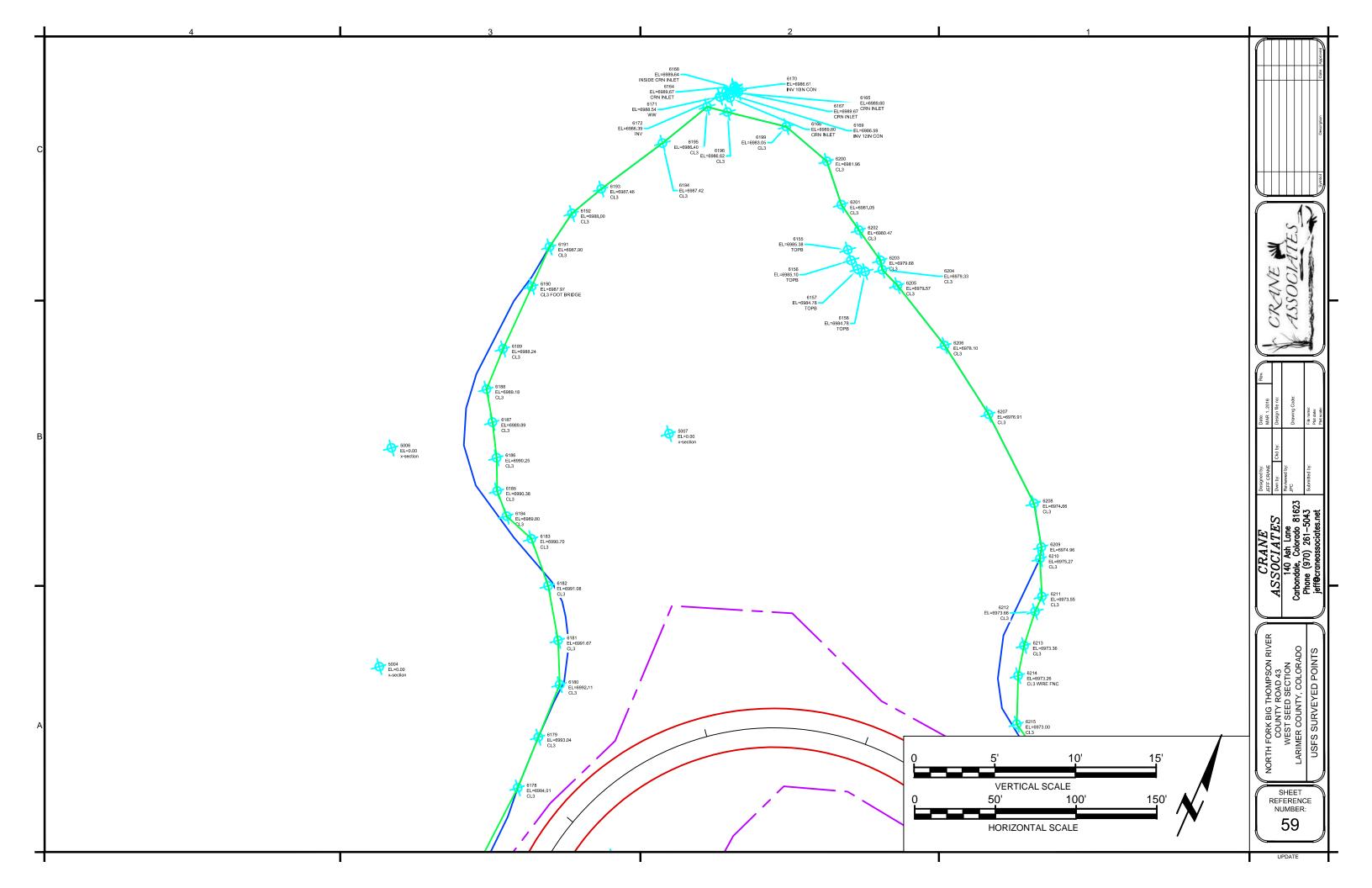








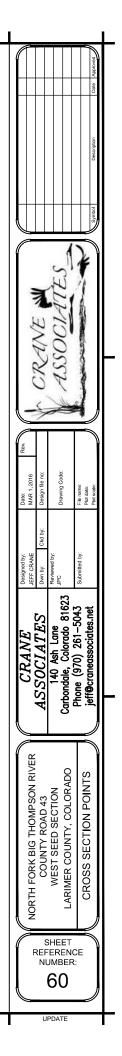




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7053	XS 26	1408718.3217	3016765.7107					
7077	XS 21	1408362.5853	3017501.6488					
7027	XS 37	1407988.1471	3016024.4877					
7051	XS 27	1408819.6823	3016636.2445					
7028	XS 36	1408189.5596	3016088.6065					
7052	XS 26	1408844.5037	3016840.7491					
7030	XS 35	1408270.0578	3016171.9350					
7031	XS 35	1408172.9649	3016321.8607					
7095	BR6 US	1408679.4860	3018297.7959					
7037	BR4 US	1408512.2805	3016316.0727					
7101	XS 13	1408824.3231	3018503.6575					
7038	BR4 DS	1408421.7510	3016262.1094					
7102	XS 13A	1408977.6380	3018523.3457					
7039	BR4 DS	1408529.2097	3016291.7088					
7103	XS 13A	1408793.1863	3018541.7286					
7040	XS 32	1408503.3583	3016196.4944					
7104	XS 12	1408950.6693	3018671.3105					
7041	XS 32	1408541.5836	3016292.6820					
7105	XS 12	1408780.0545	3018618.0472					

	Po	int Table	
Point #	Raw Description	Northing	Easting
Foint #		Northing	Lasting
7047	XS 29	1408741.2377	3016467.7290
7048	XS 28	1408908.2697	3016518.5922
7049	XS 28	1408773.6056	3016596.9274
7050	XS 27	1408913.0091	3016615.5154
7024	XS 38	1408040.5603	3015786.2775
7025	XS 38	1407874.3997	3015914.2759
7026	XS 37	1408112.7600	3015953.5369
7032	XS 34	1408371.5213	3016248.0842
7096	XS 15	1408767.3934	3018219.2379
7033	XS 34	1408340.3339	3016405.4750
7097	XS 15	1408734.8953	3018317.7418
7034	XS 33	1408402.3071	3016246.7141
7098	XS 14	1408891.0375	3018300.5975
7035	XS 33	1408467.1323	3016390.0509
7099	XS 14	1408760.3352	3018380.9002
7036	BR4 US	1408403.9533	3016286.2259
7100	XS 13	1408972.8138	3018521.2840
7042	XS 31	1408659.6881	3016198.5853
7043	XS 31	1408637.3641	3016294.5790
7044	XS 30	1408775.2030	3016233.9973

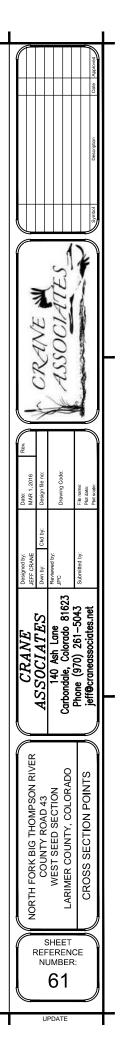
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7045	XS 30	1408692.9662	3016345.1816
7046	XS 29	1408843.2026	3016396.6365
7054	XS 26A	1408844.5096	3016840.8280
7055	XS 26A	1408686.6483	3016777.1736
7056	XS 25	1408744.3855	3016966.4550
7057	XS 25	1408633.8601	3016872.7827
7058	XS 24	1408622.5100	3017079.5375
7059	XS 24	1408525.3429	3016931.7858
7060	XS 23A	1408511.6141	3017133.8186
7061	XS 23A	1408412.6956	3016996.4677
7062	XS 23	1408478.3153	3017138.7334
7063	XS 23	1408421.0758	3017034.3393
7064	BR4 US	1408430.3997	3017155.4811
7065	BR4 US	1408477.1298	3017052.6249
7066	XS 22.7	1408447.7714	3017180.2348
7067	XS 22.7	1408436.4256	3017033.5087
7068	BR5 DS	1408394.2279	3017154.1050
7069	BR5 DS	1408440.7121	3017051.7890
7070	XS 22.3	1408411.6638	3017148.8899
7071	XS 22.3	1408349.3211	3017053.0230



Point Table			
Point #	Raw Description	Northing	Easting
7121	XS 9	1408482.7572	3019133.9161
7122	XS 8	1408667.2861	3019153.6066
7123	XS 8	1408536.8536	3019255.2202
7124	XS 7	1408700.1053	3019178.3075
7125	XS 7	1408632.1170	3019340.9070
7127	XS 6	1408812.0549	3019373.2787
7128	XS 6A	1408776.5216	3019146.2691
7129	XS 6A	1408814.6988	3019378.7195
7130	XS 5	1408836.8866	3019141.8829
7131	XS 5	1408973.0785	3019292.0830
7132	XS 5A	1408891.4990	3019116.2251
7133	XS 5A	1408959.8718	3019285.7629
7134	XS 4	1408928.3117	3019105.5229
7135	XS 4	1409063.5443	3019194.1068
7136	XS 3	1409109.8230	3019026.6193
7137	XS 3	1409146.7915	3019131.4370
7000	XS 47	1408535.9739	3015169.0468
7001	XS 47	1408405.9798	3015191.0292
7002	XS 46.5	1408523.8948	3015281.0241
7003	XS 46.5	1408410.3046	3015221.7739

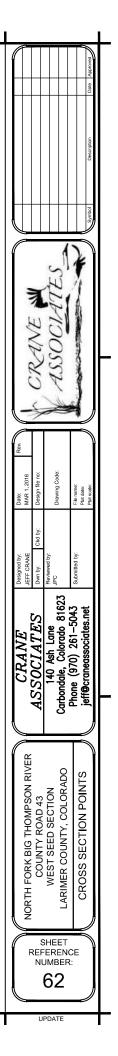
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Point #	Raw Description	Northing	Easting	
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7073	XS 22	1408275.6871	3017177.1523	
7074	XS 21.5	1408445.8757	3017344.9005	
7075	XS 21.5	1408321.5751	3017364.4155	
7076	XS 21	1408452.7376	3017492.5738	
7106	XS 11.5	1408882.8588	3018842.0458	
7107	XS 11.5	1408713.6876	3018737.5567	
7108	XS 11	1408806.8386	3018940.2141	
7109	XS 11	1408688.3488	3018810.7306	
7110	XS 10	1408673.0188	3019025.7297	
7111	XS 10	1408556.8072	3018957.6158	
7112	BR7 US	1408636.5729	3019042.4272	
7113	BR7 US	1408654.7839	3018939.3712	
7114	XS 9.7	1408615.5096	3019026.9988	
7115	XS 9.7	1408530.4138	3018952.9033	
7116	XS 9.5	1408562.5096	3019051.6986	
7117	XS 9.5	1408499.5621	3019039.2267	
7118	XS 9A	1408672.5612	3019072.3118	
7119	XS 9A	1408482.0881	3019134.0569	
7120	XS 9	1408649.4795	3019100.7157	

	Pol	int Table	
Doint #			
Point #	Raw Description	Northing	Easting
7004	XS 46	1408413.9488	3015496.5284
7005	XS 46	1408321.0603	3015425.5818
7006	XS 45	1408379.5255	3015600.1170
7007	XS 45	1408273.0502	3015478.9987
7008	XS 44	1408245.3479	3015670.7882
7009	XS 44	1408181.4879	3015545.2457
7010	XS 43	1408116.1161	3015713.9364
7011	XS 43	1408097.4283	3015655.4707
7012	BR3 US	1408036.1706	3015736.9501
7013	BR3 US	1408099.3828	3015632.0422
7014	BR3 DS	1407992.2609	3015744.9119
7015	BR3 DS	1408056.8089	3015637.7891
7016	XS 42	1408003.8694	3015740.3487
7017	XS 42	1407903.0376	3015611.4723
7018	XS 41	1408011.6041	3015778.6685
7019	XS 41	1407825.9895	3015676.7628
7020	XS 40	1408041.7027	3015786.5485
7021	XS 40	1407812.2711	3015764.2663
7022	XS 39	1408041.6614	3015786.5656
7023	XS 39	1407828.2307	3015830.3184

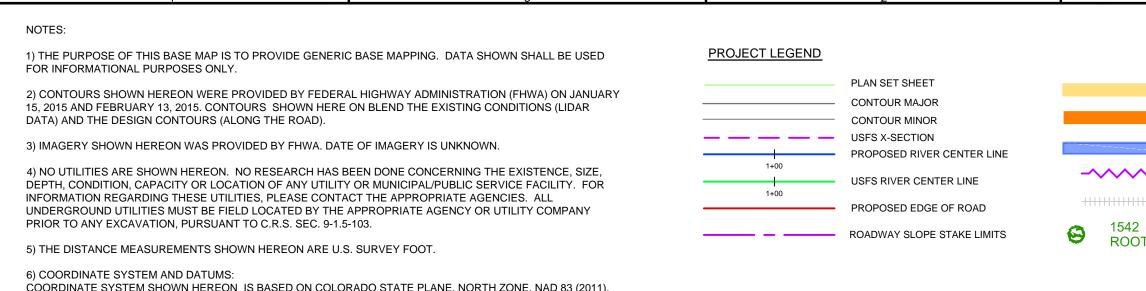


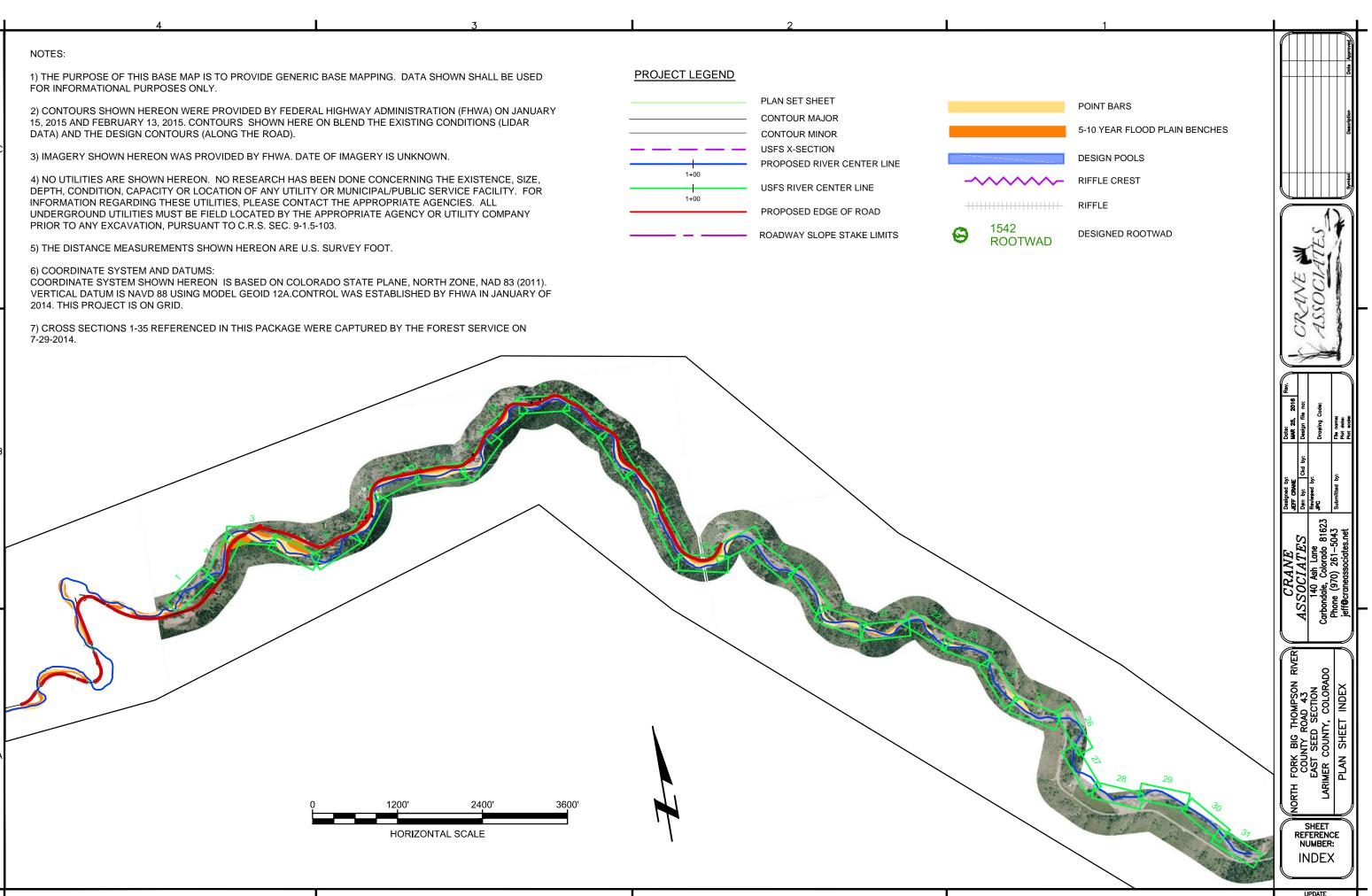
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7079	XS 20.5	1408364.2676	3017638.0630
7080	XS 20	1408503.0608	3017765.3960
7081	XS 20	1408367.6328	3017773.7564
7082	XS 19.5	1408508.3612	3017930.7745
7083	XS 19.5	1408367.2772	3017931.6045
7084	XS 19A	1408567.8474	3018088.3715
7085	XS 19A	1408365.6753	3018175.1487
7086	XS 19	1408558.7723	3018149.8671
7087	XS 19	1408327.8948	3018184.0207
7088	XS 18	1408558.6761	3018151.0250
7089	XS 18	1408413.4049	3018318.8538
7090	XS 17	1408558.6269	3018153.4527
7091	XS 17	1408506.5356	3018360.1971
7092	XS 16	1408559.8942	3018152.3066
7093	XS 16	1408597.7982	3018308.1532
7094	BR6 US	1408604.8059	3018222.2850
7126	XS 6	1408753.6773	3019189.7774
7138	XS 2	1409248.2236	3018972.7793
7139	XS 2	1409285.3297	3019105.6501

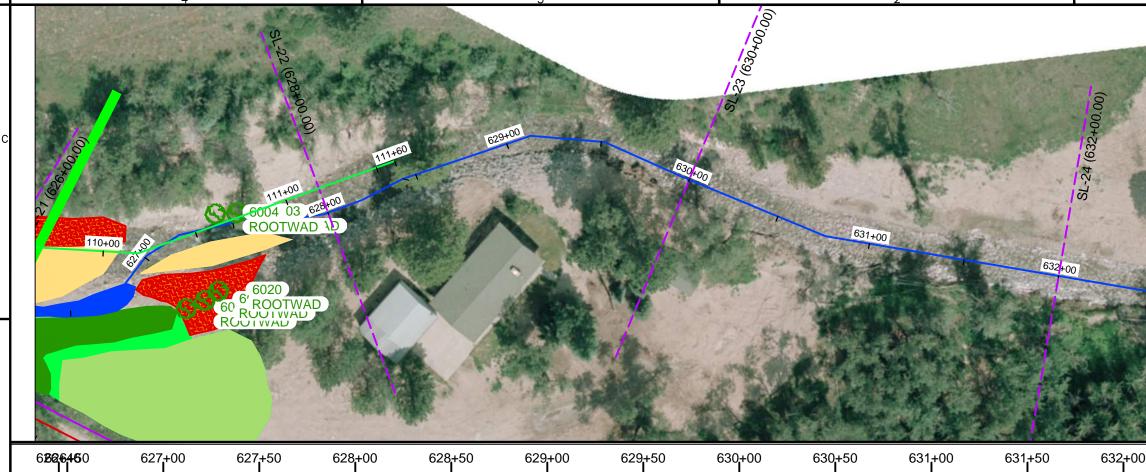
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7140	XS 1	1409357.9384	3018946.1042
7141	XS 1	1409380.9914	3019053.0867
7142	XS 1A	1409360.6431	3018937.3564
7143	XS 1A	1409414.5063	3019099.5683

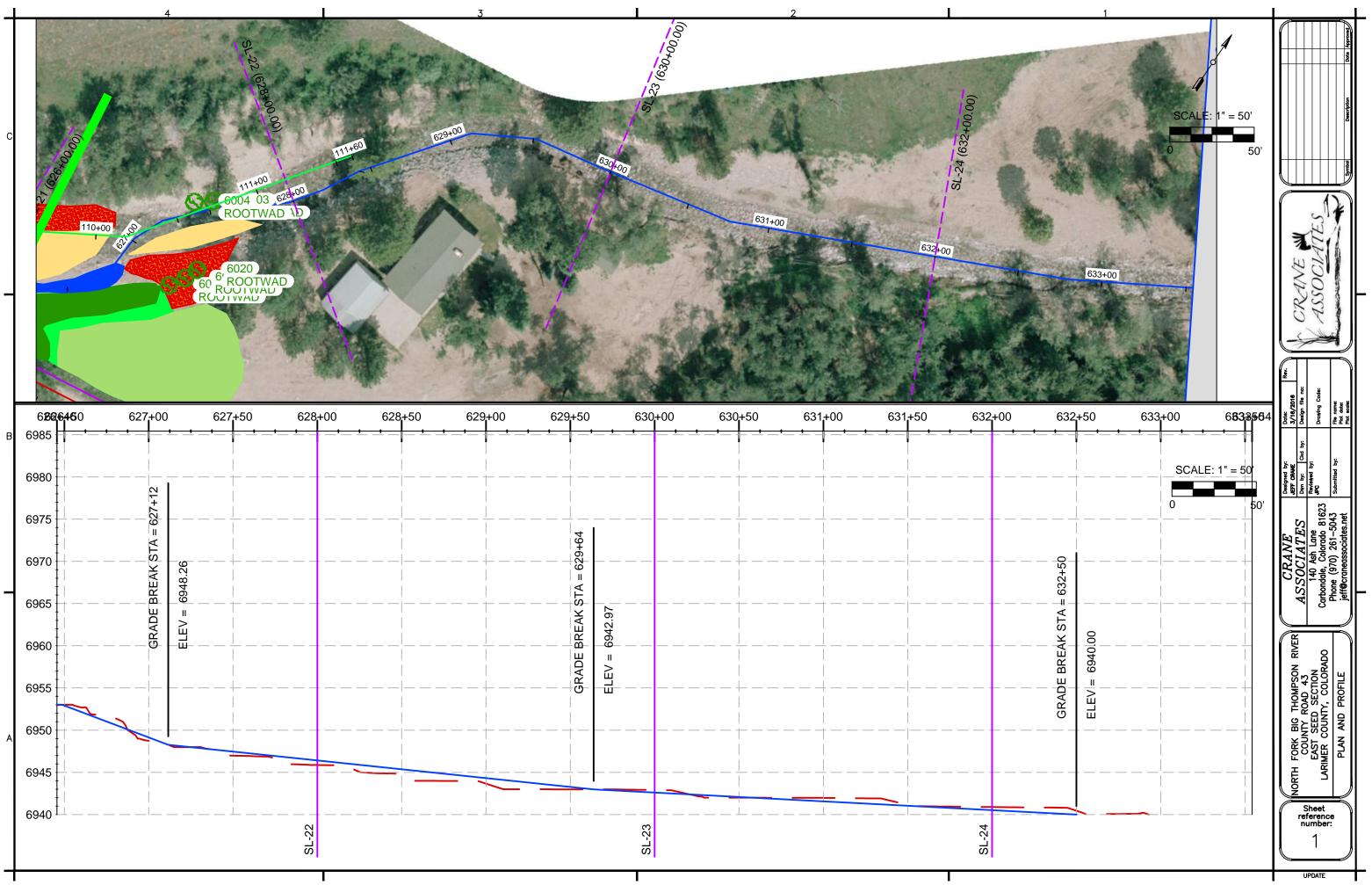


Appendix B: East Seed Plans

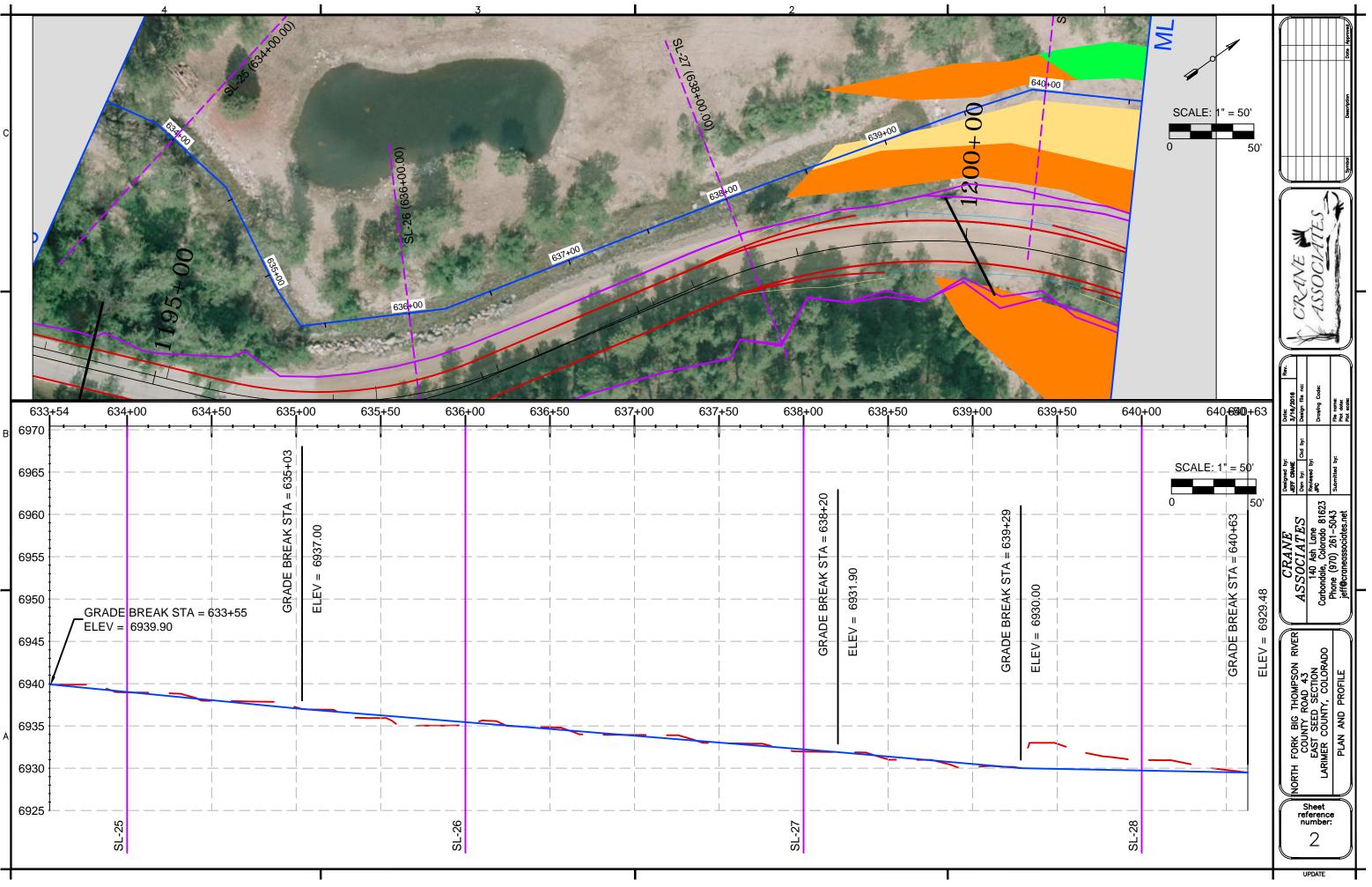


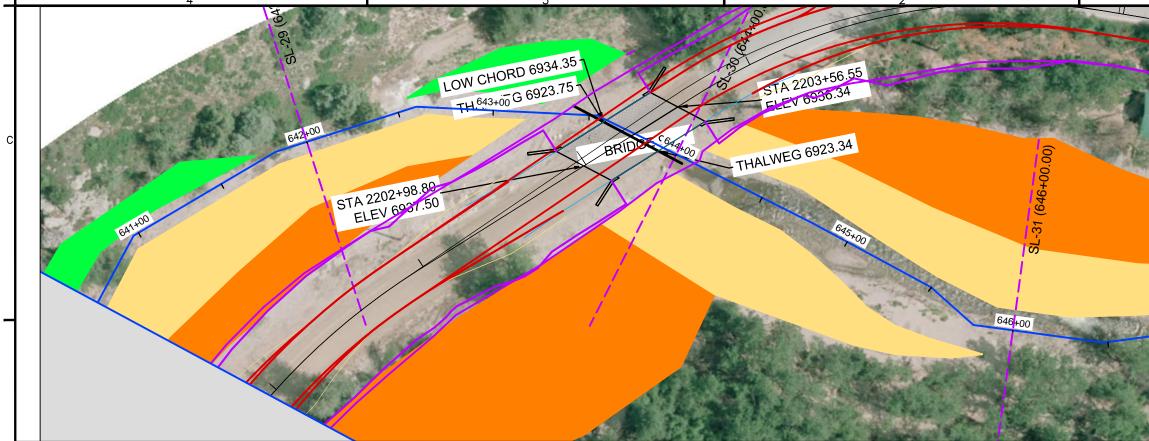


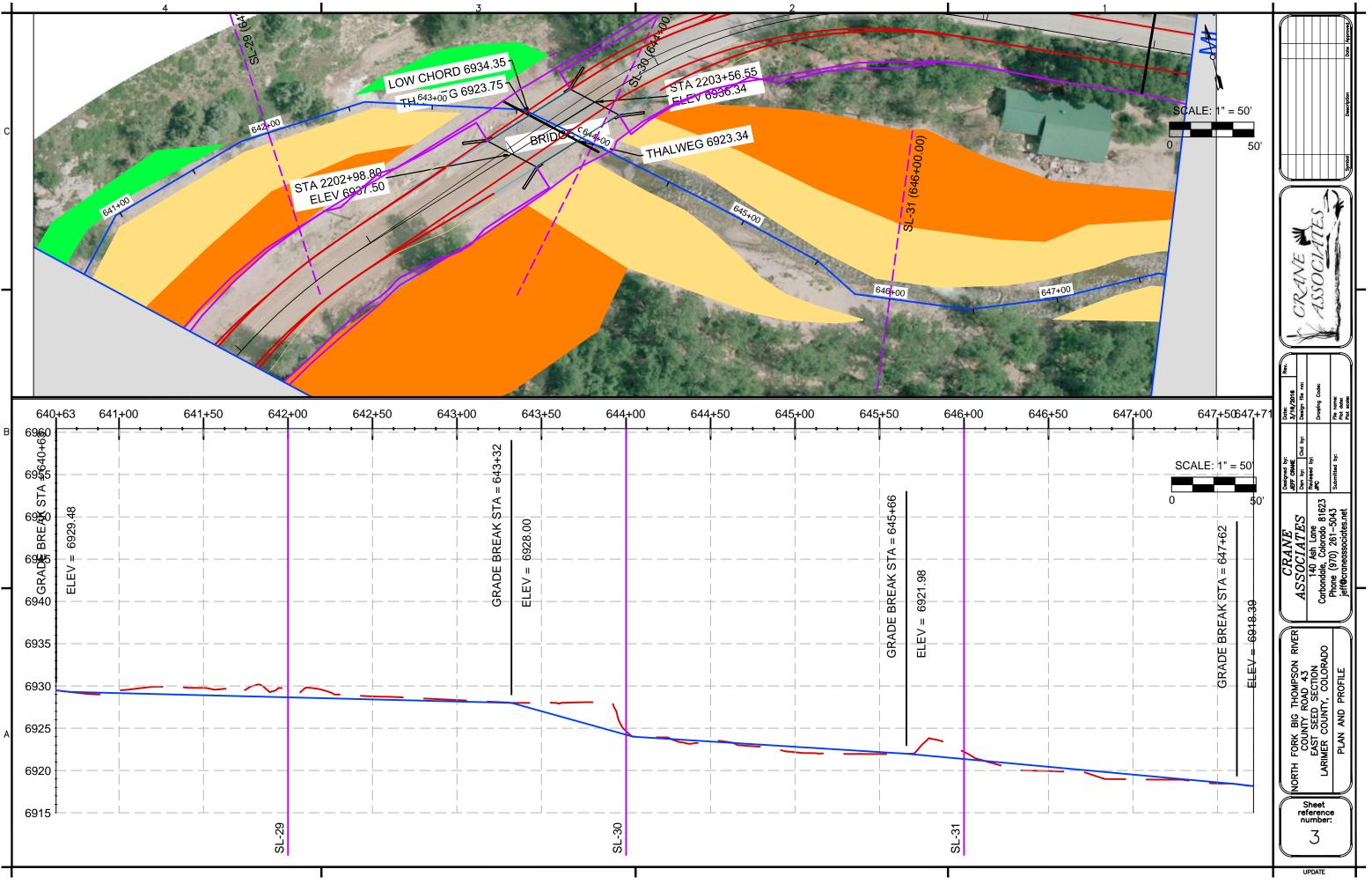


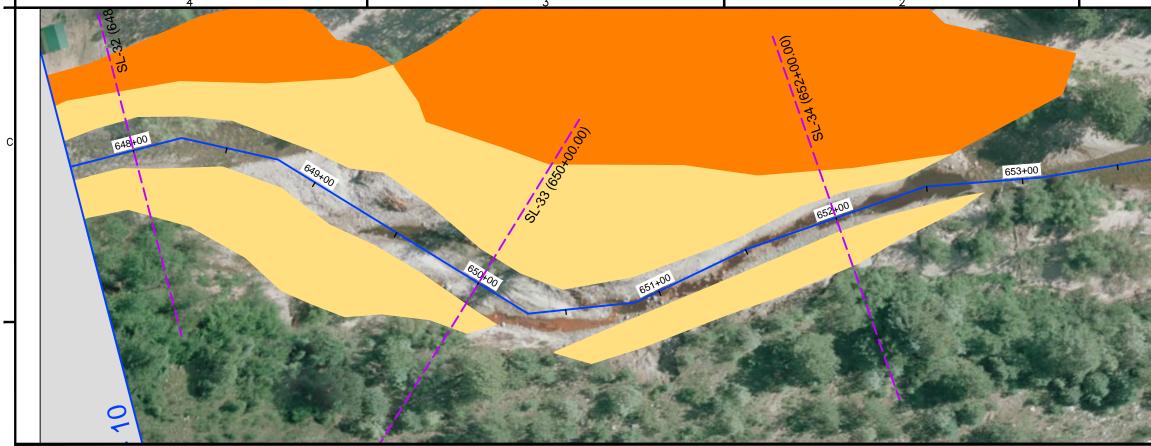


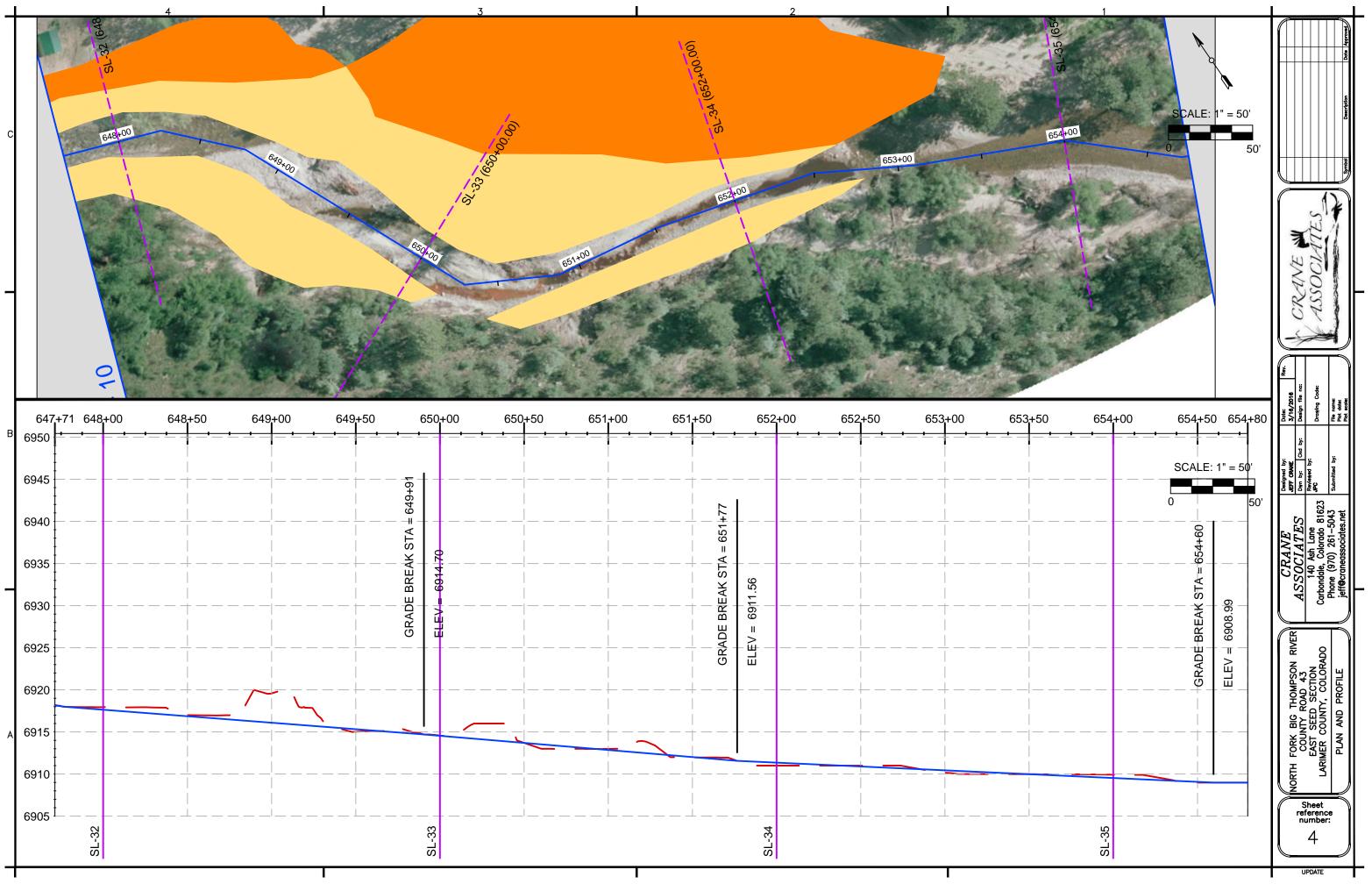




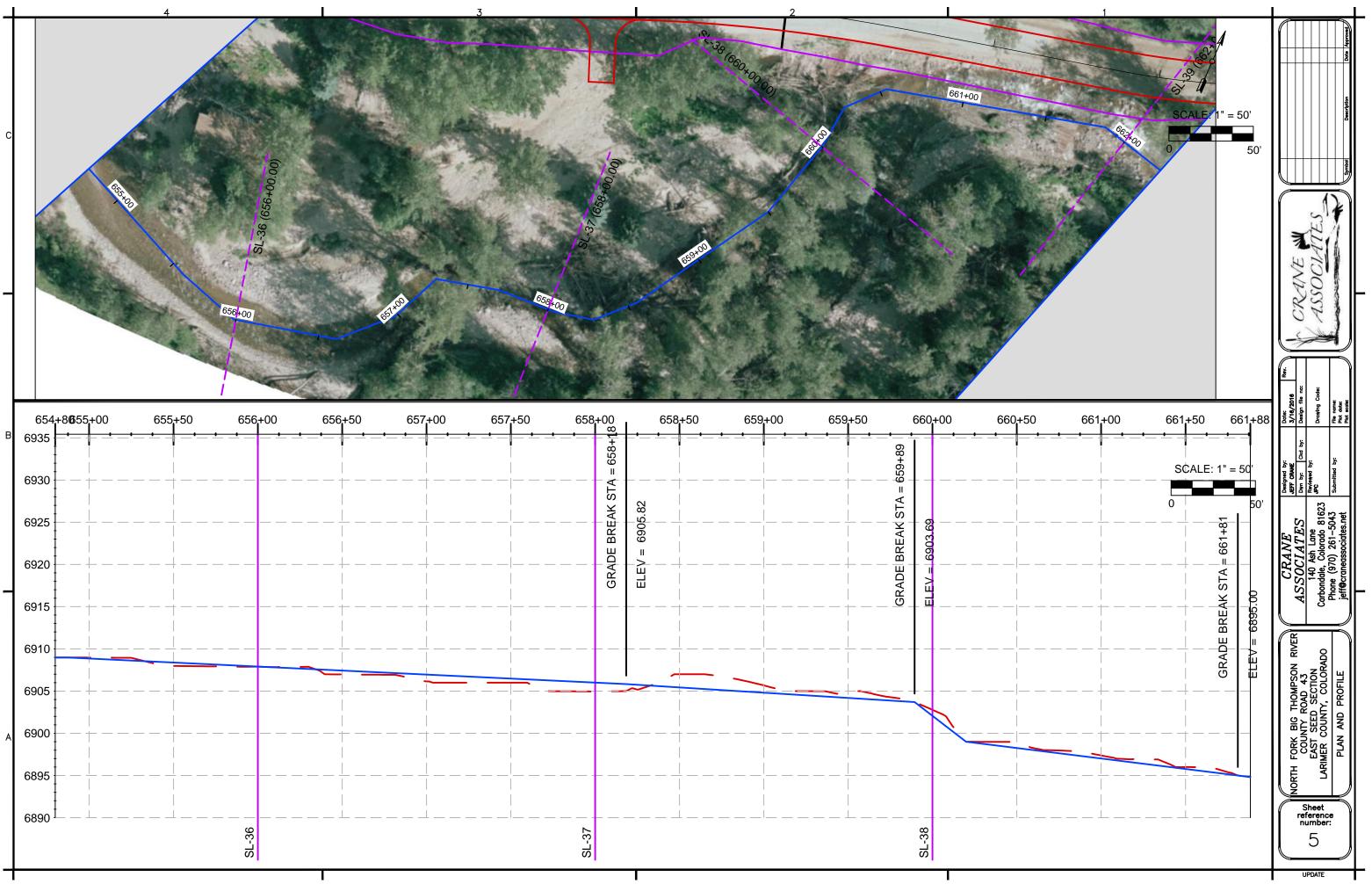


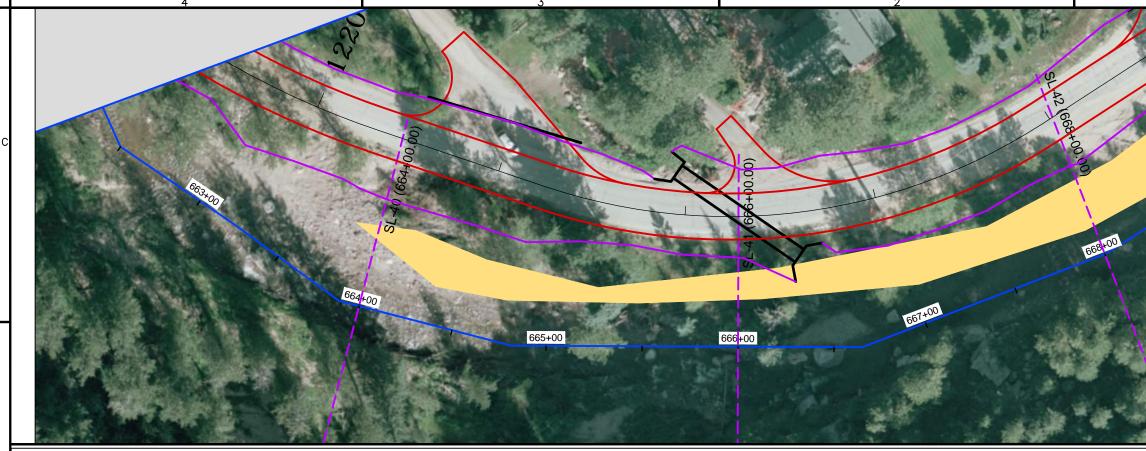


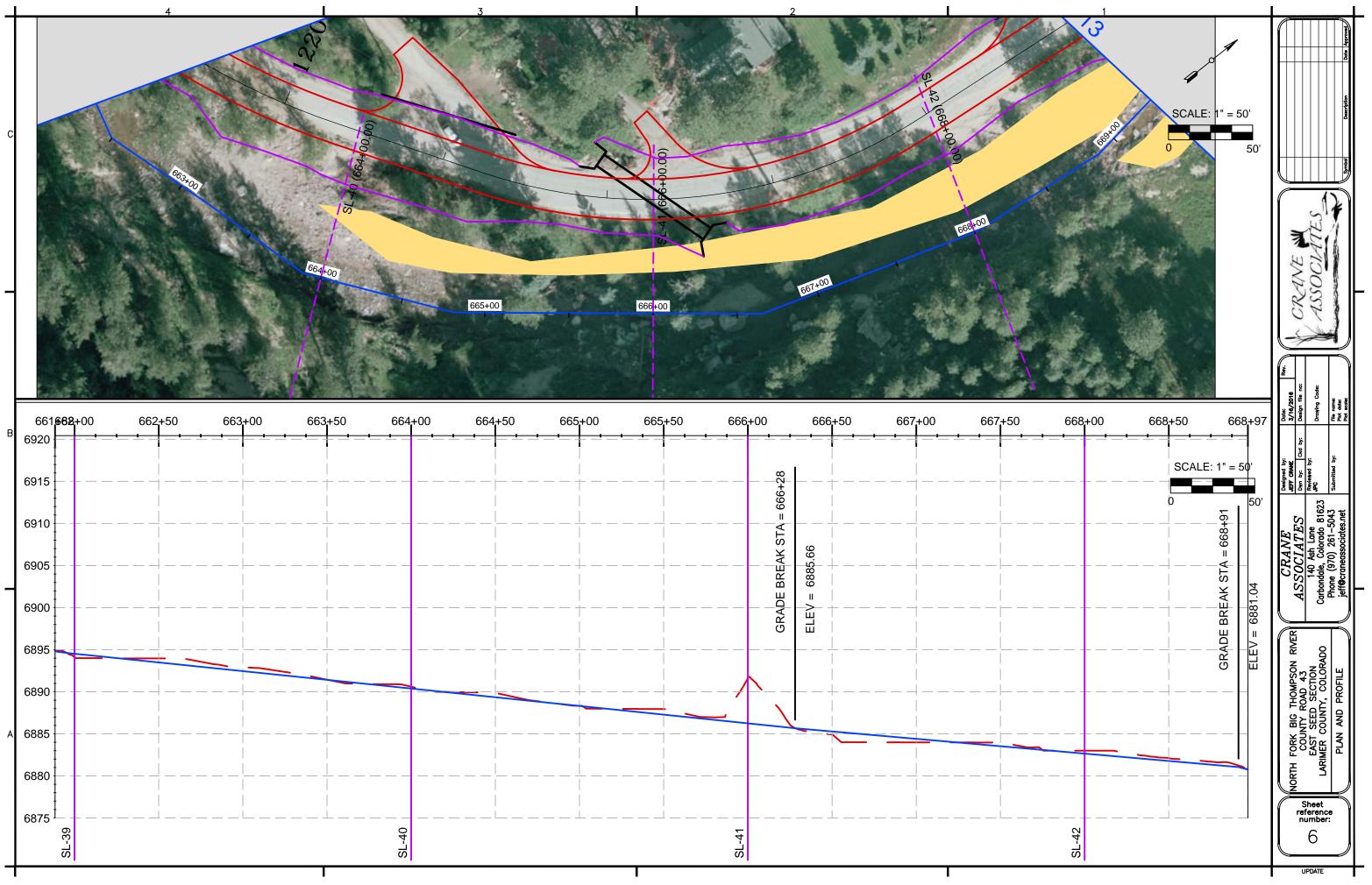


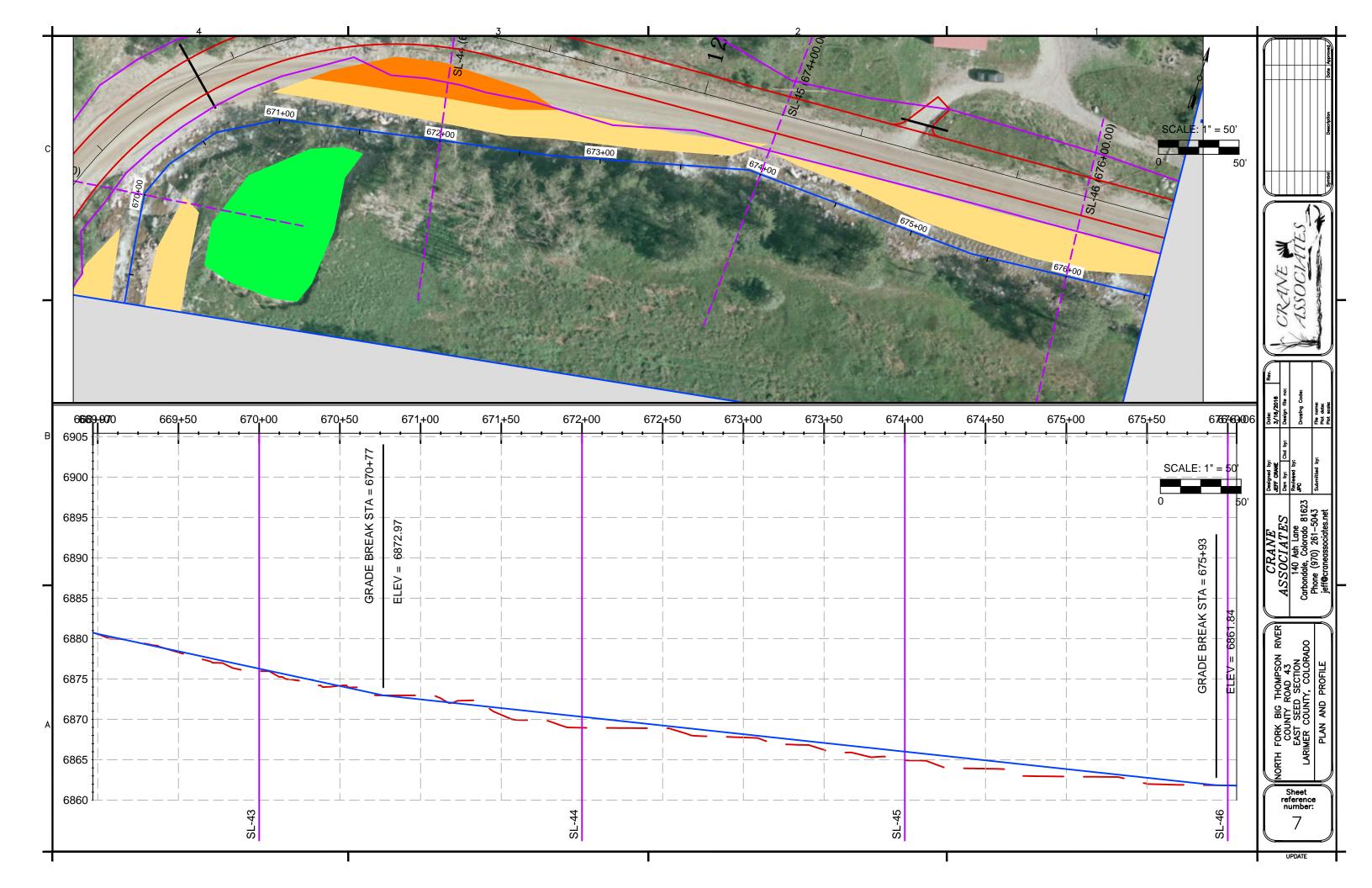


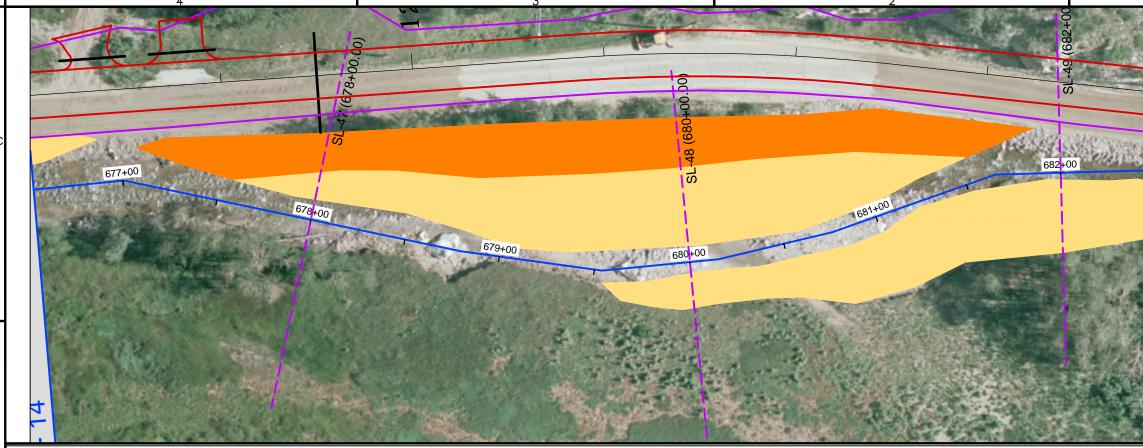


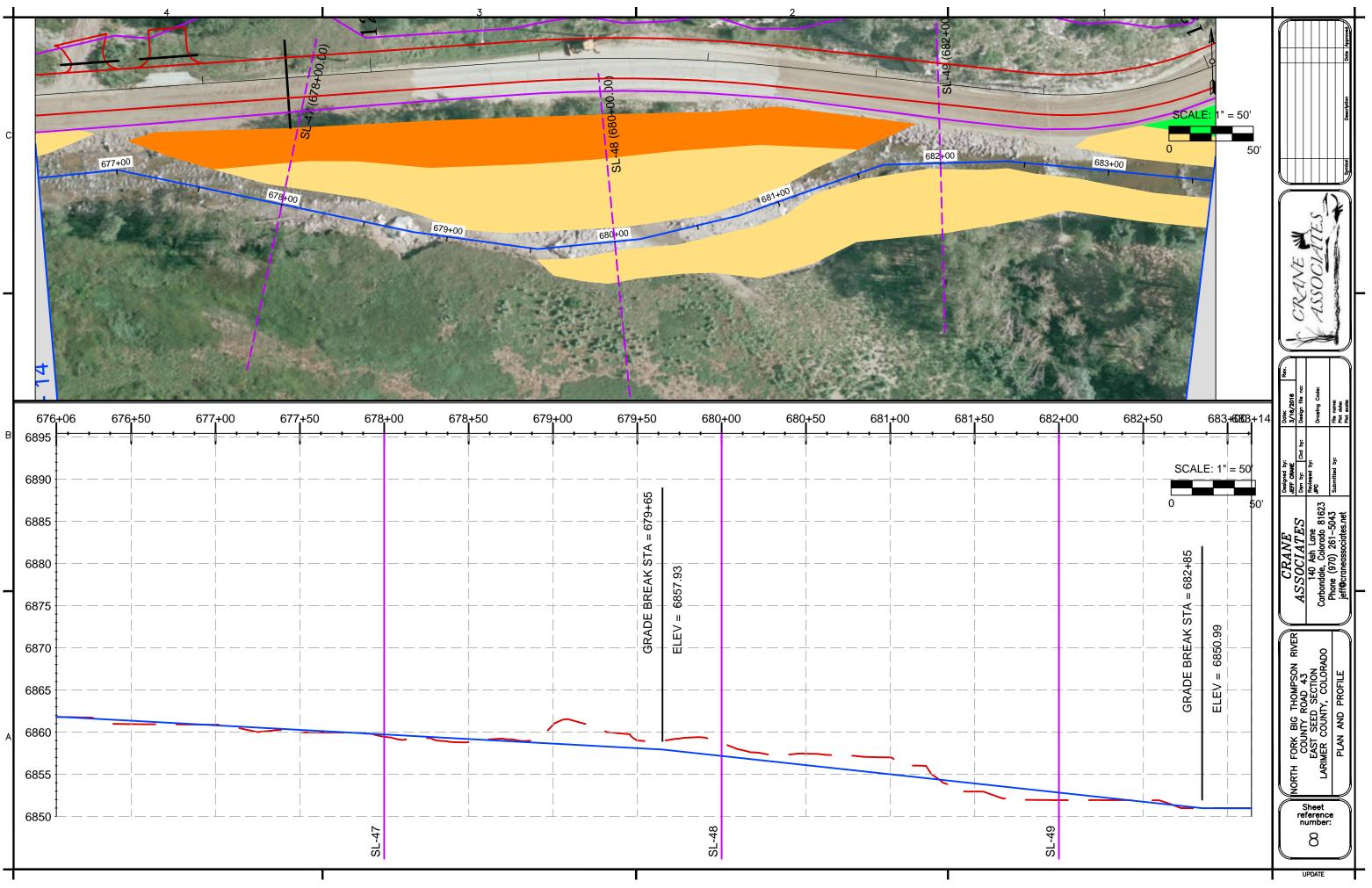


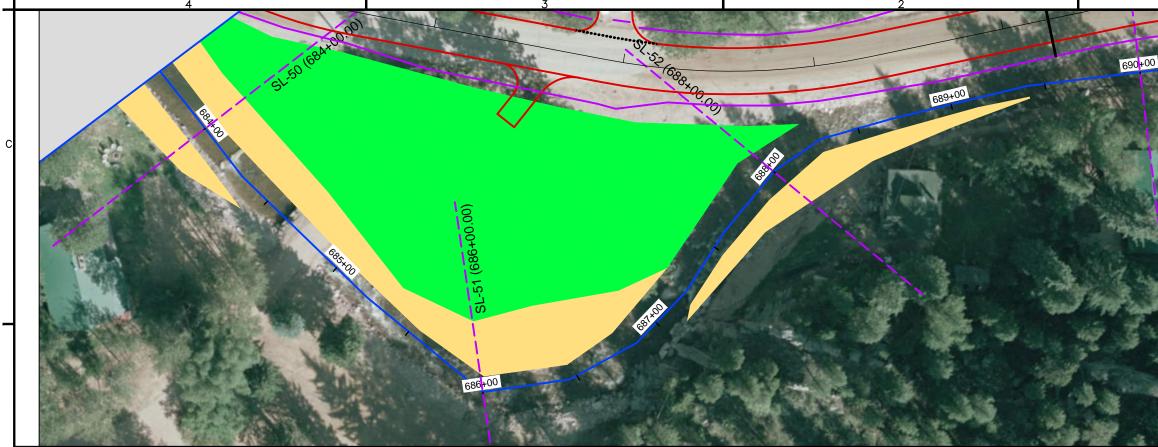




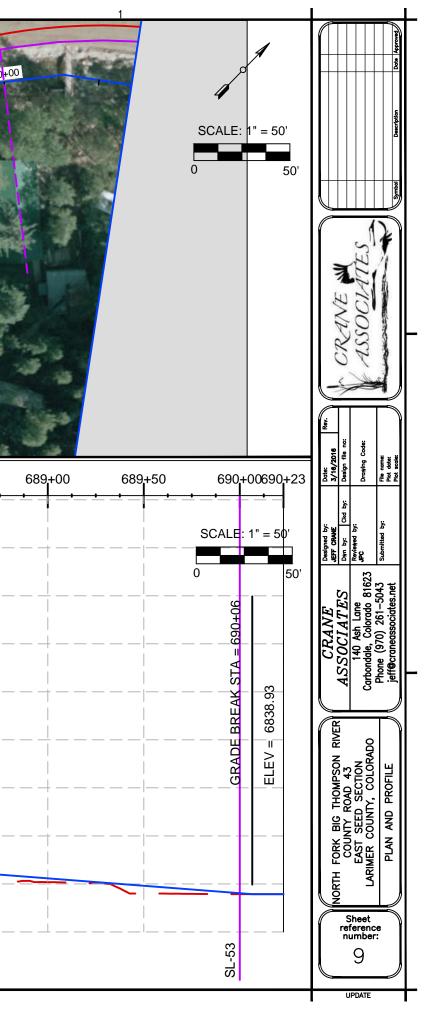


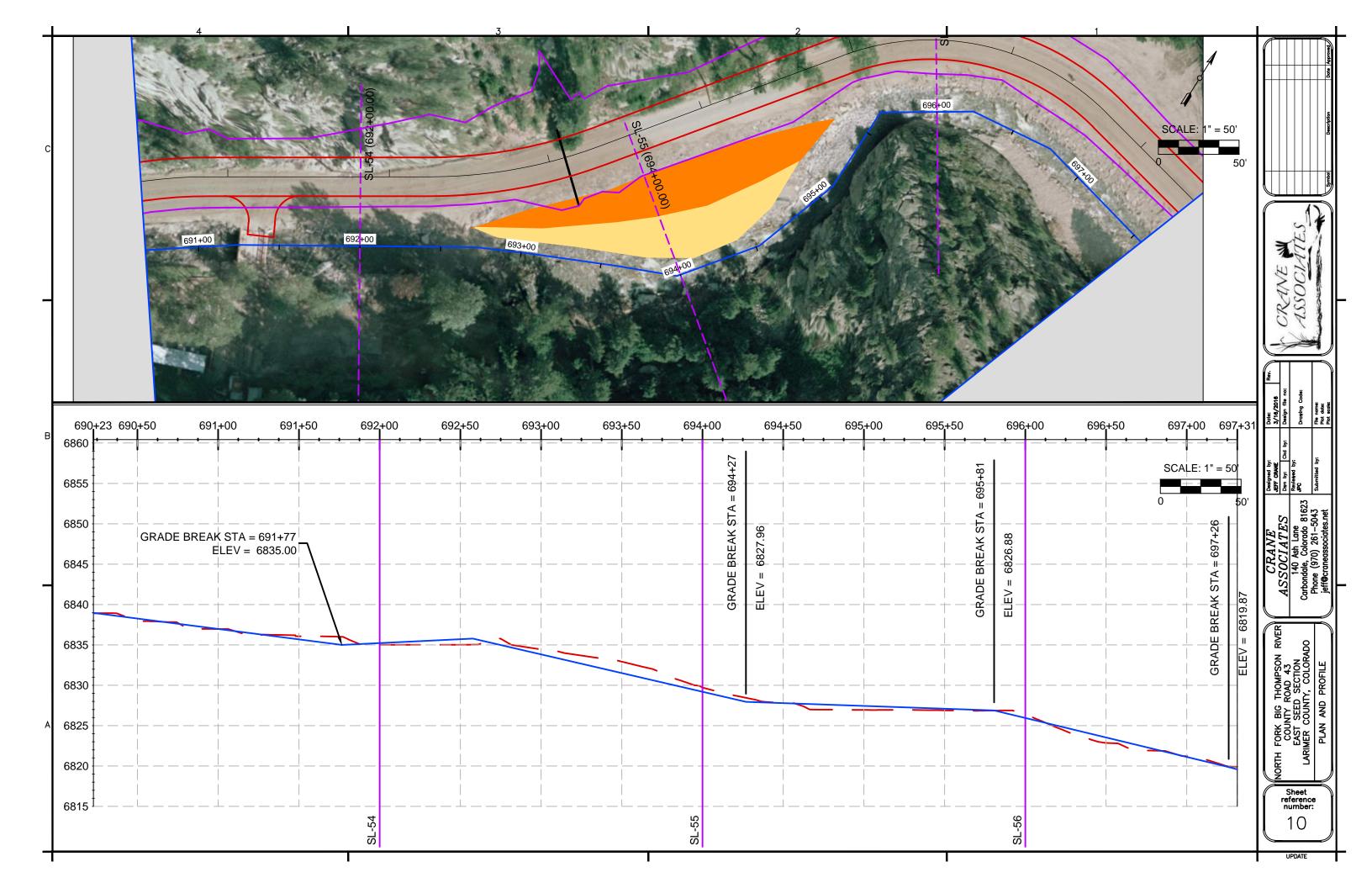


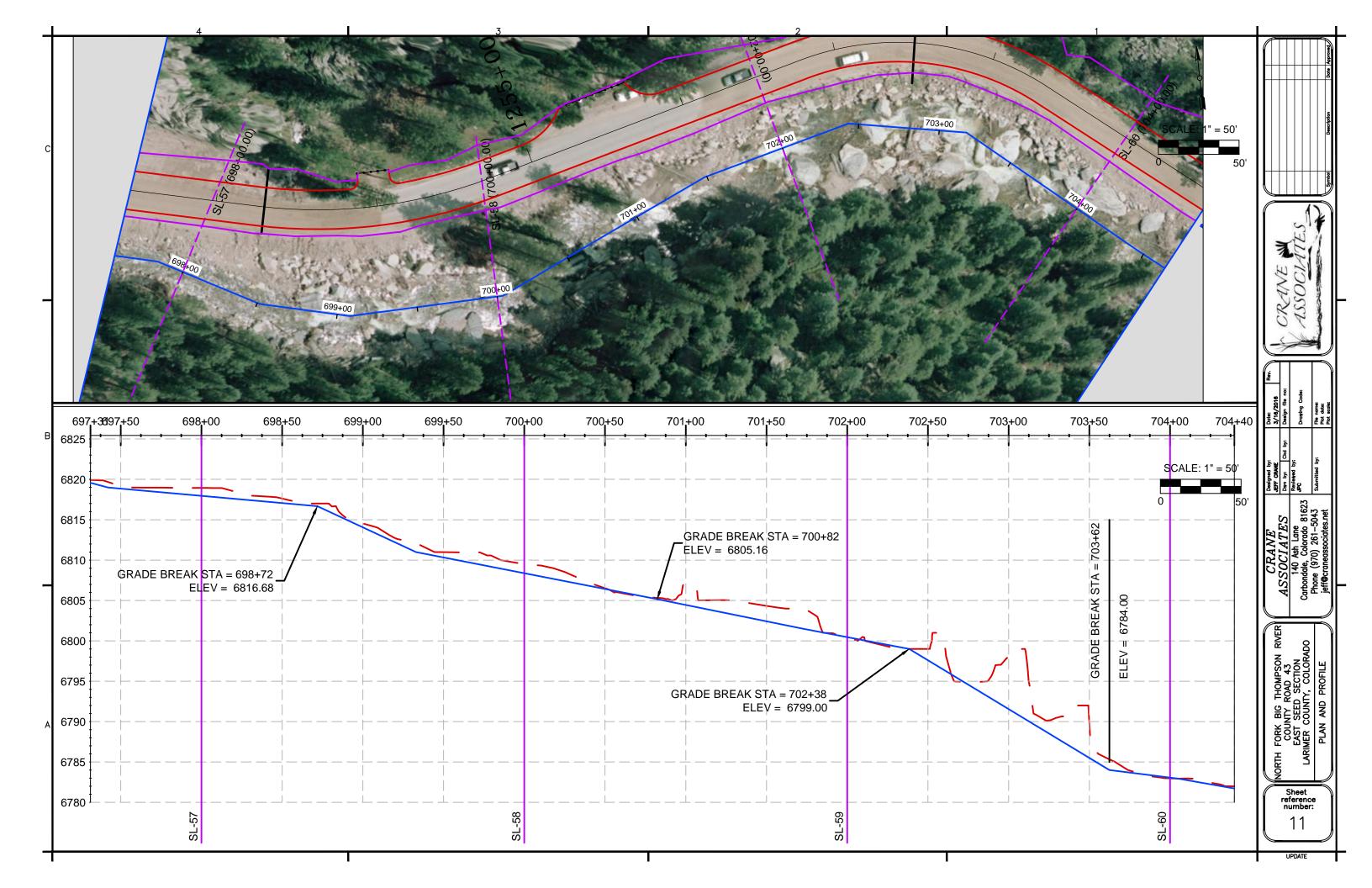


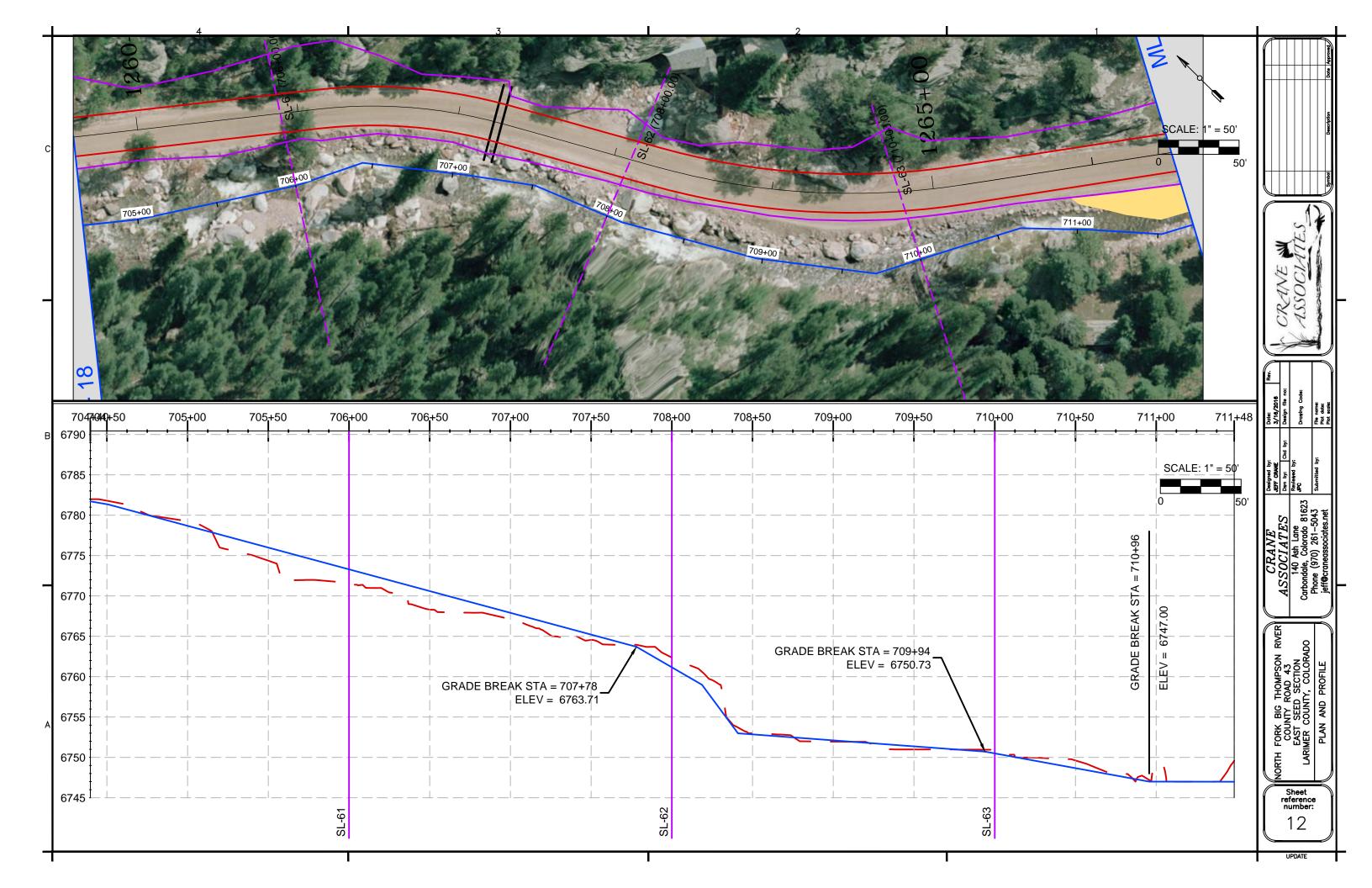


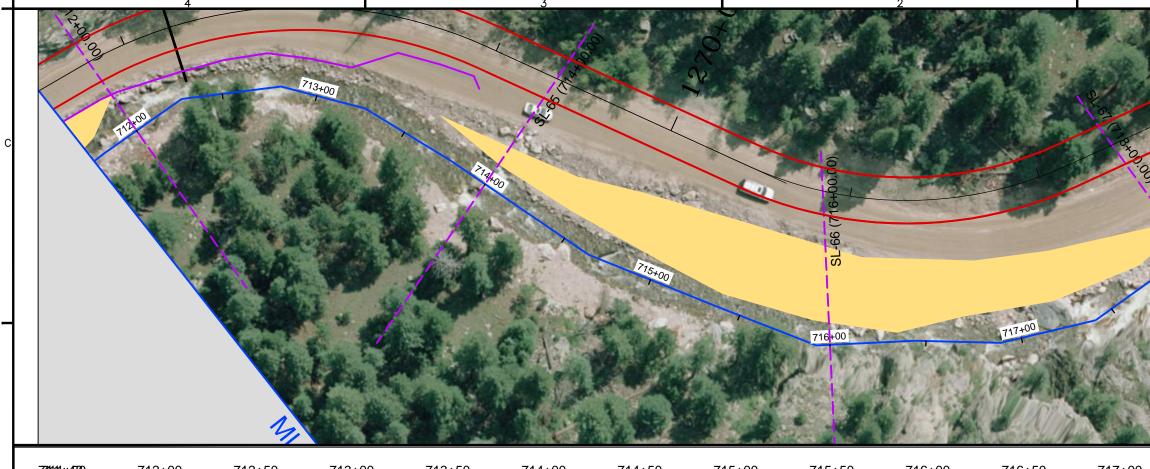
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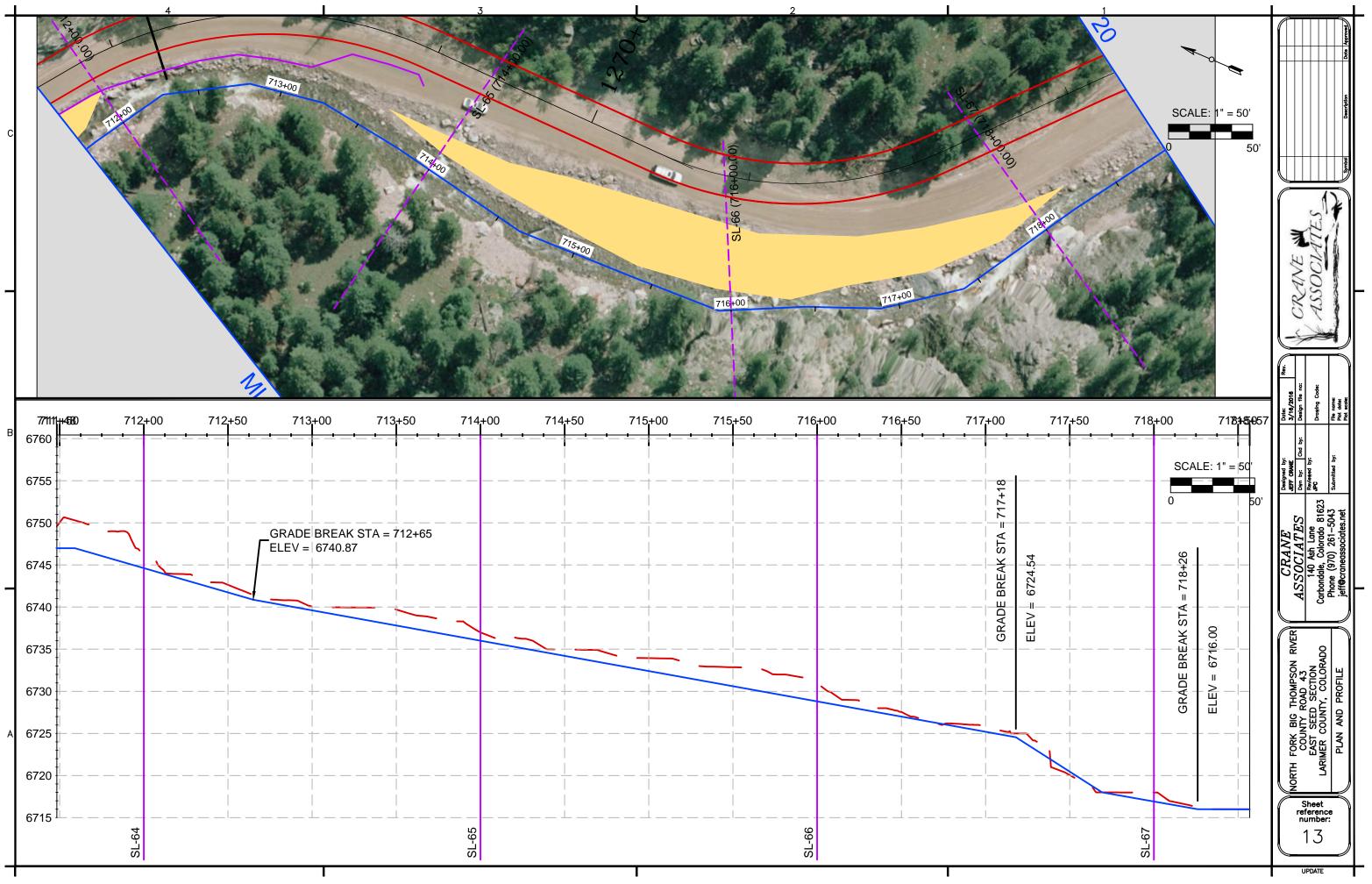


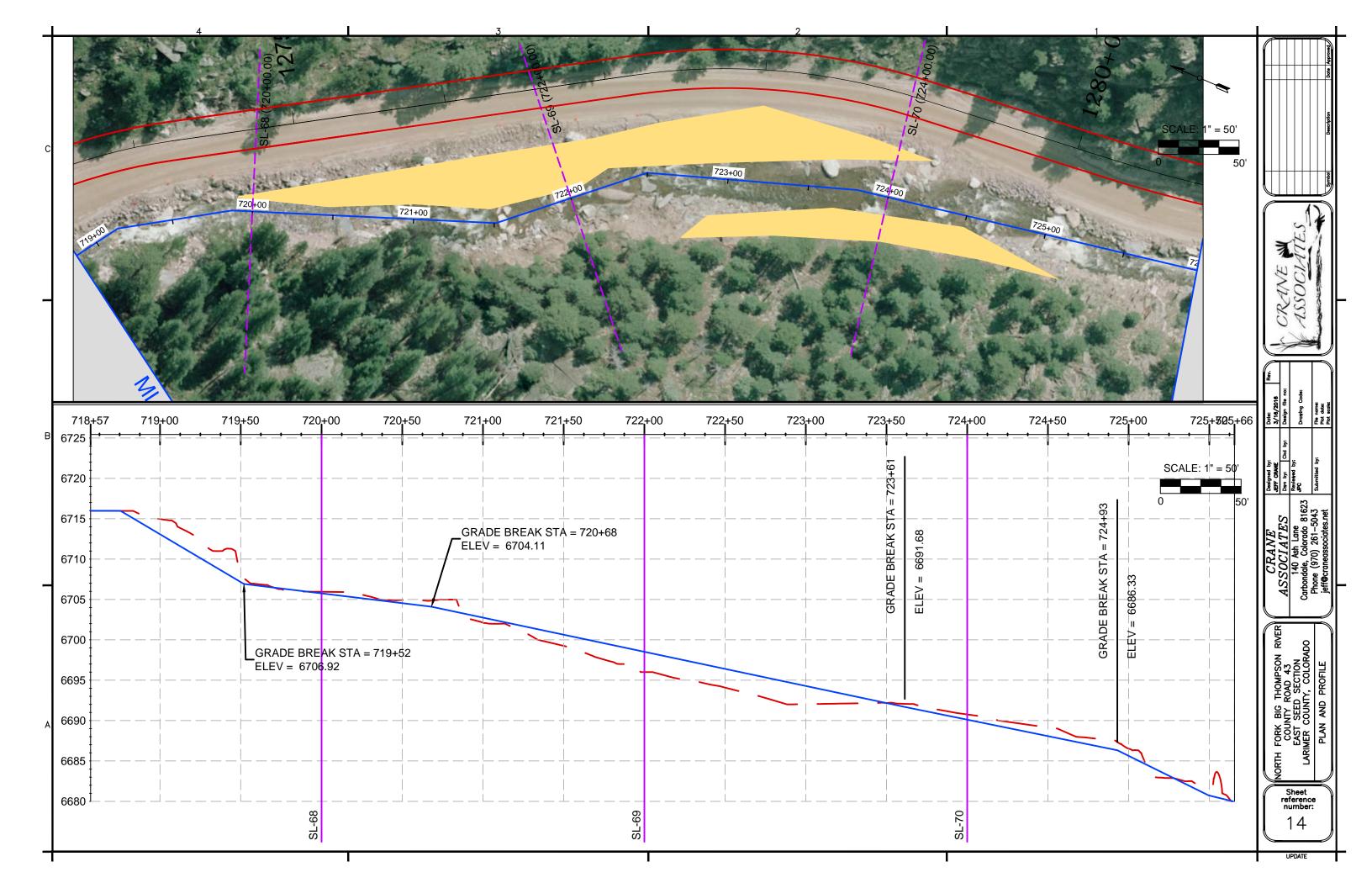


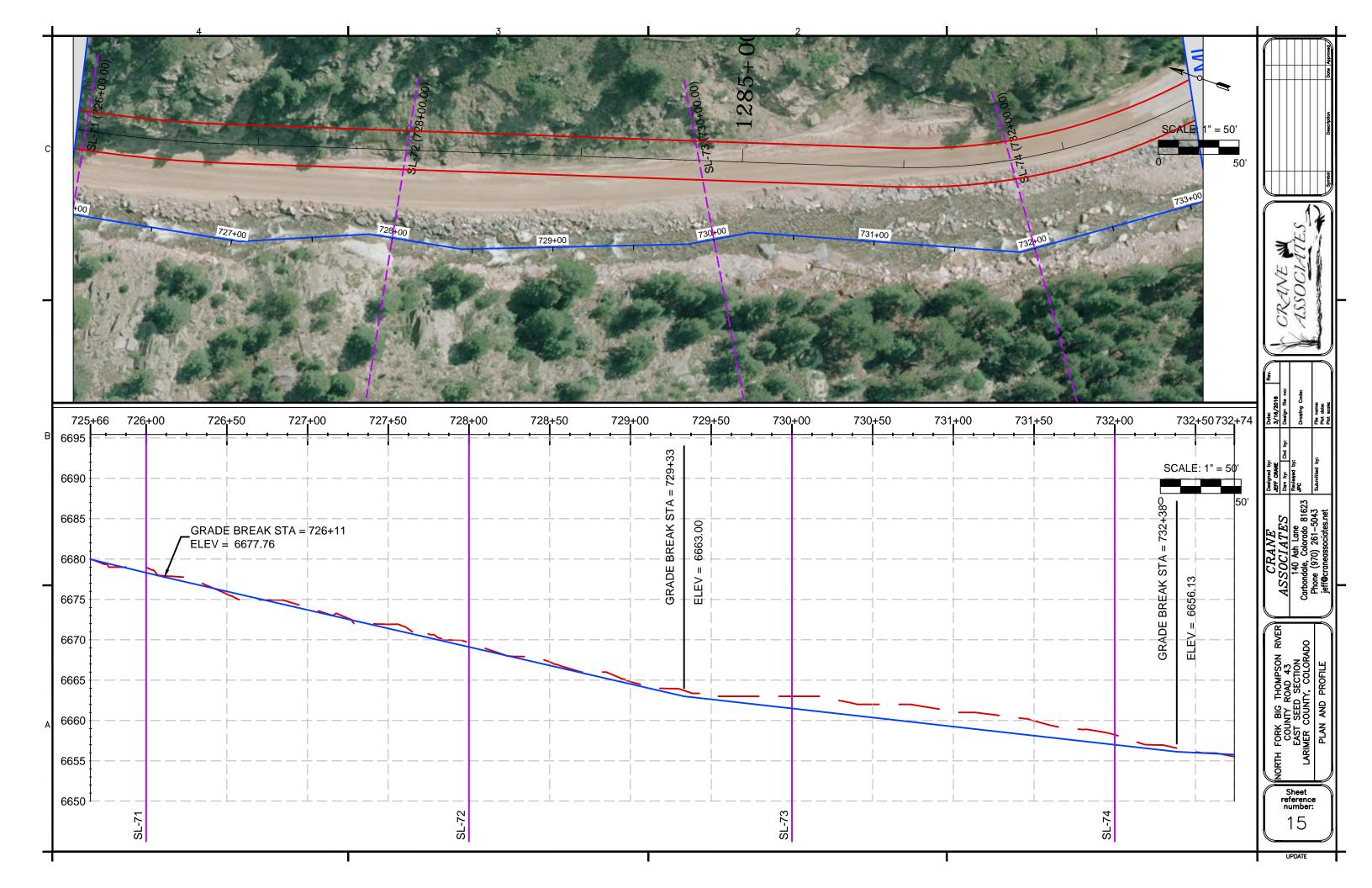


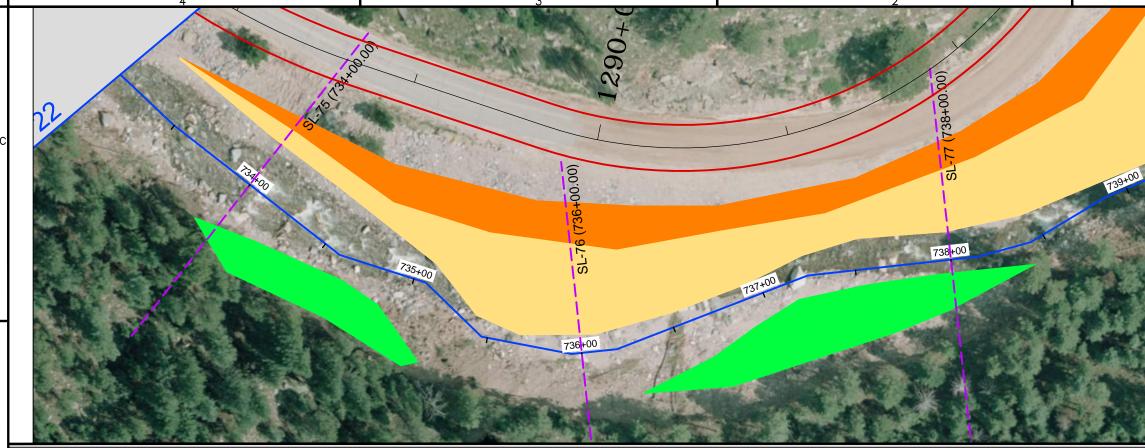


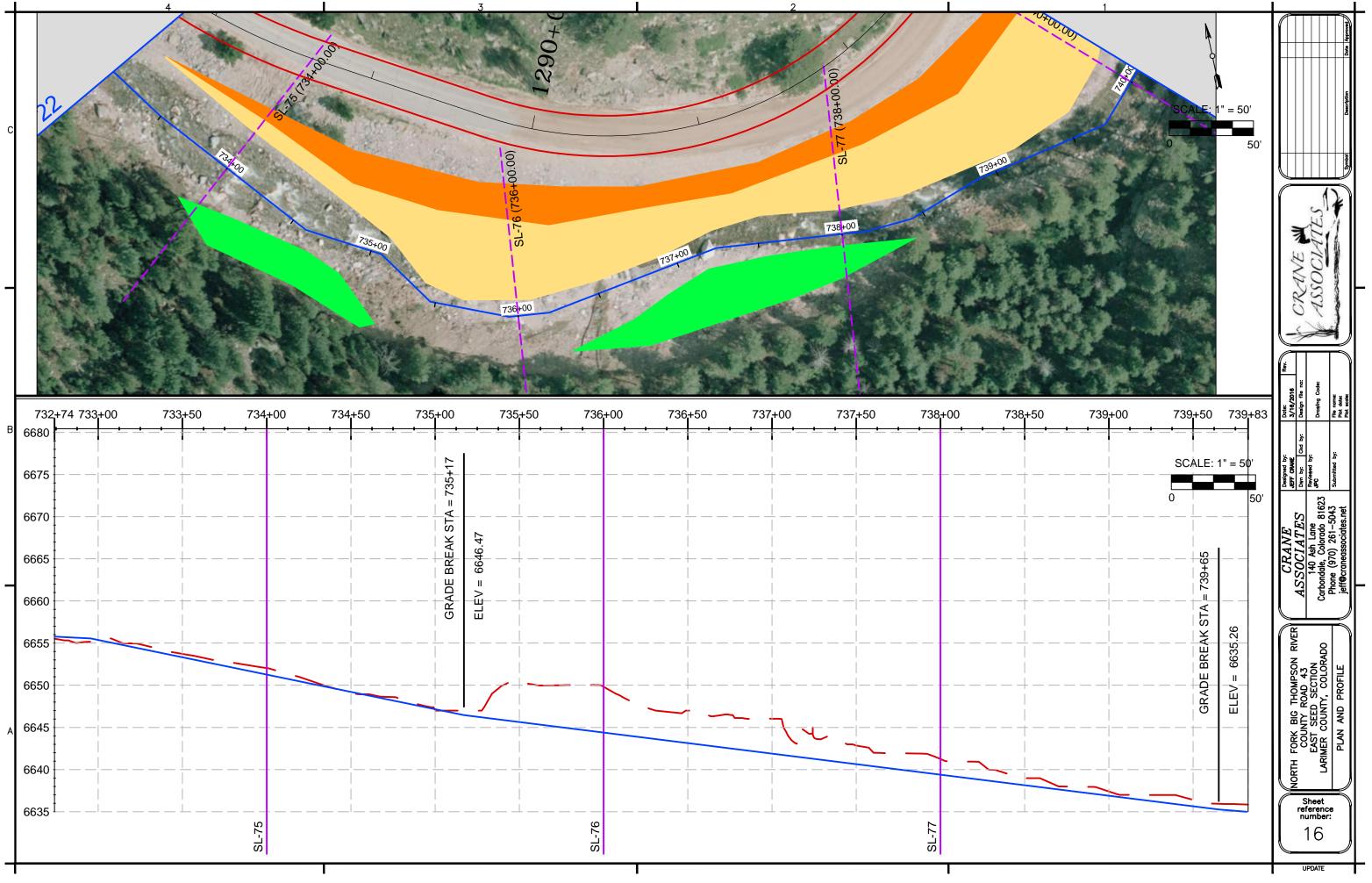


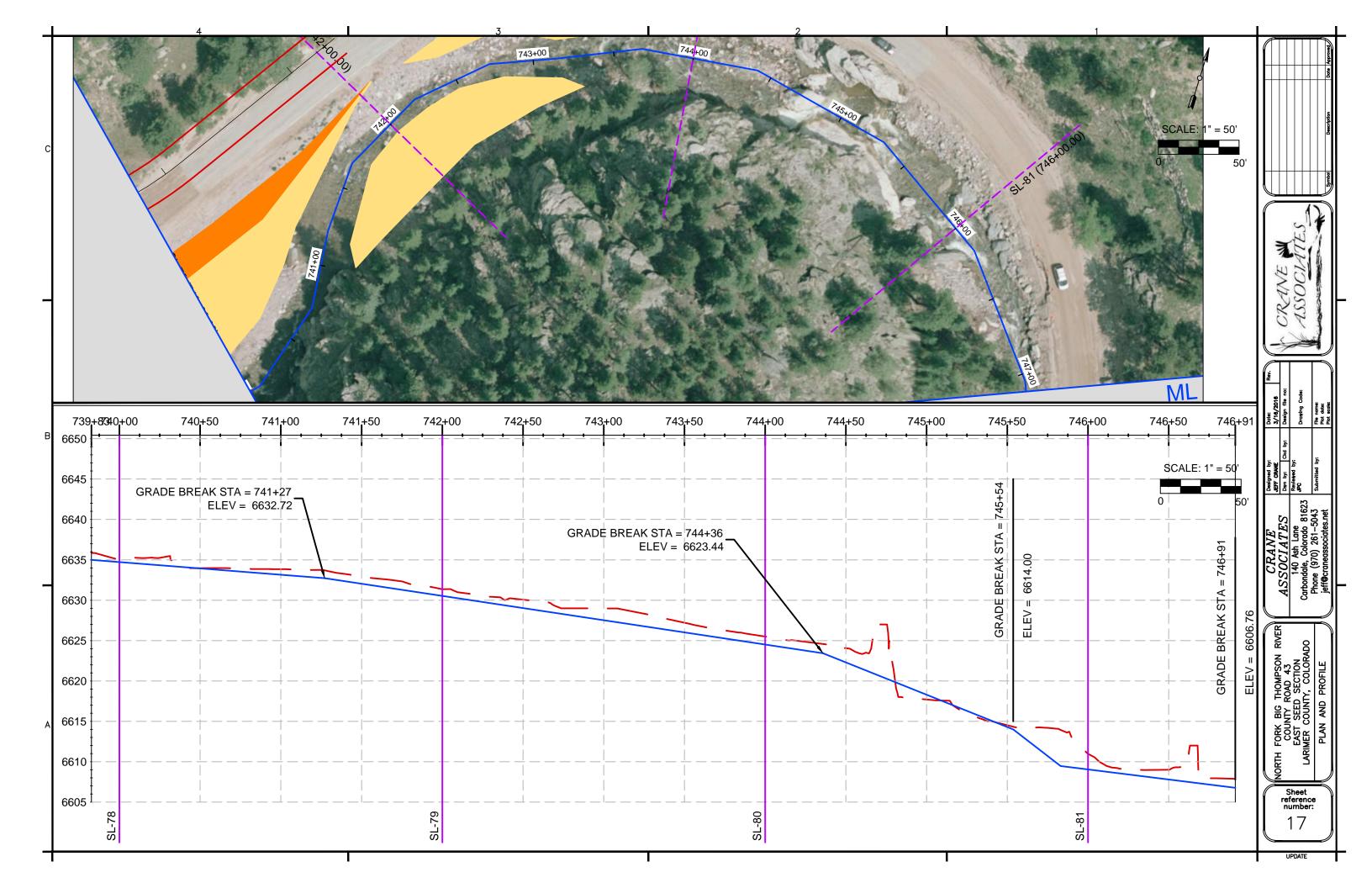


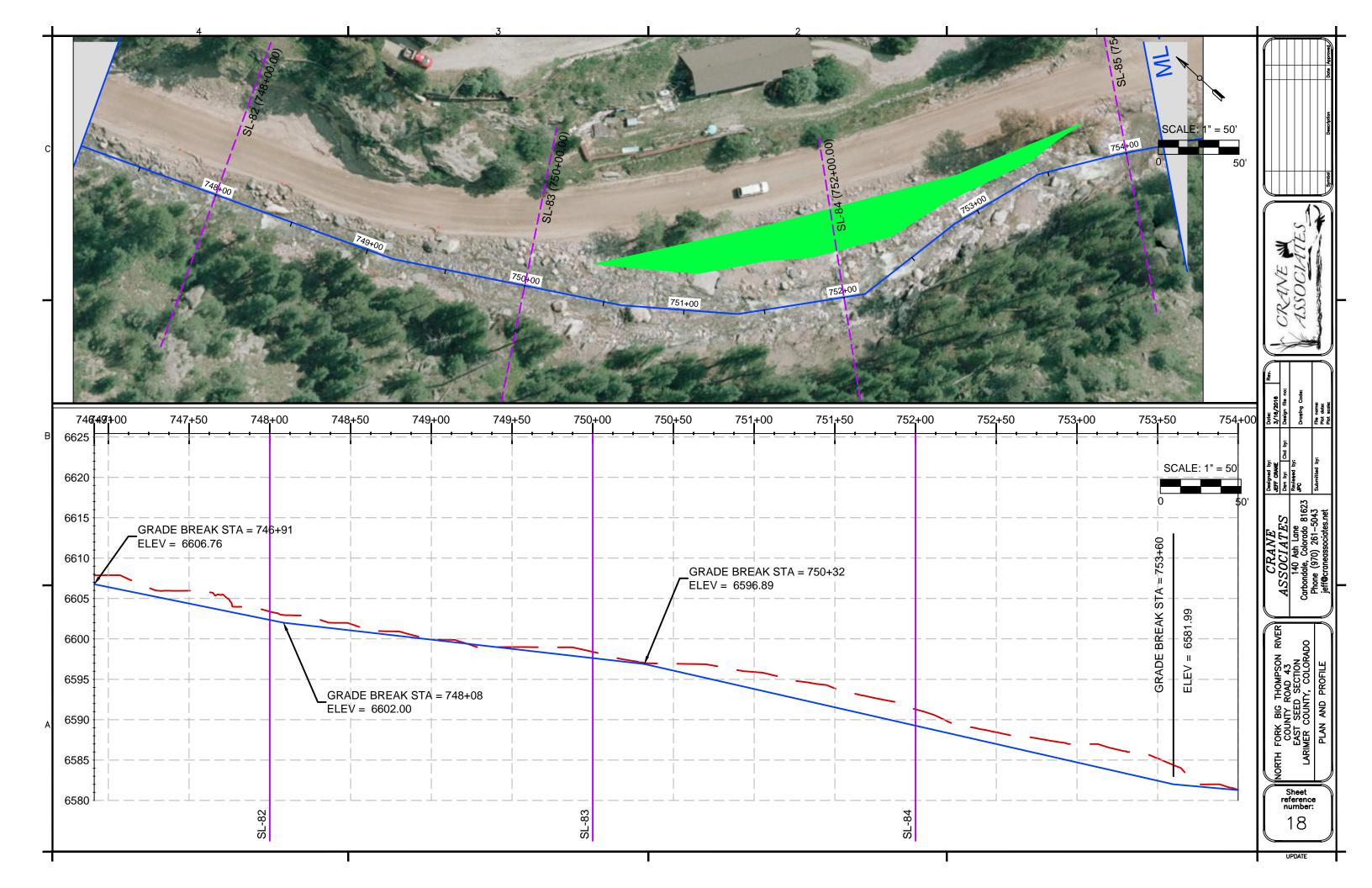


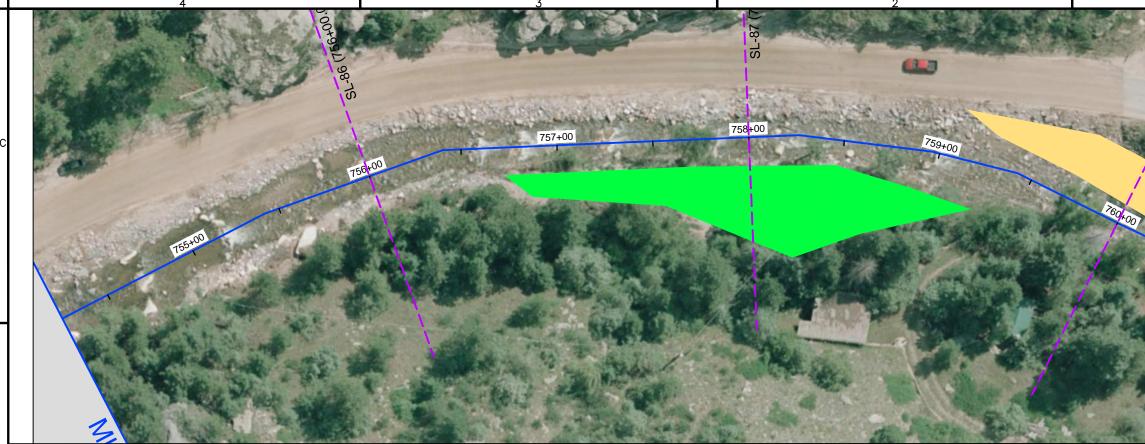


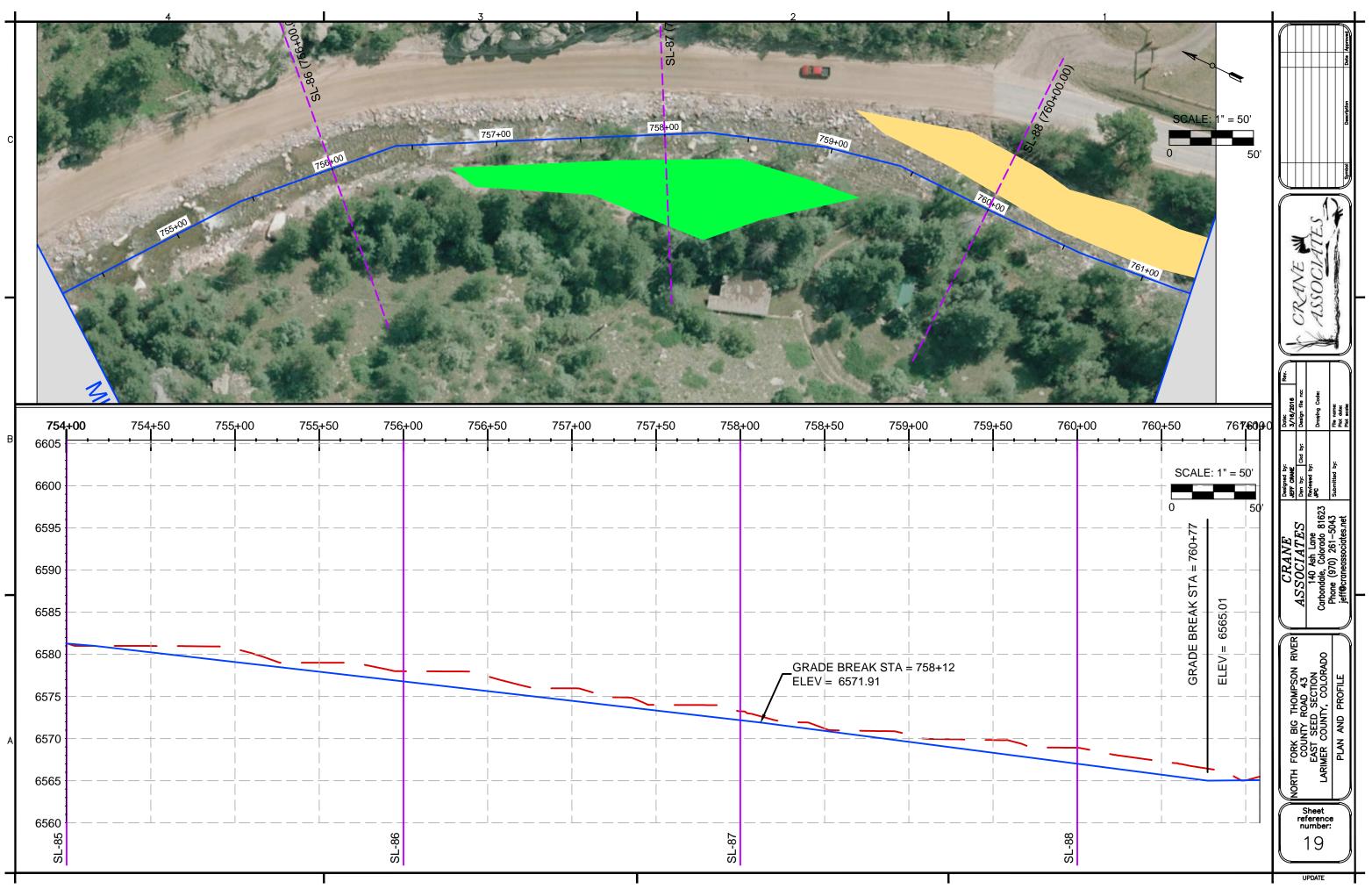


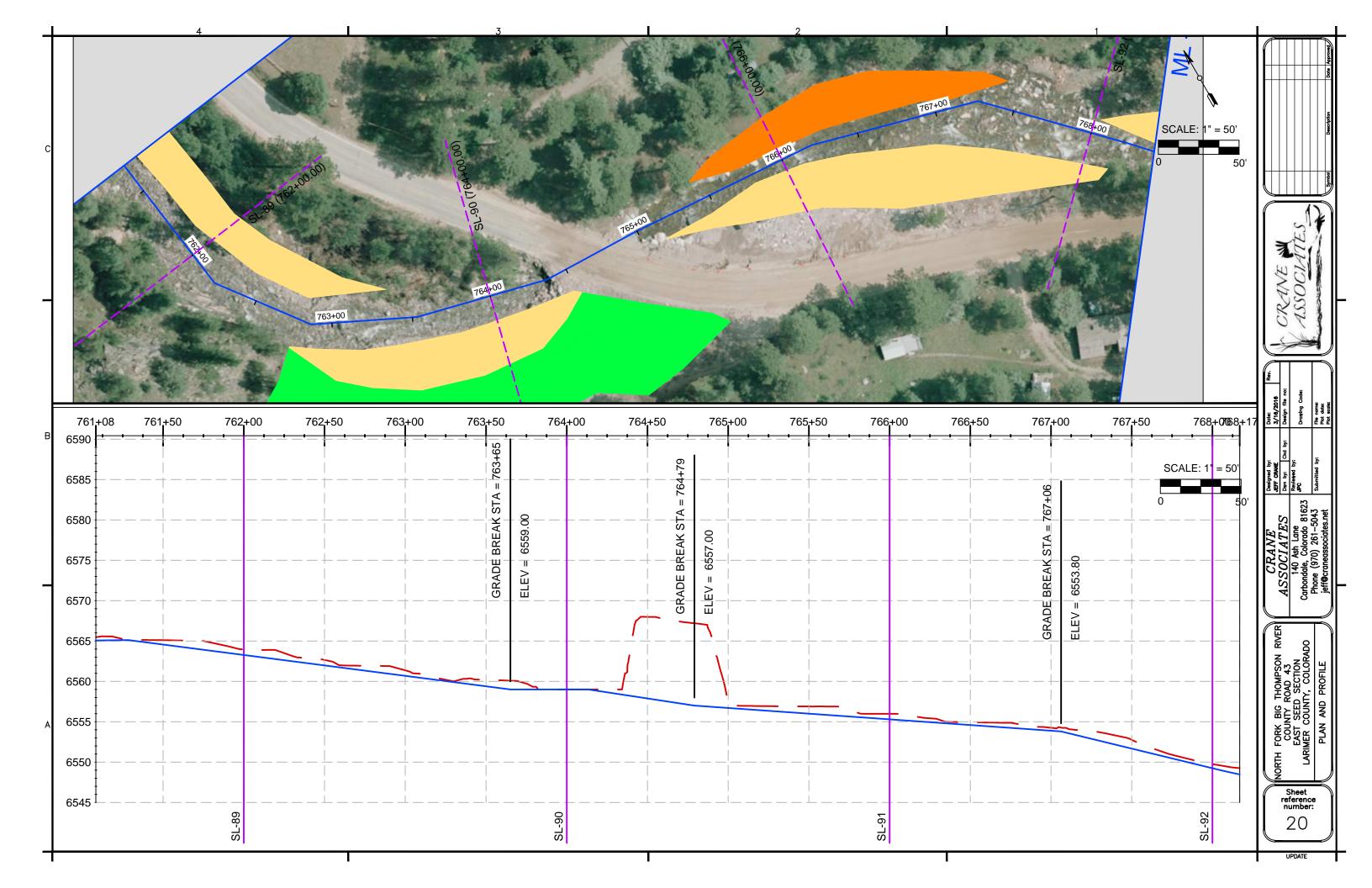


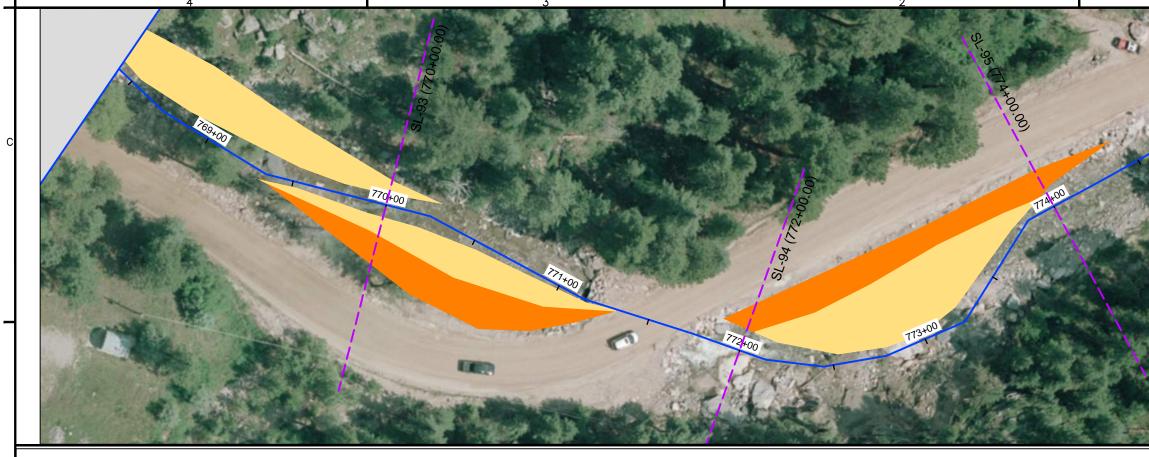


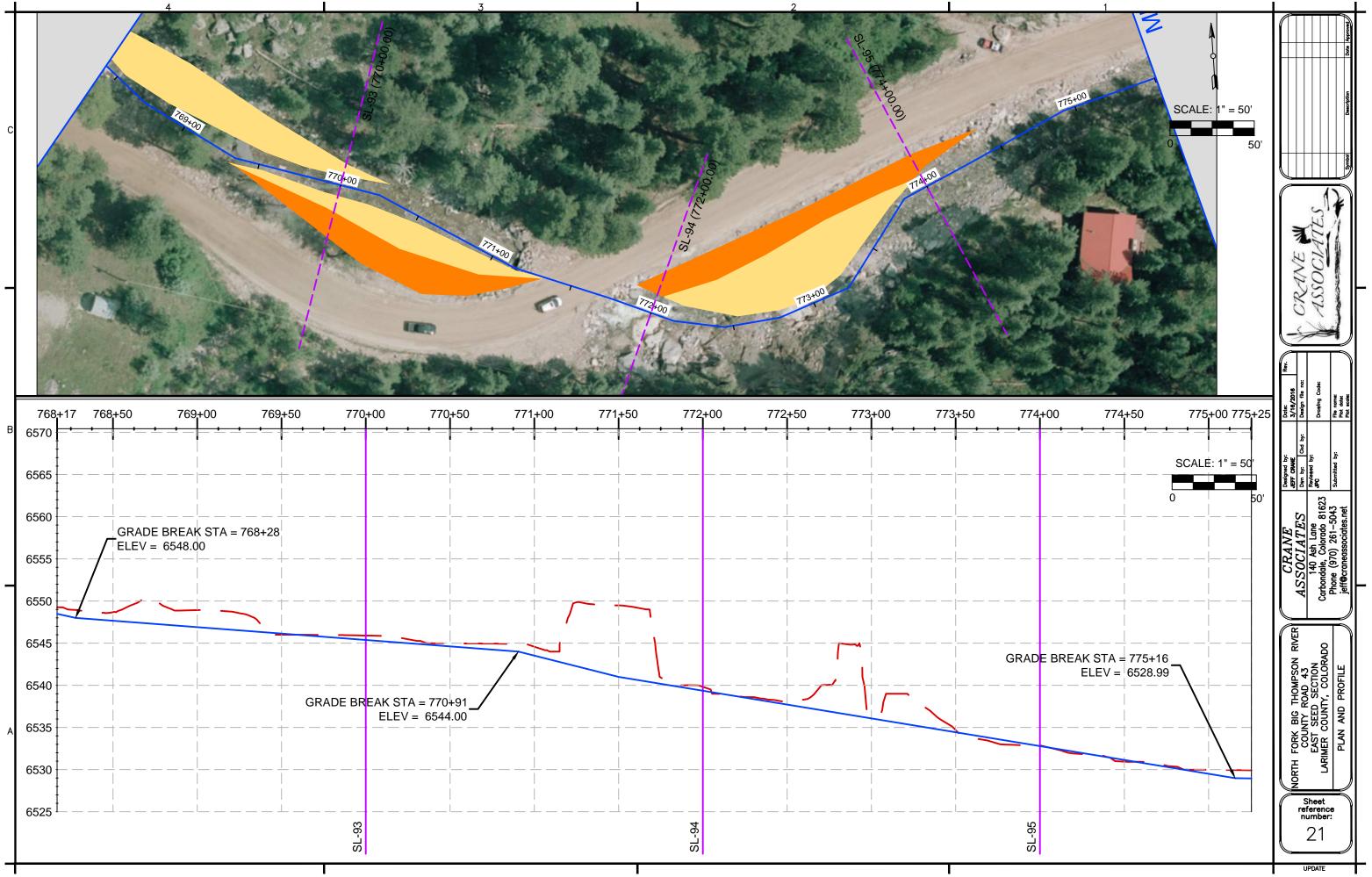


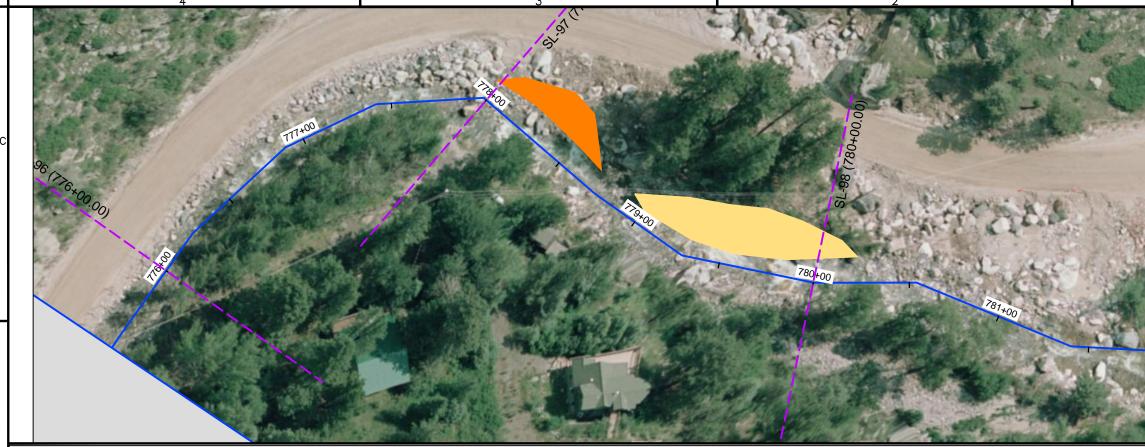


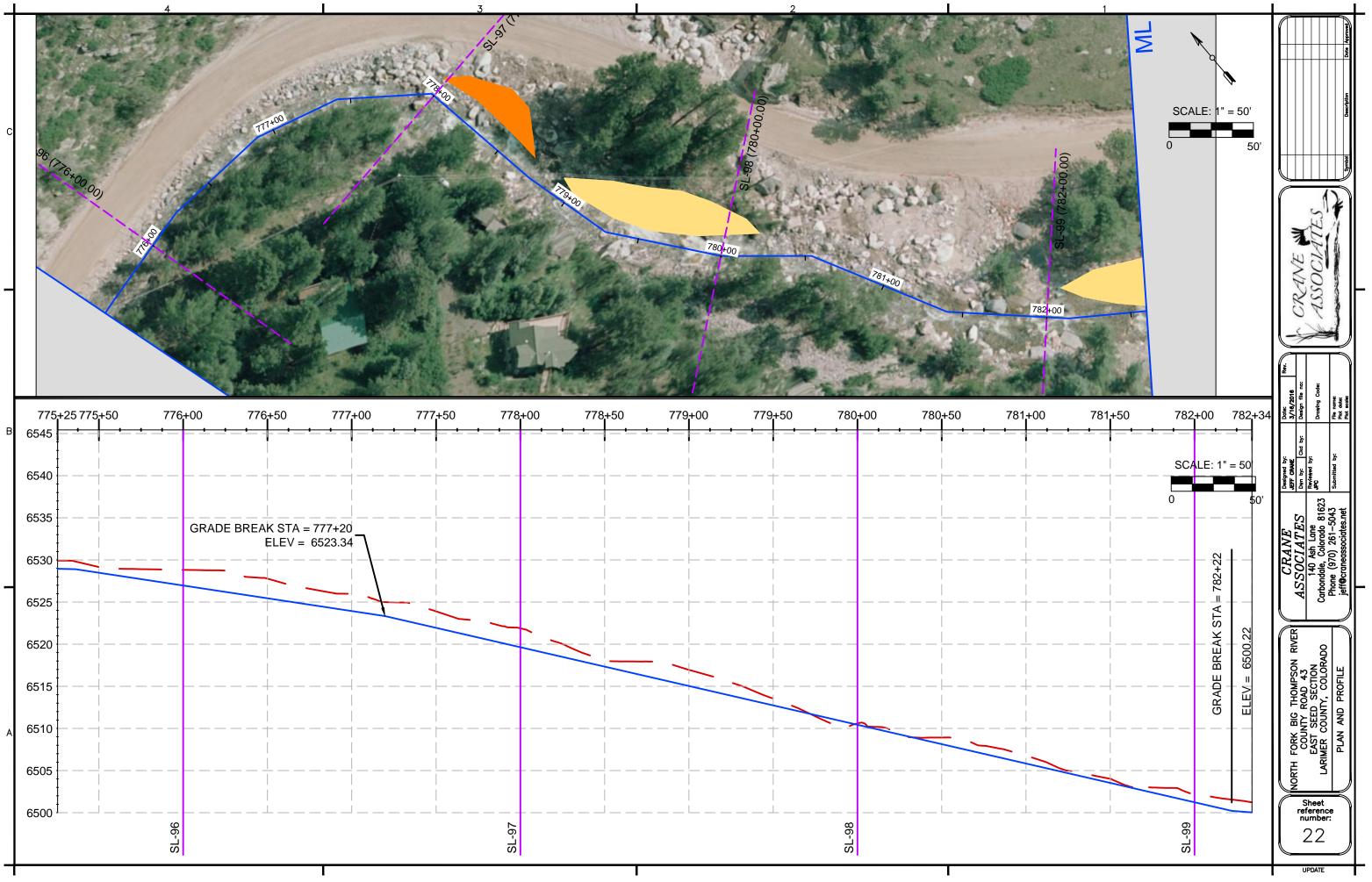


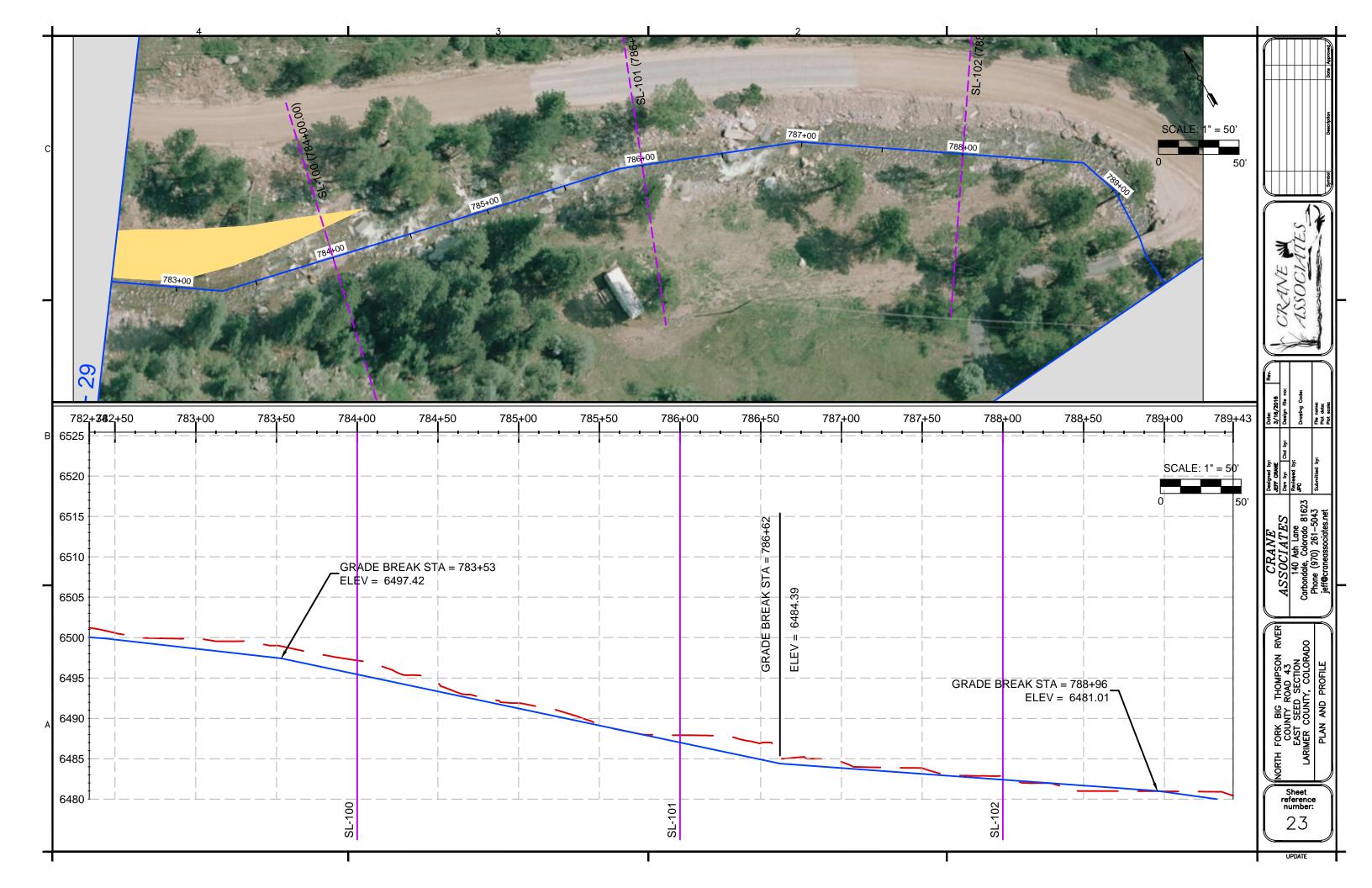


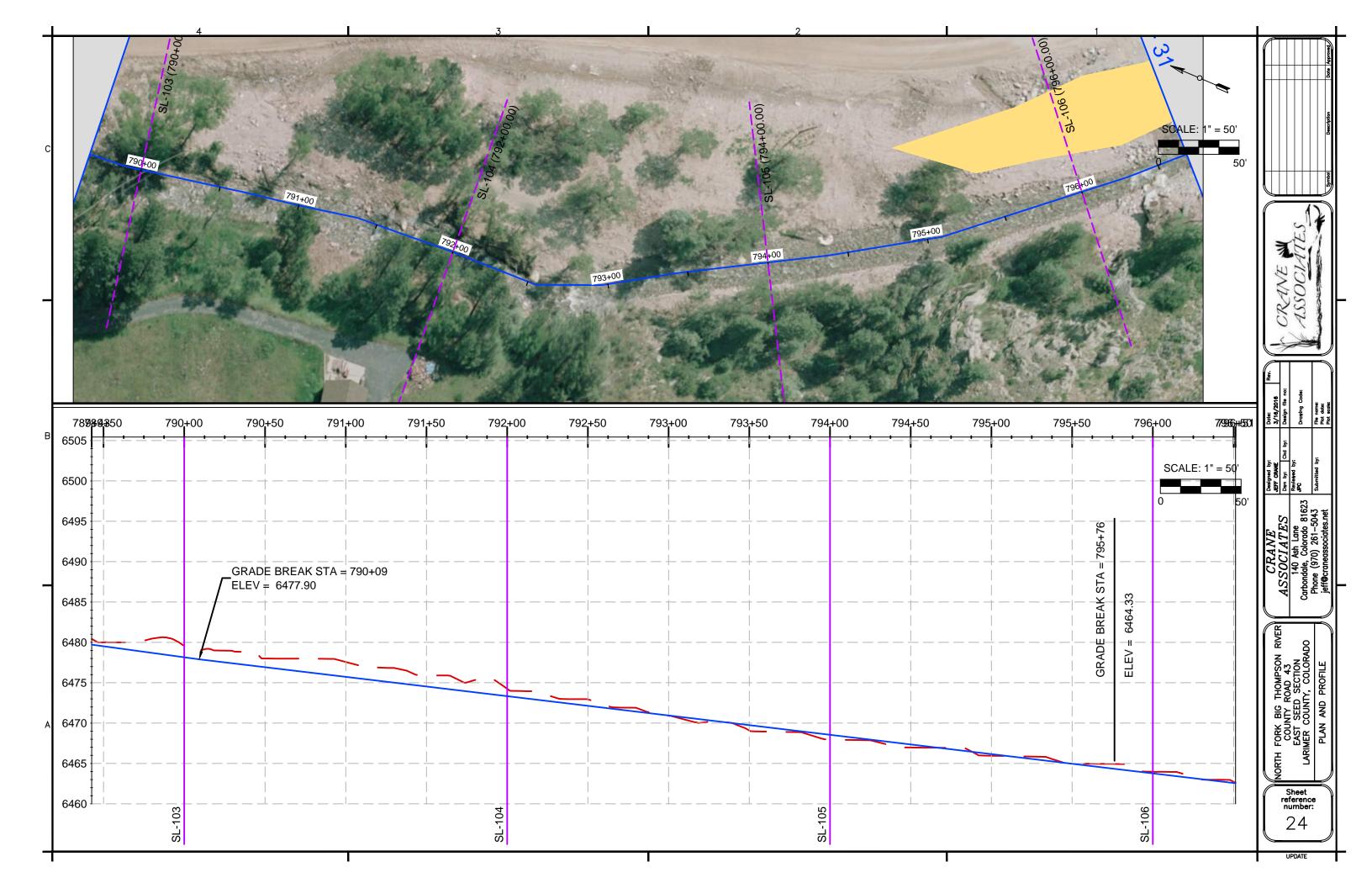


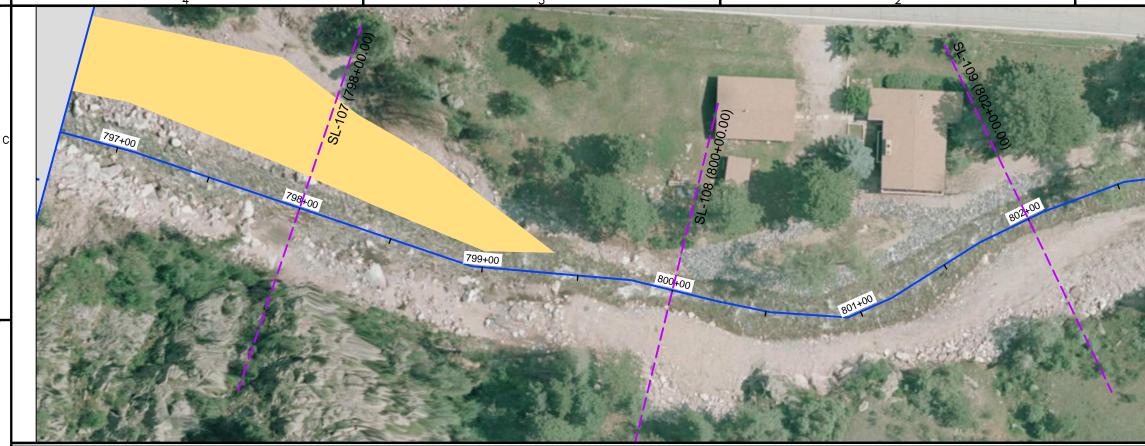


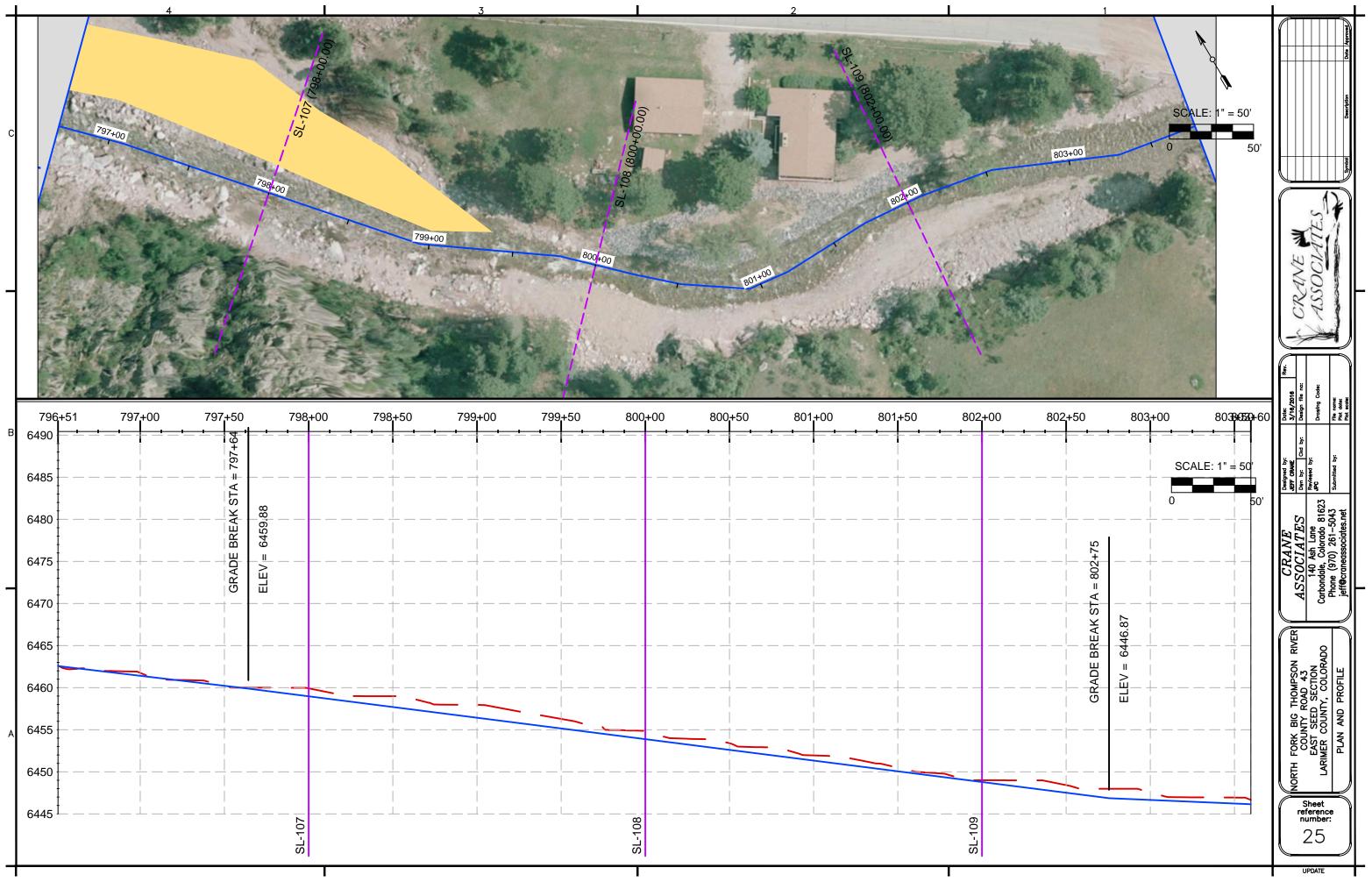


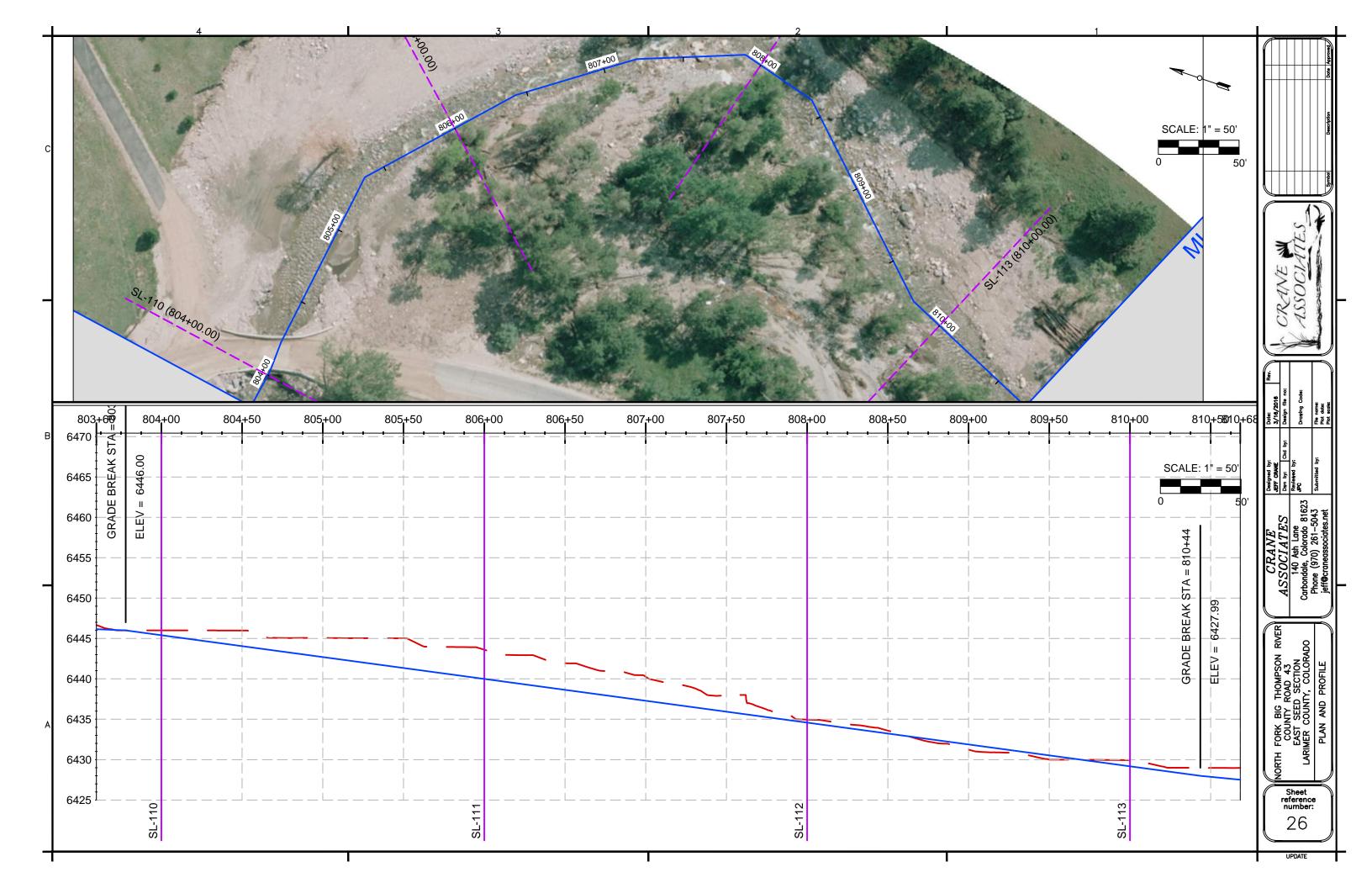


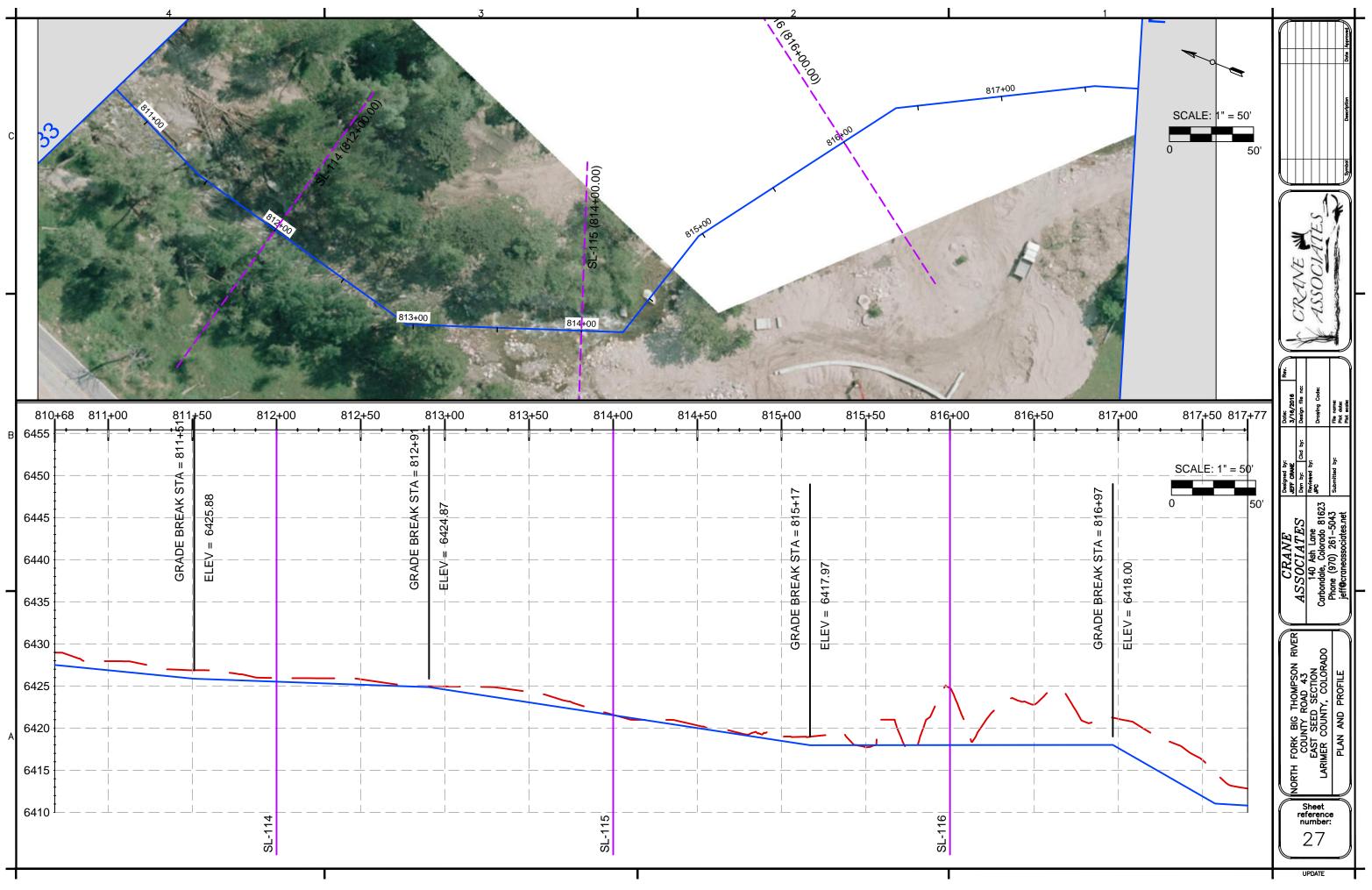


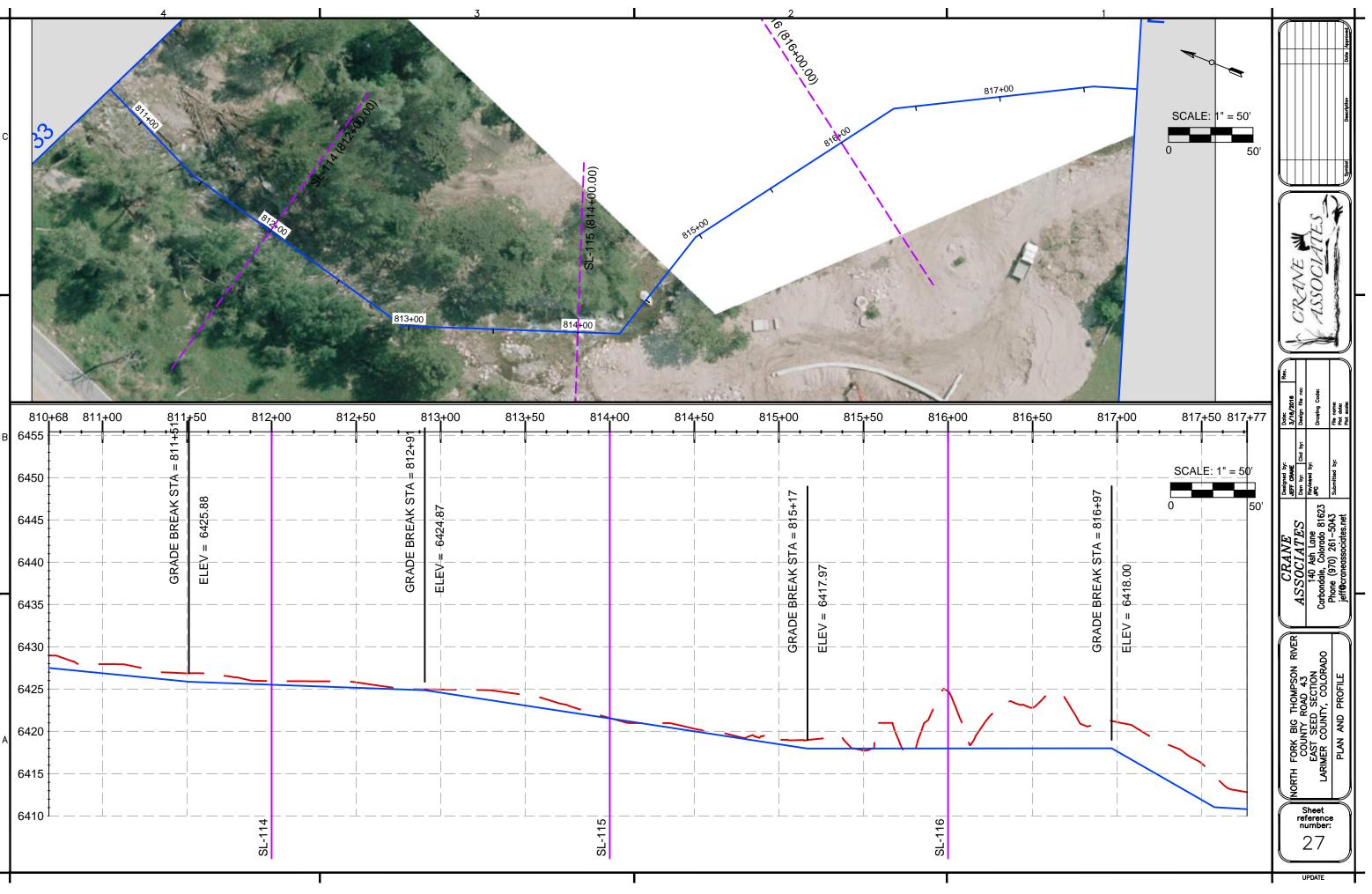


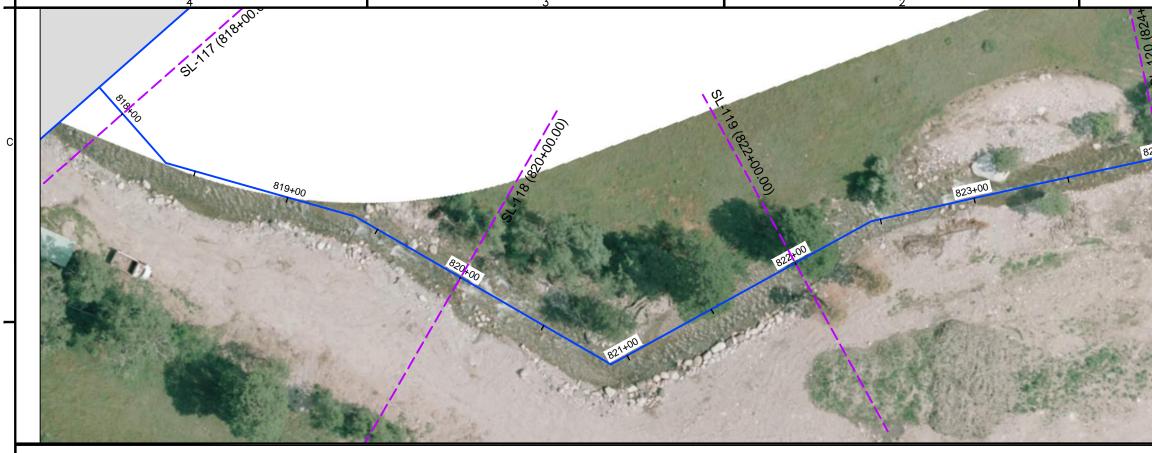


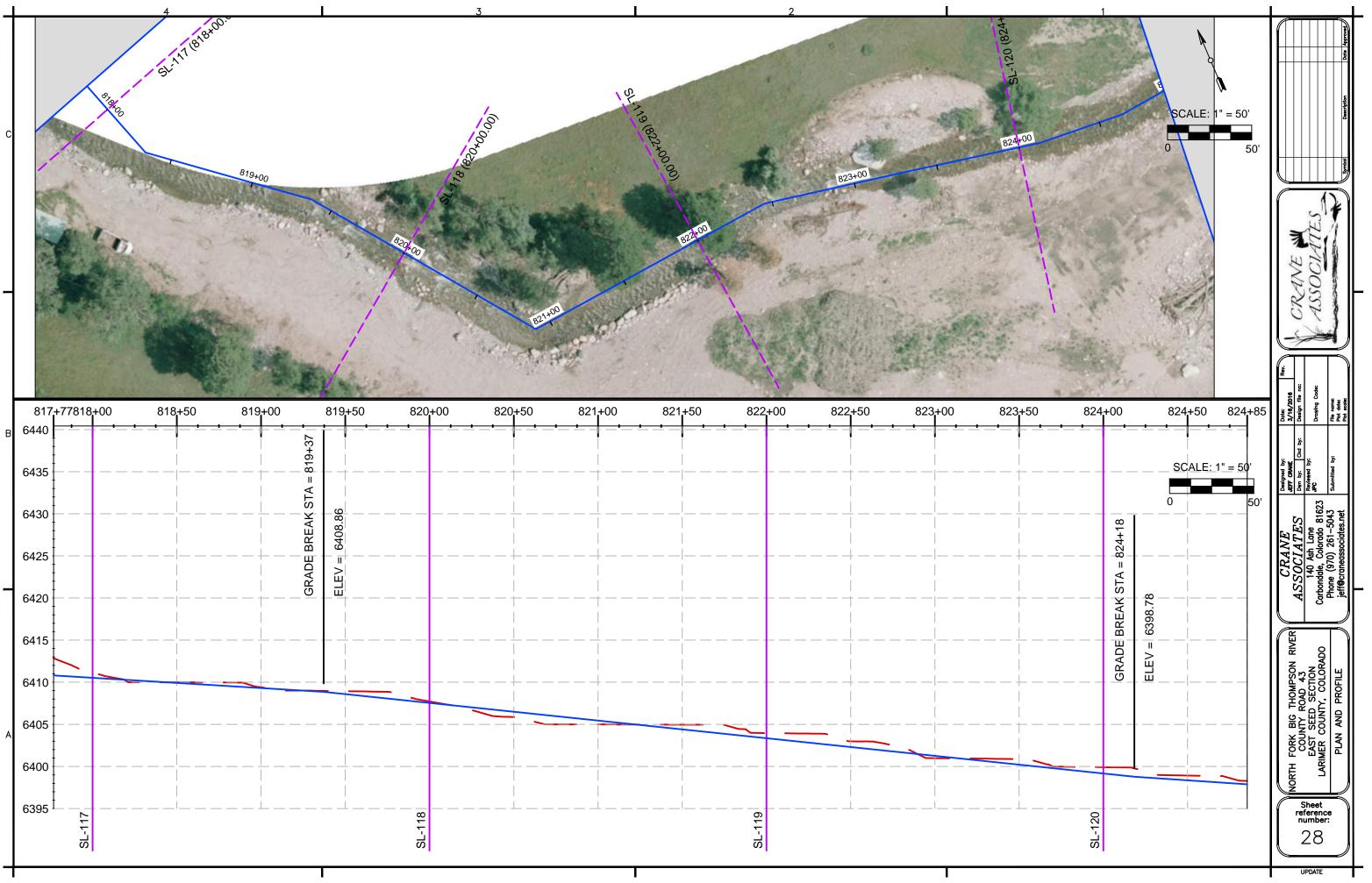


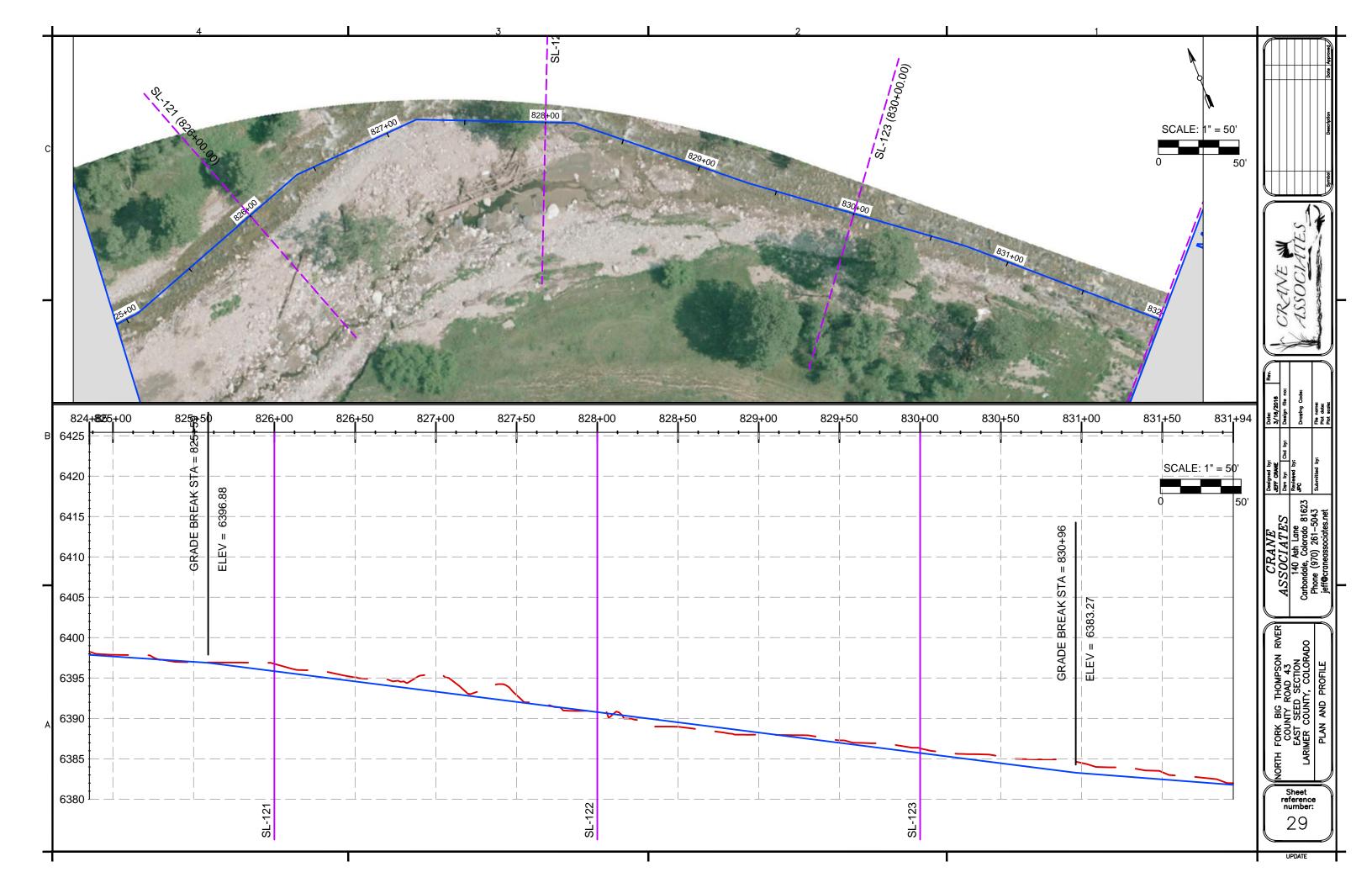


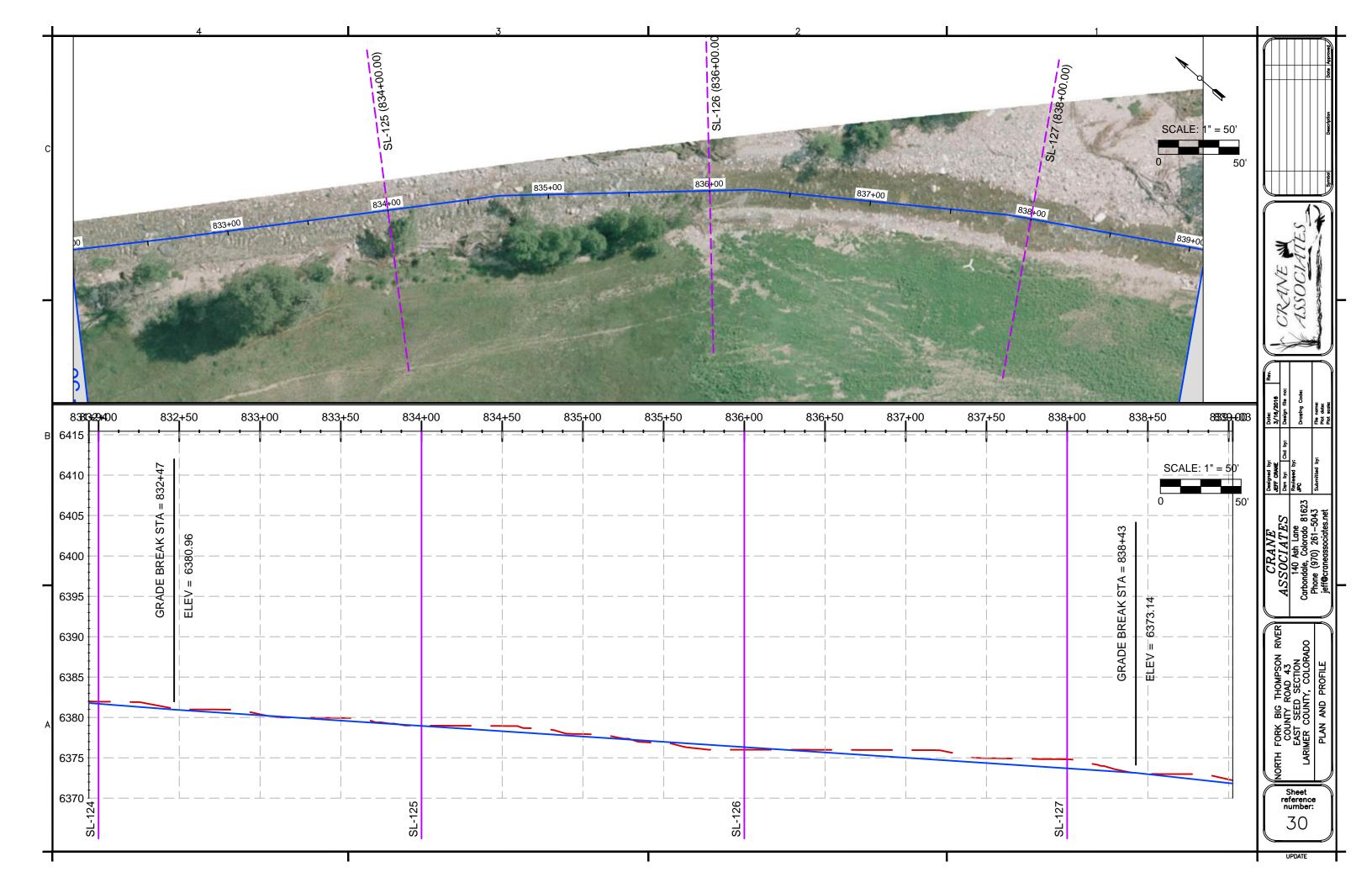


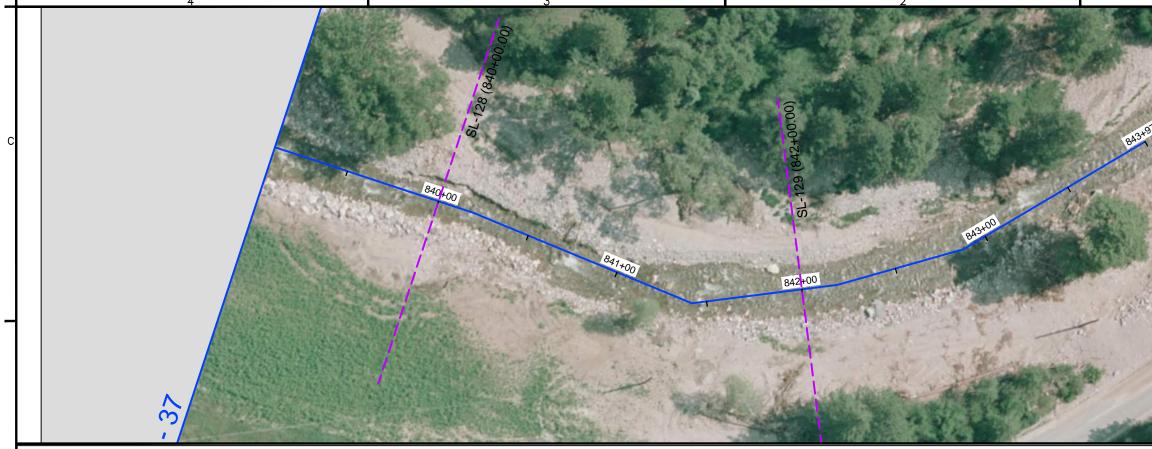


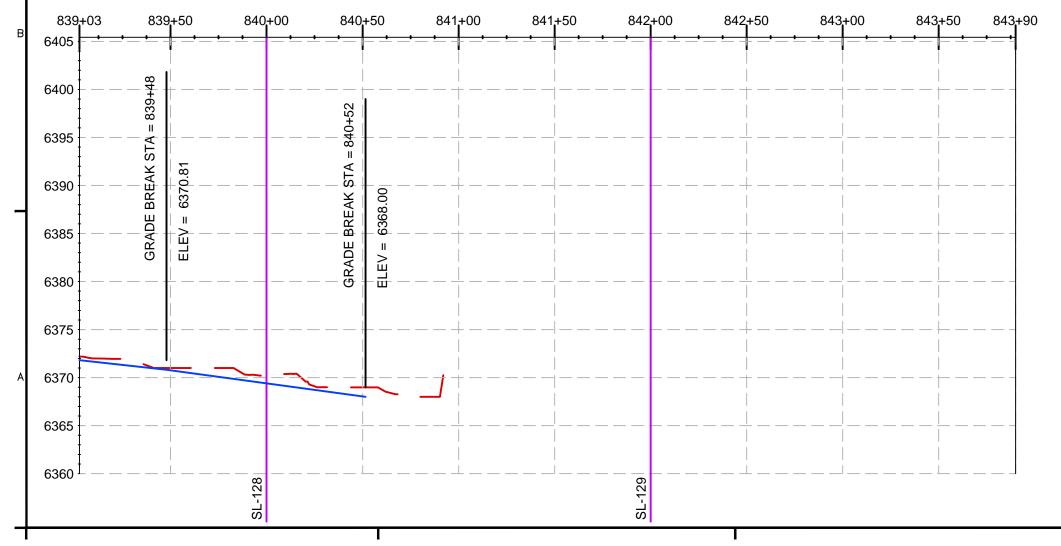


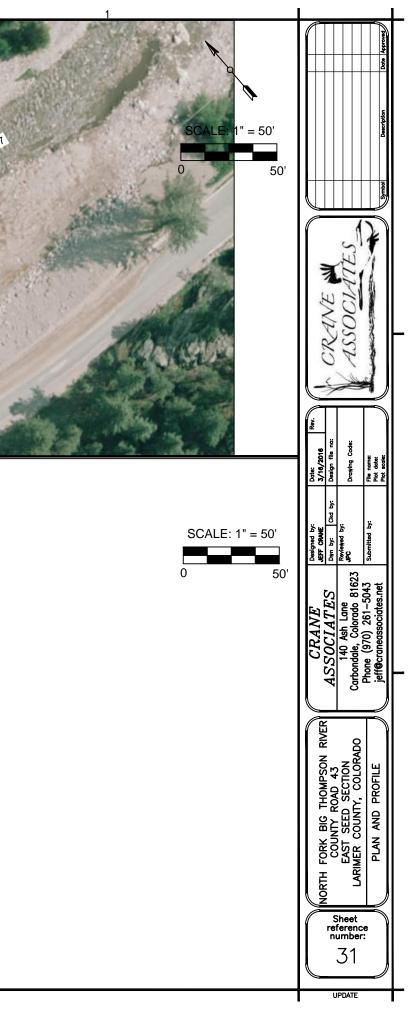


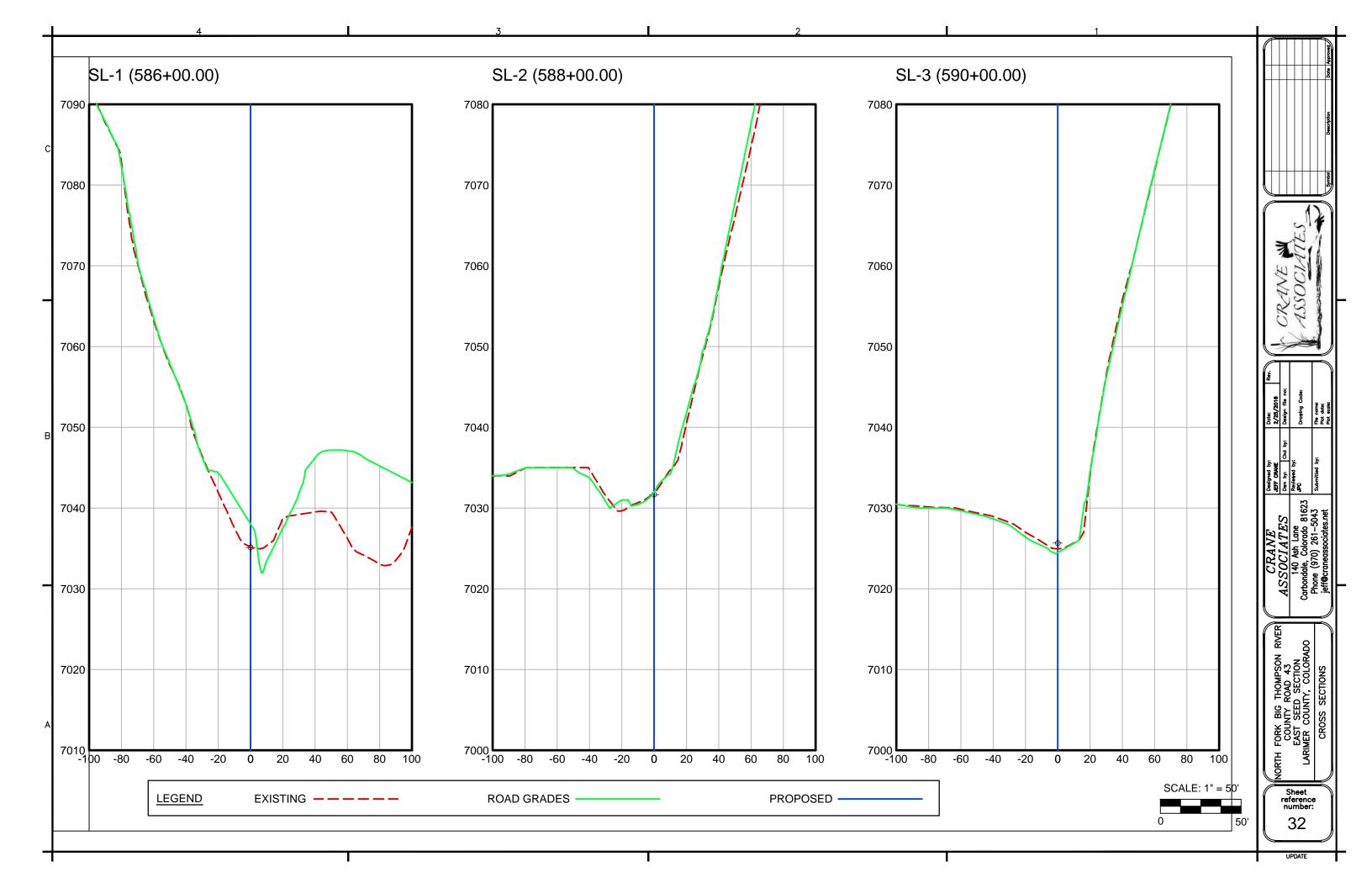


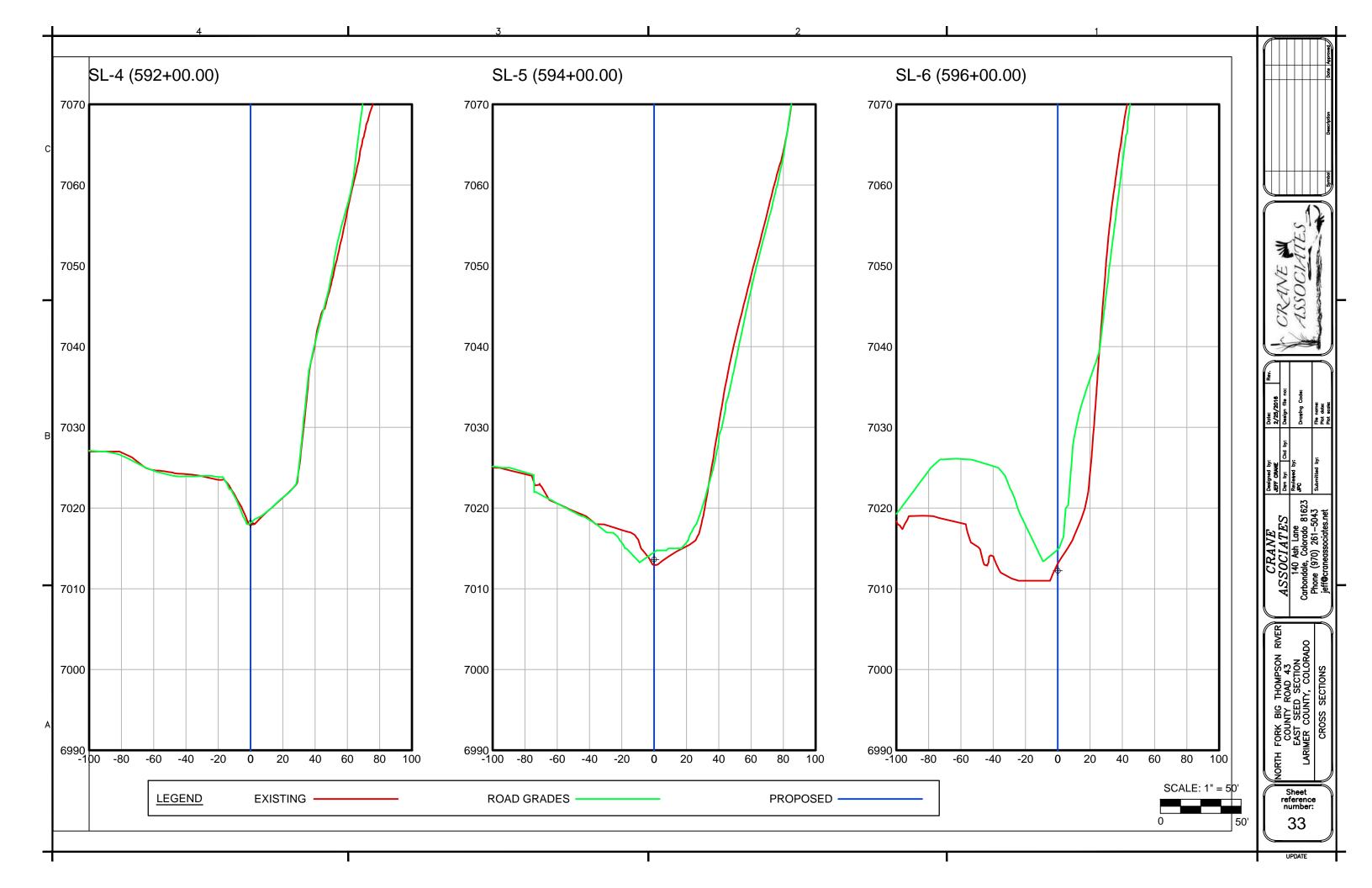


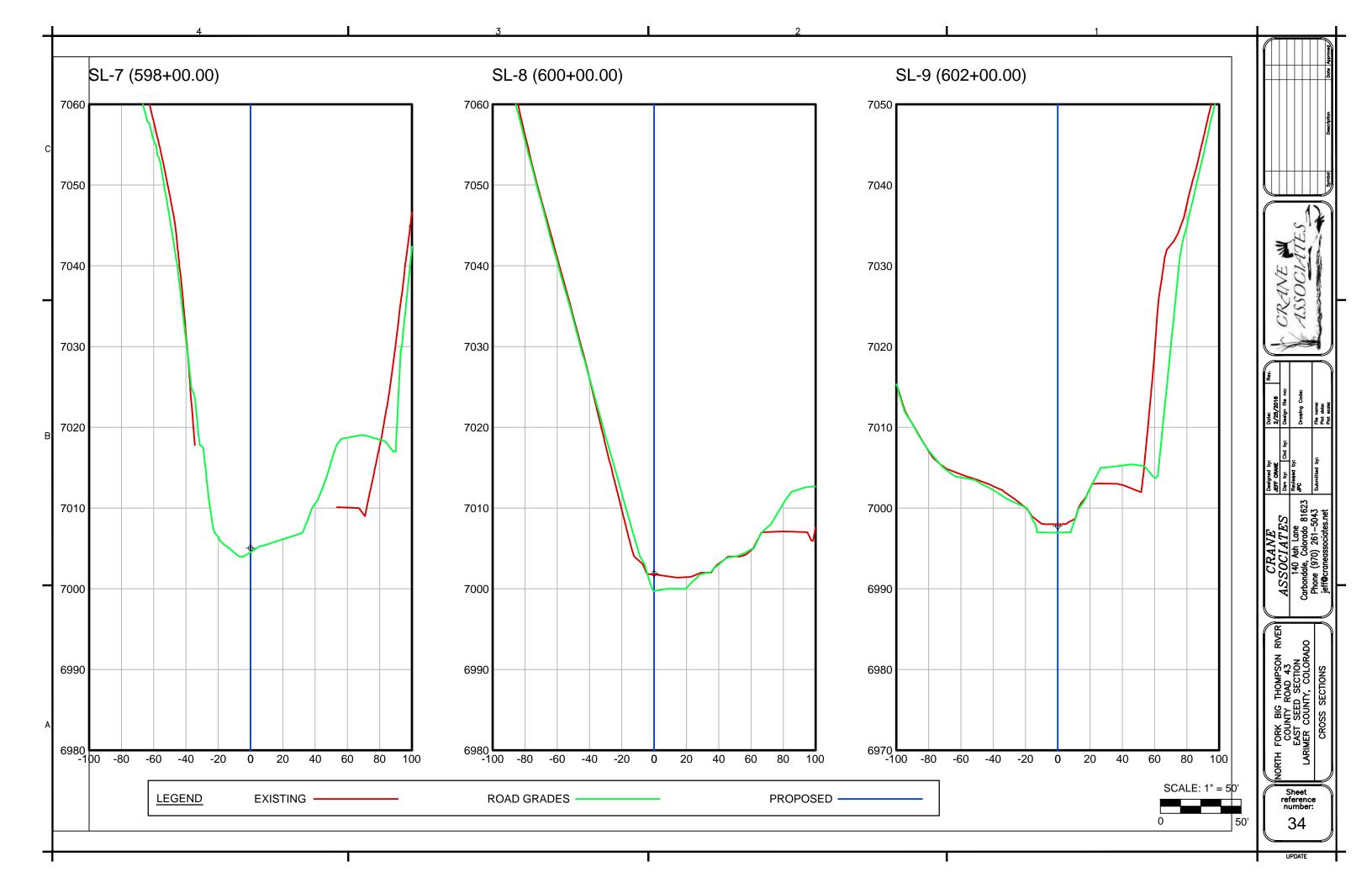


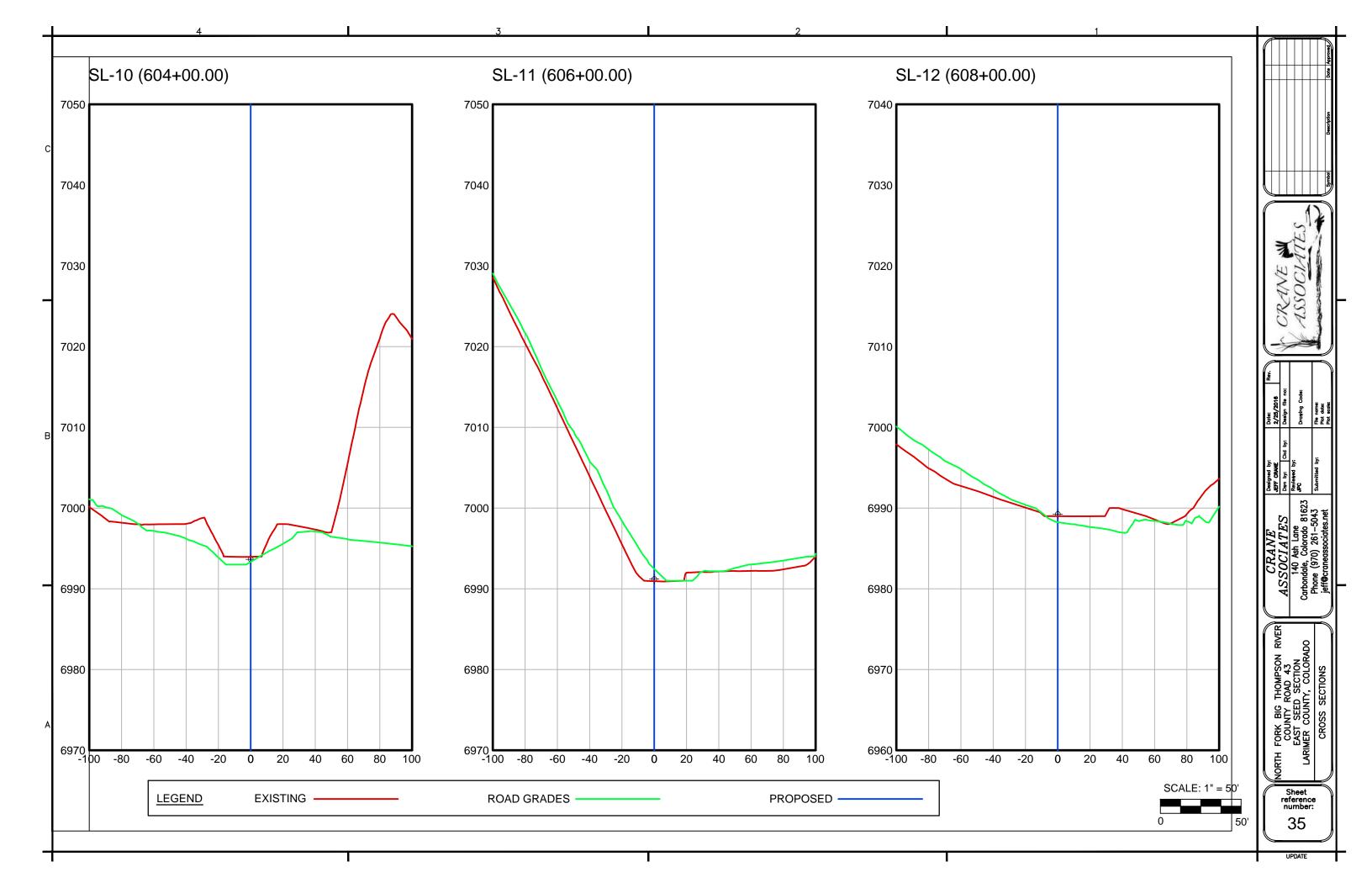


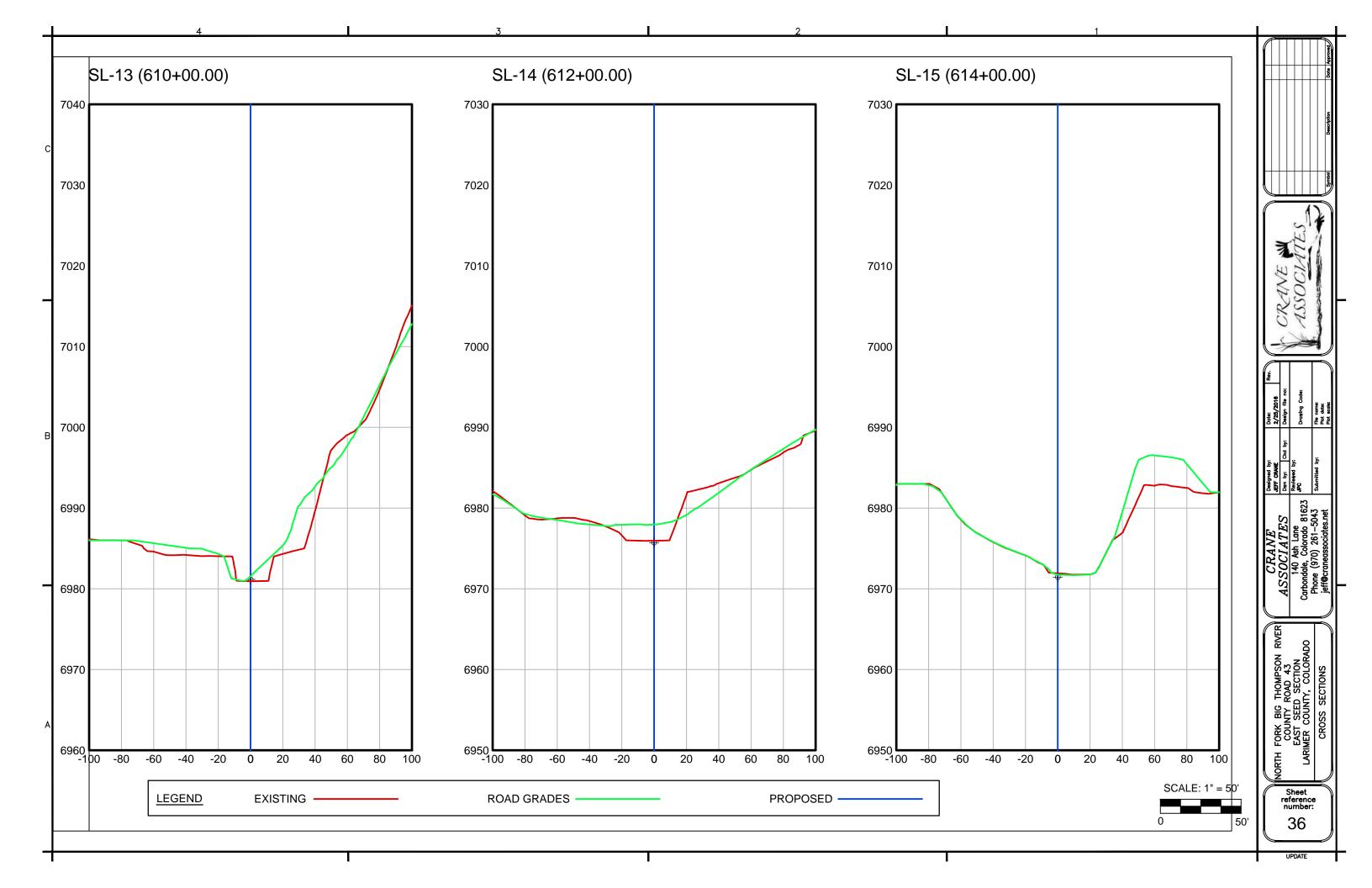


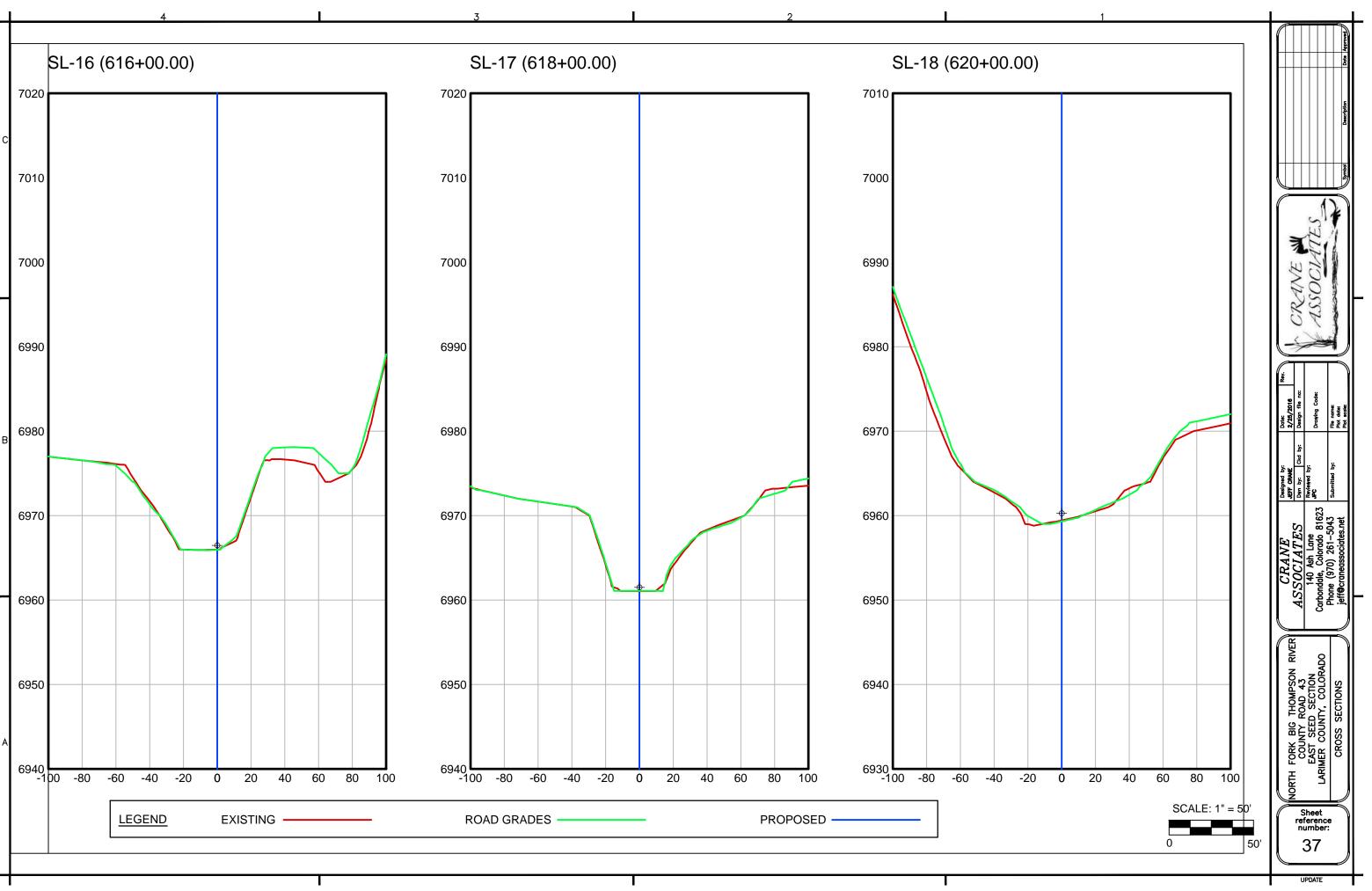


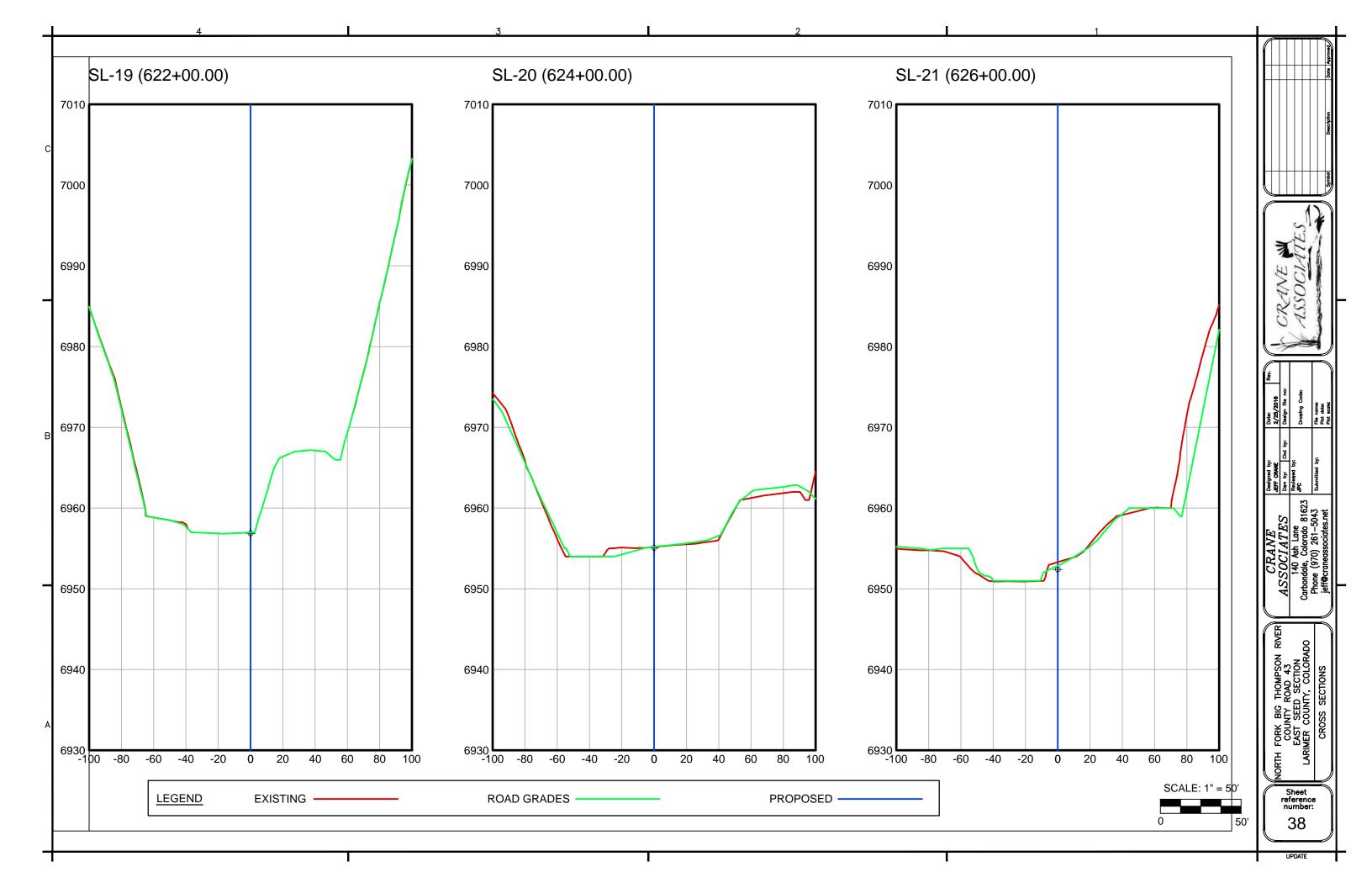


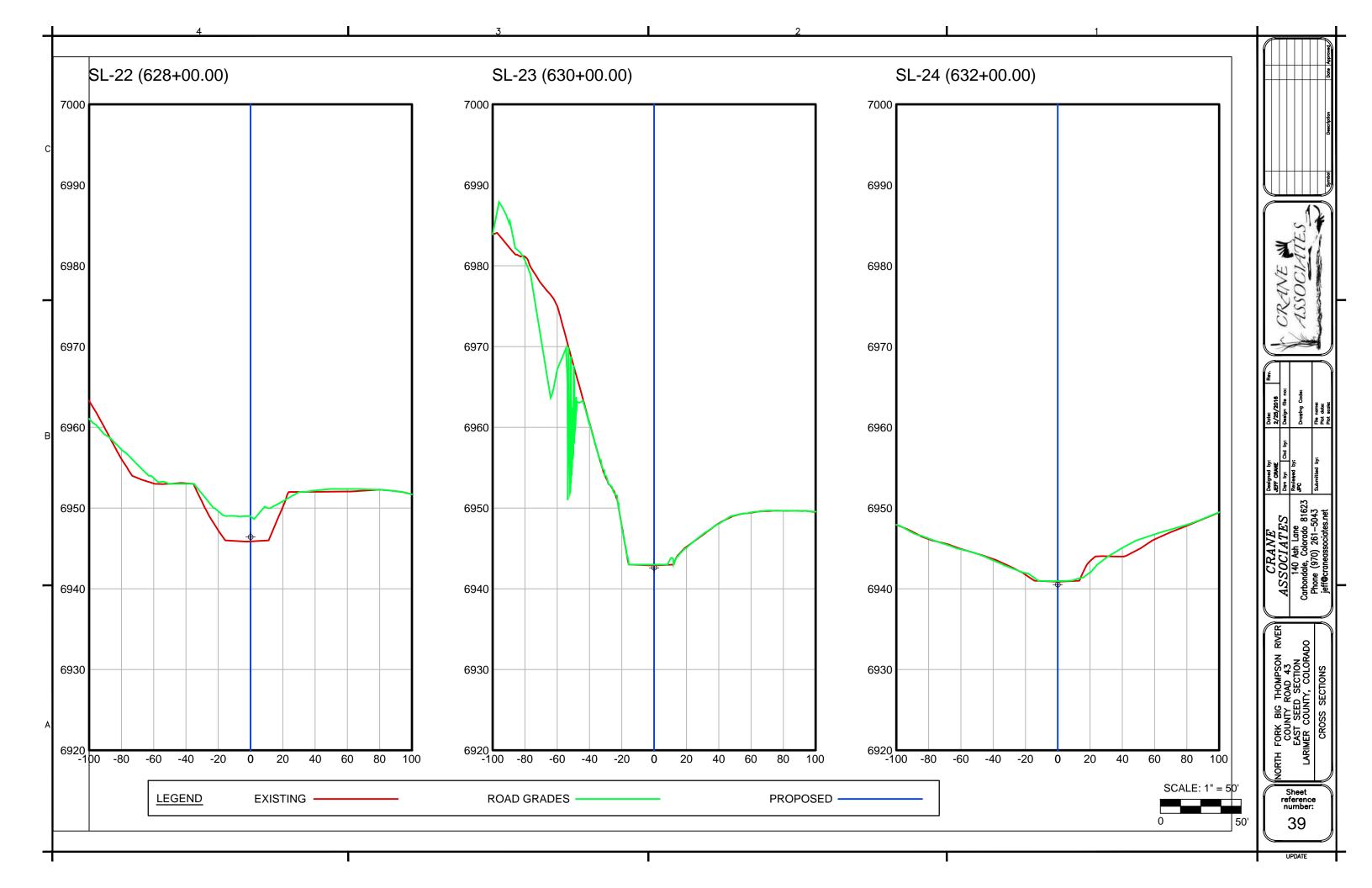


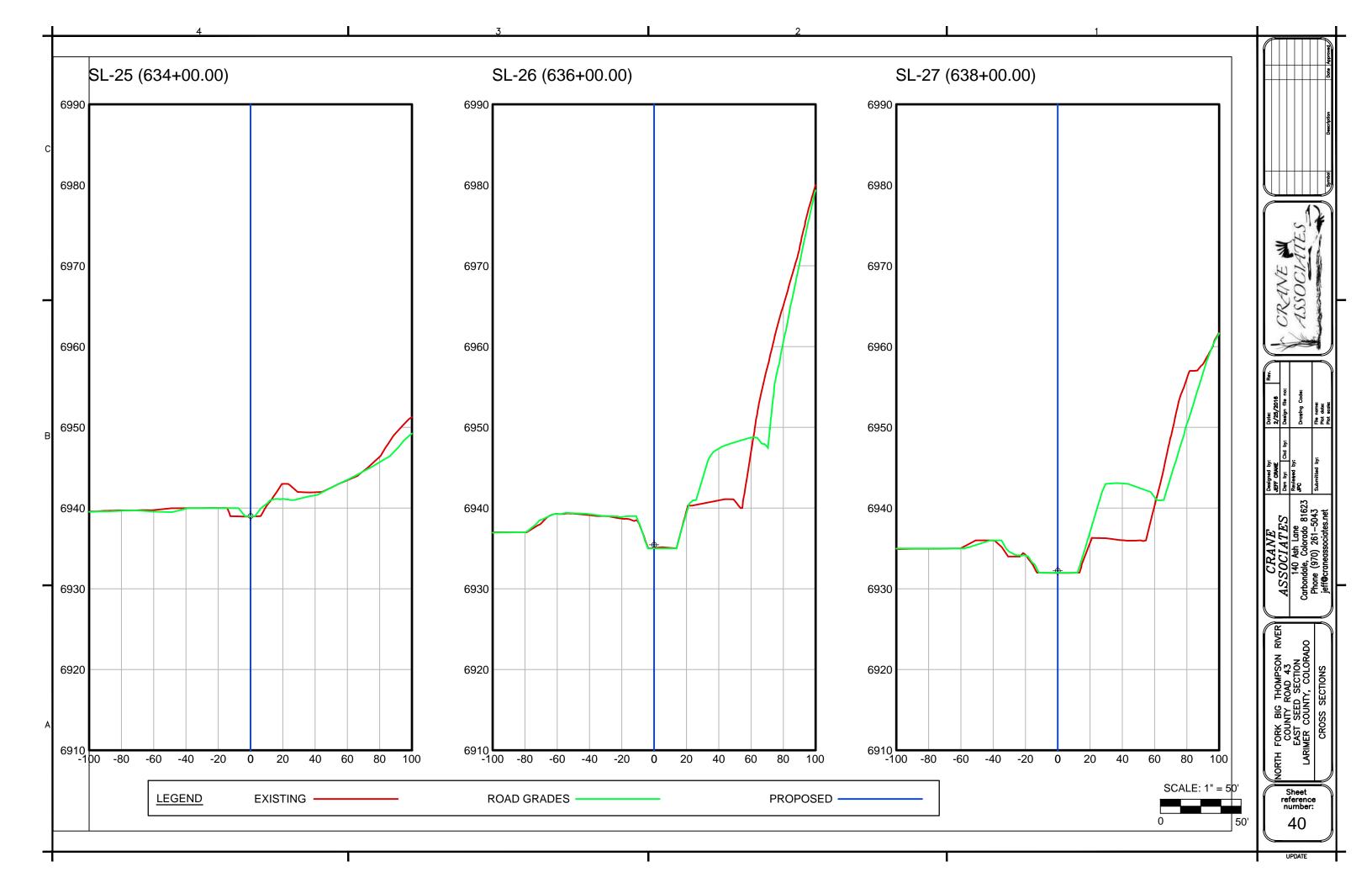


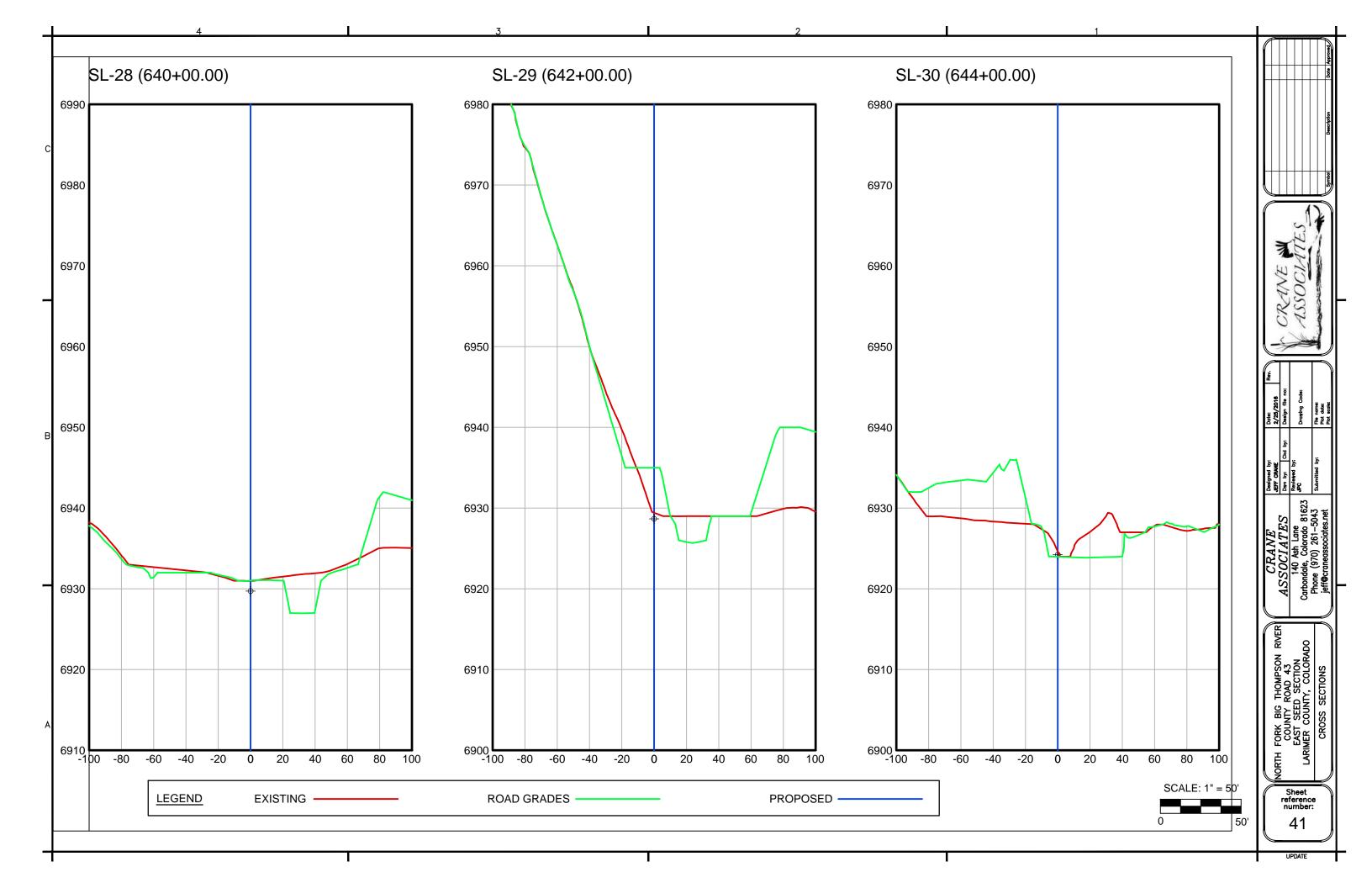


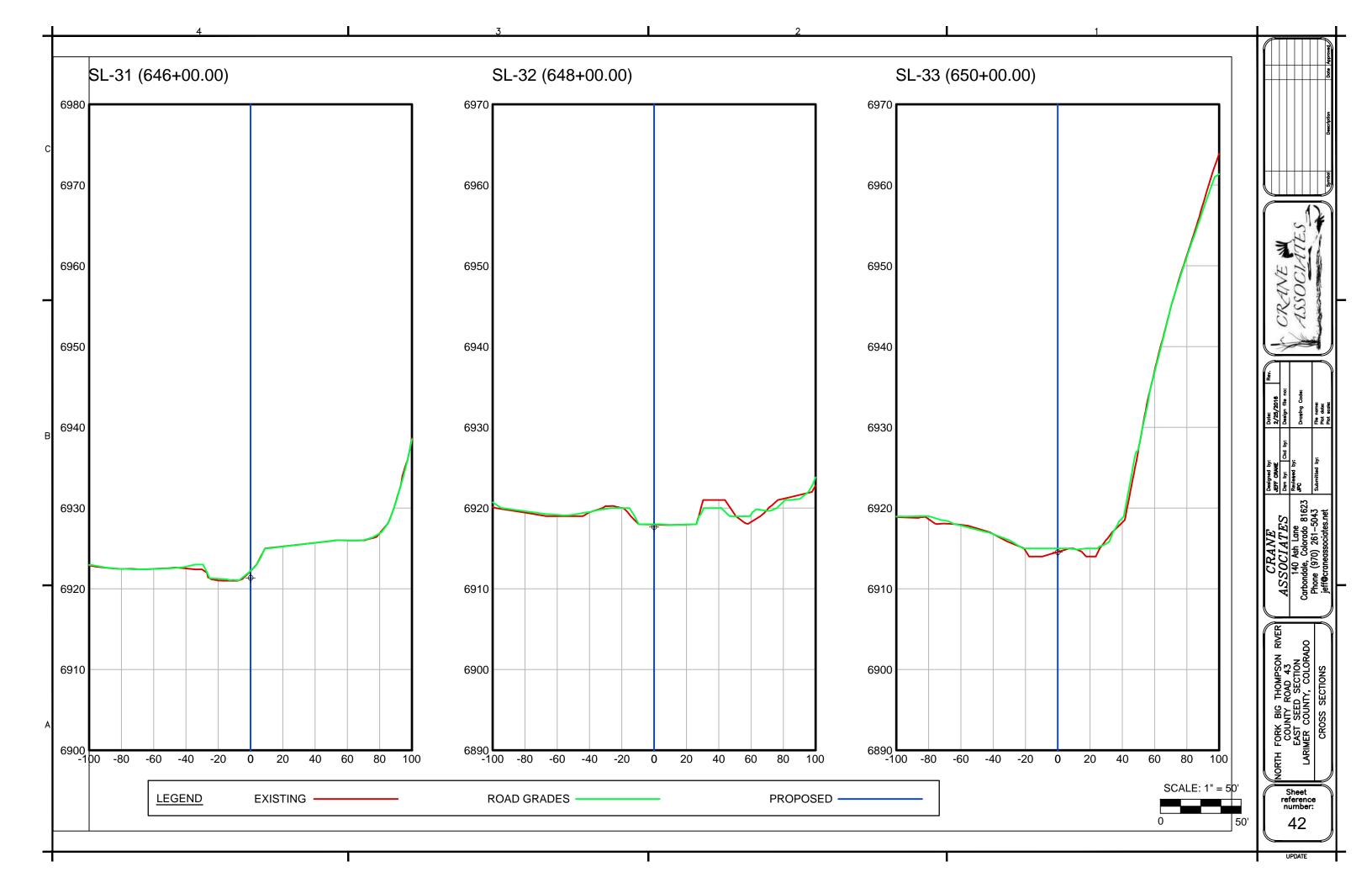


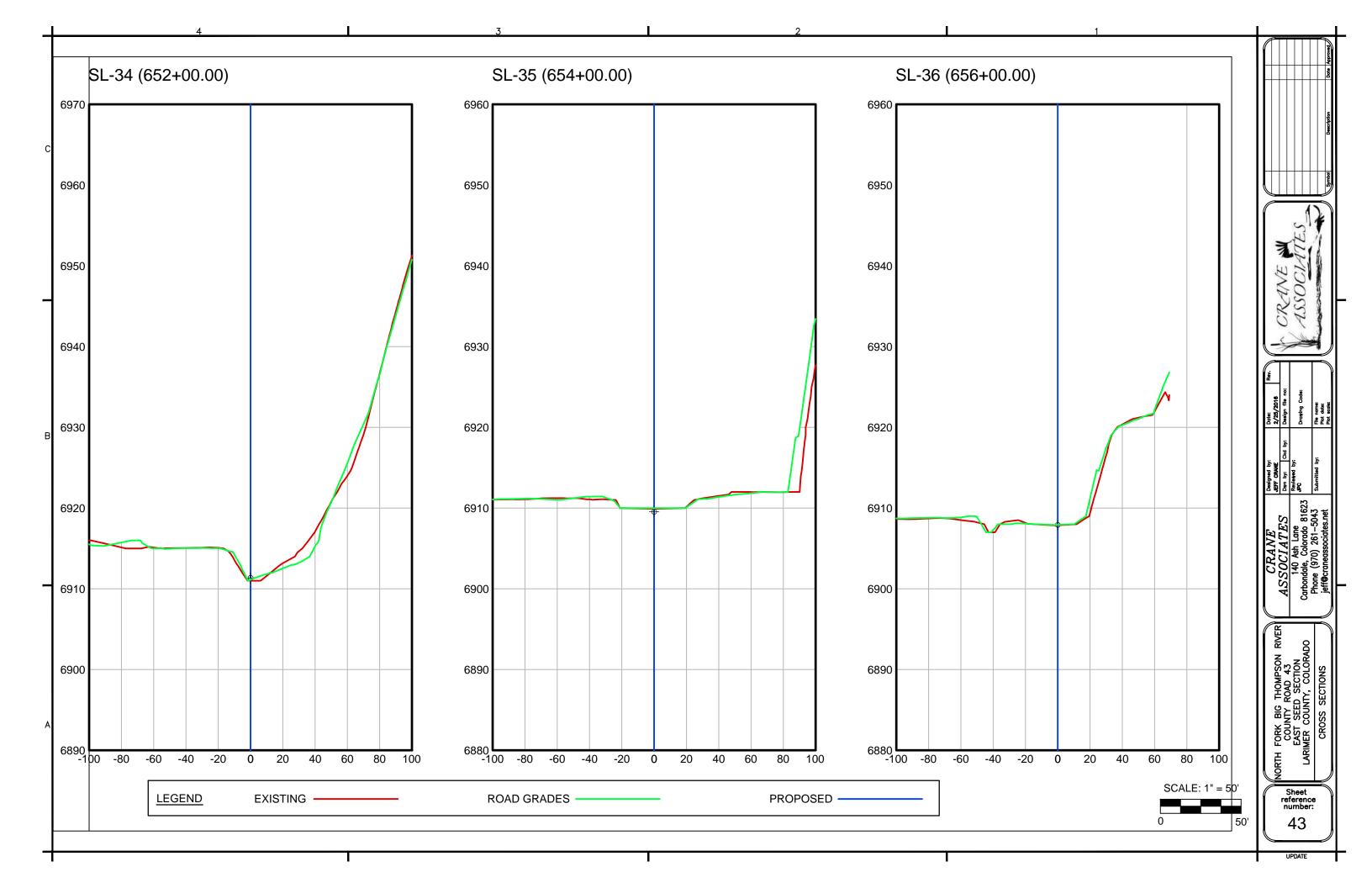


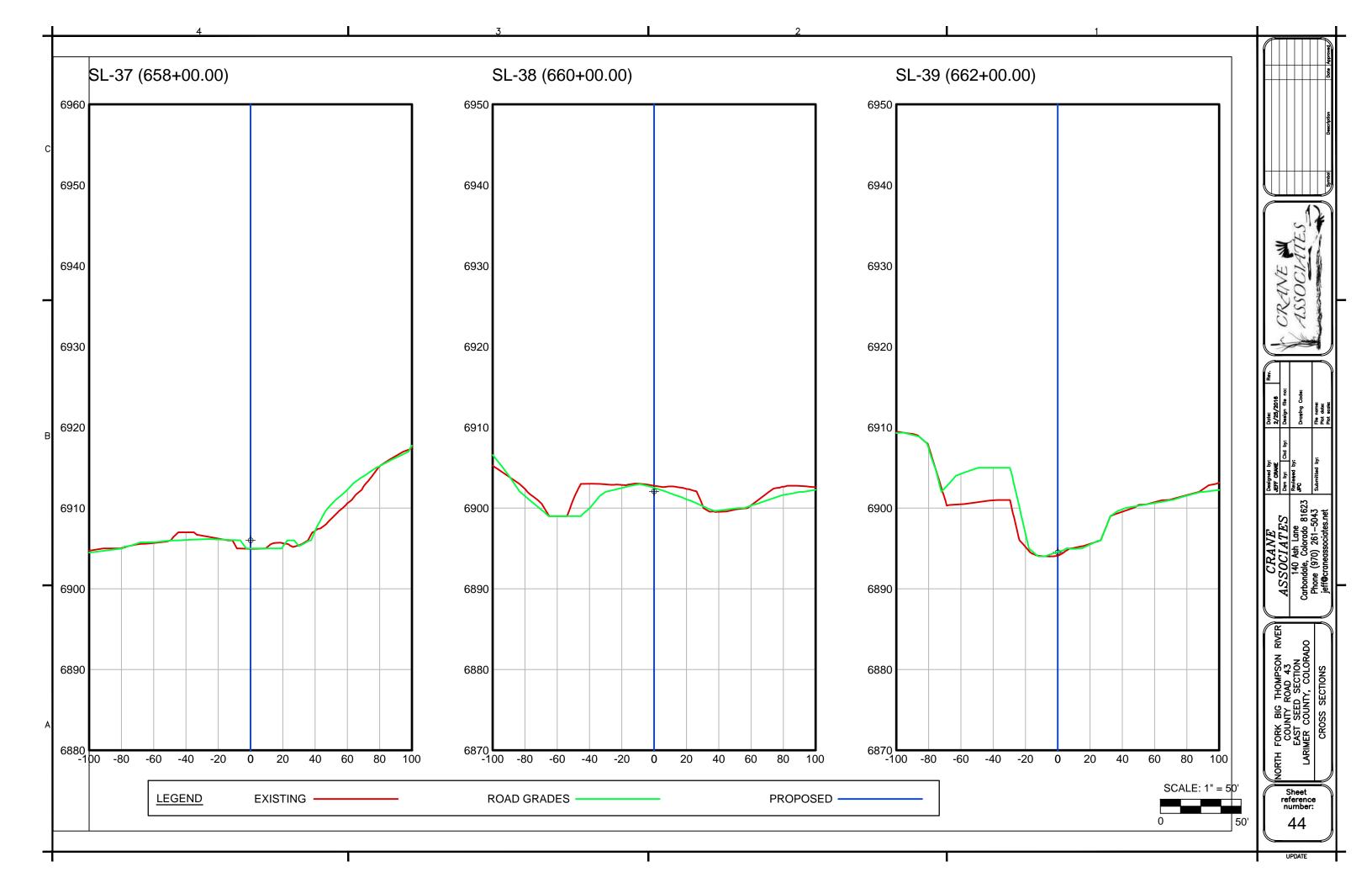


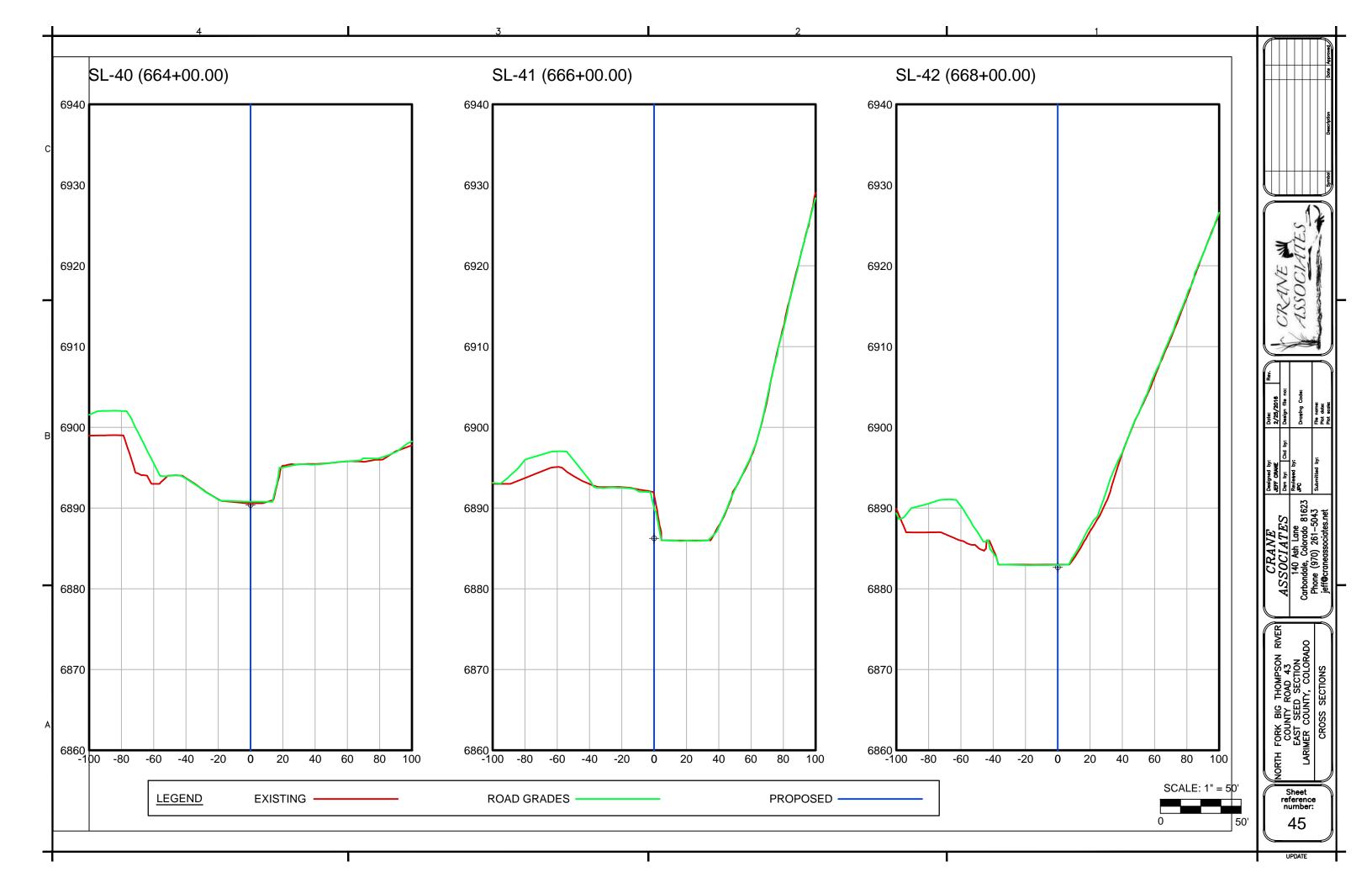


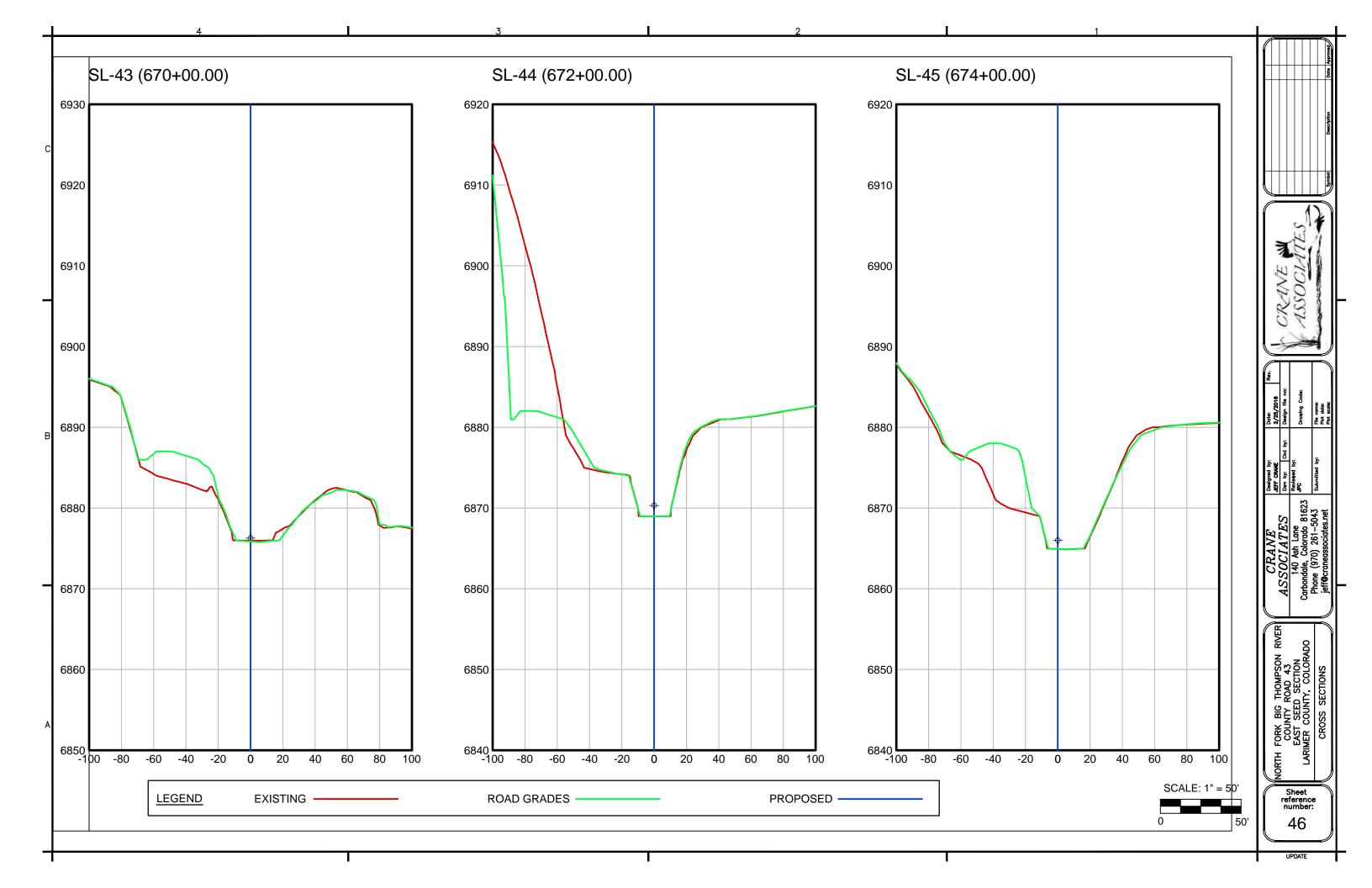


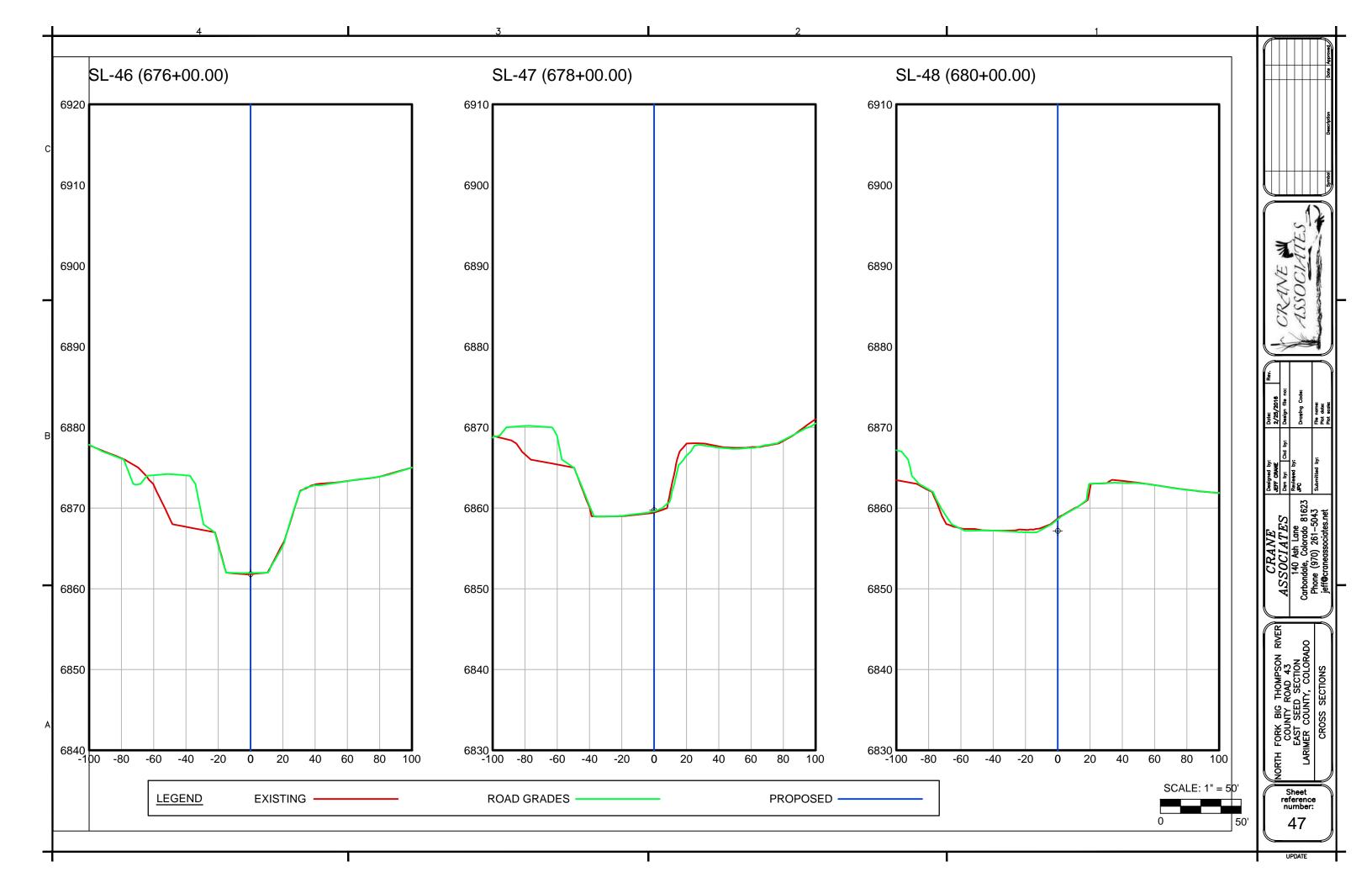


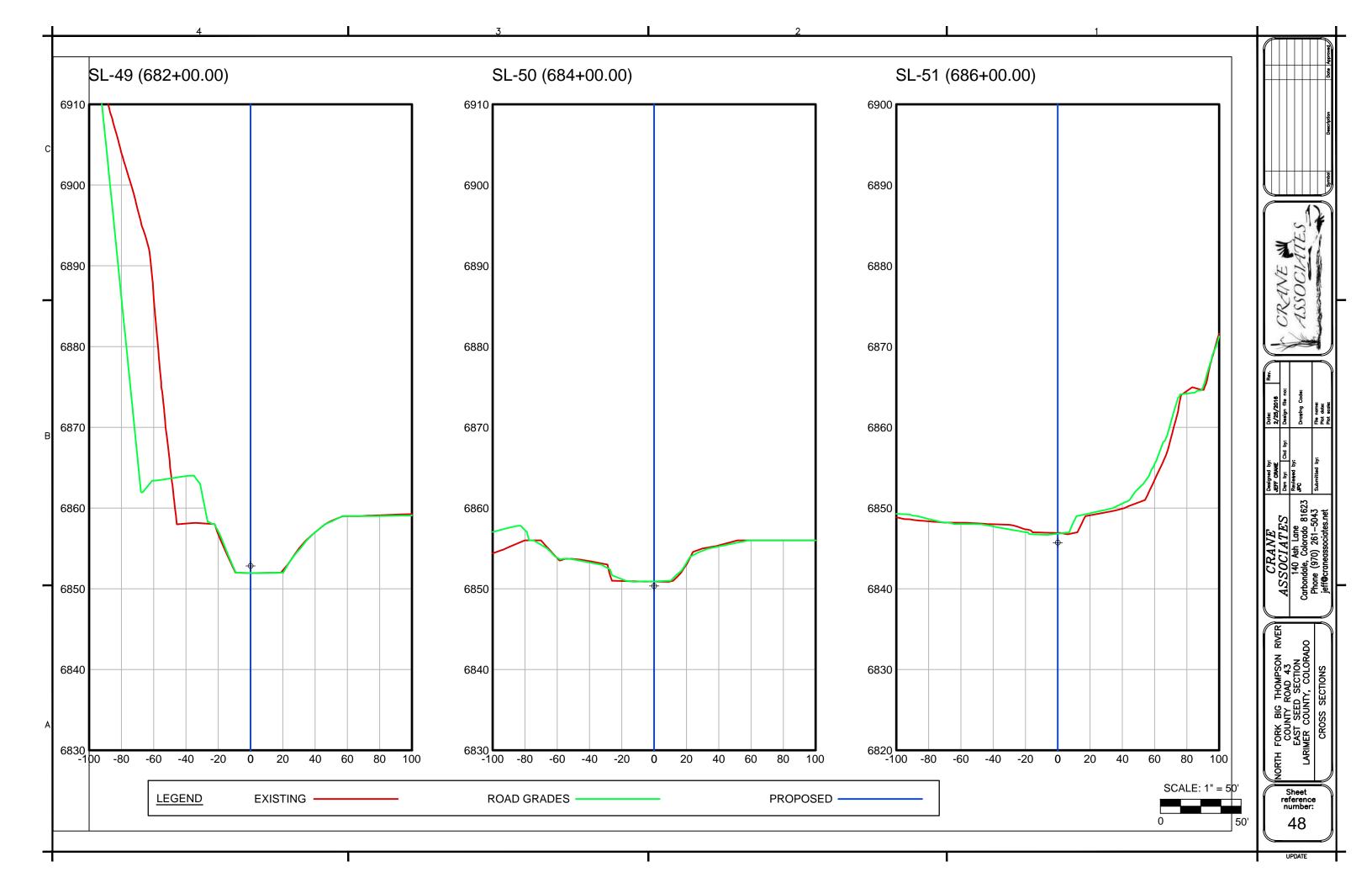


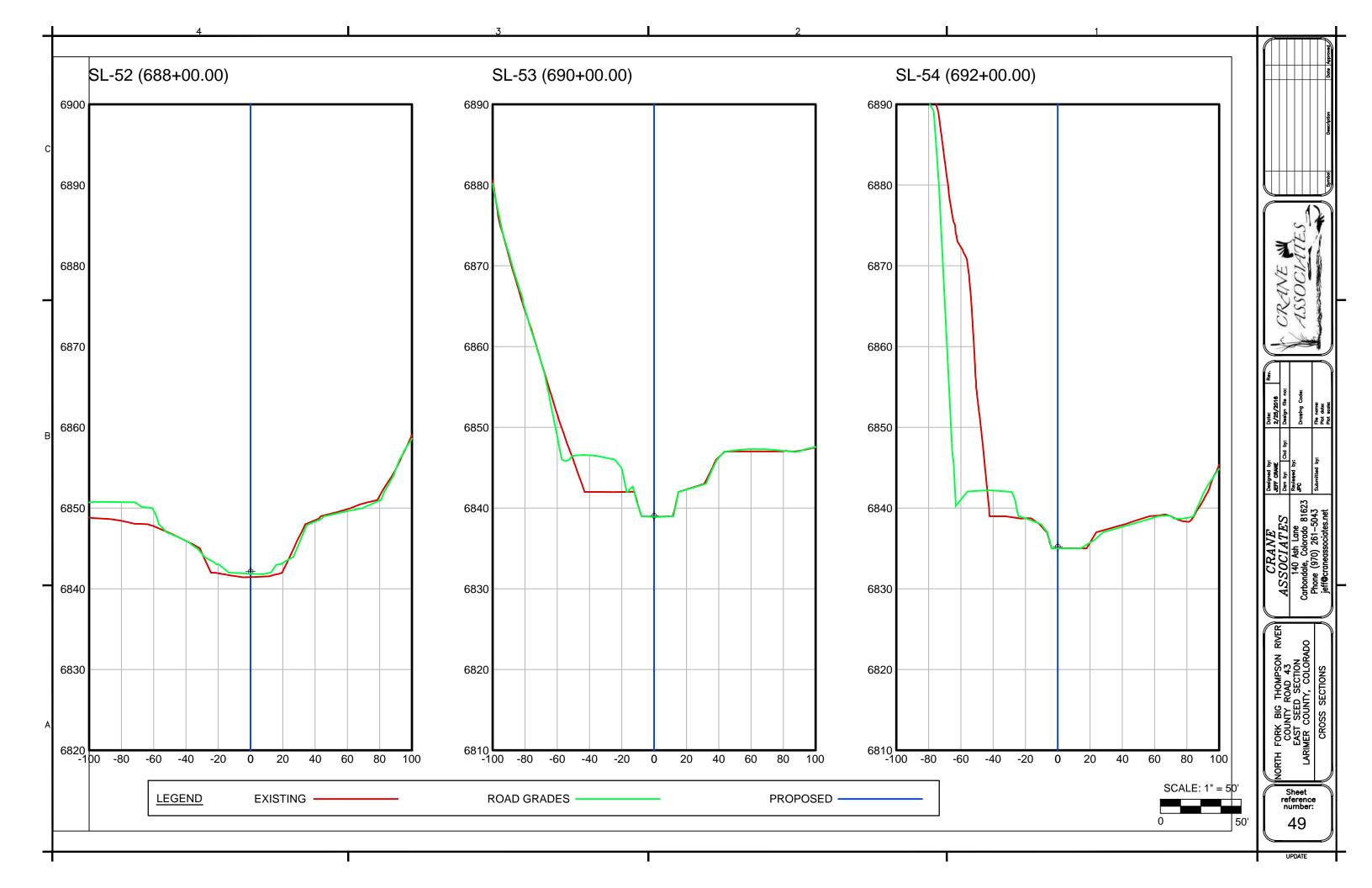


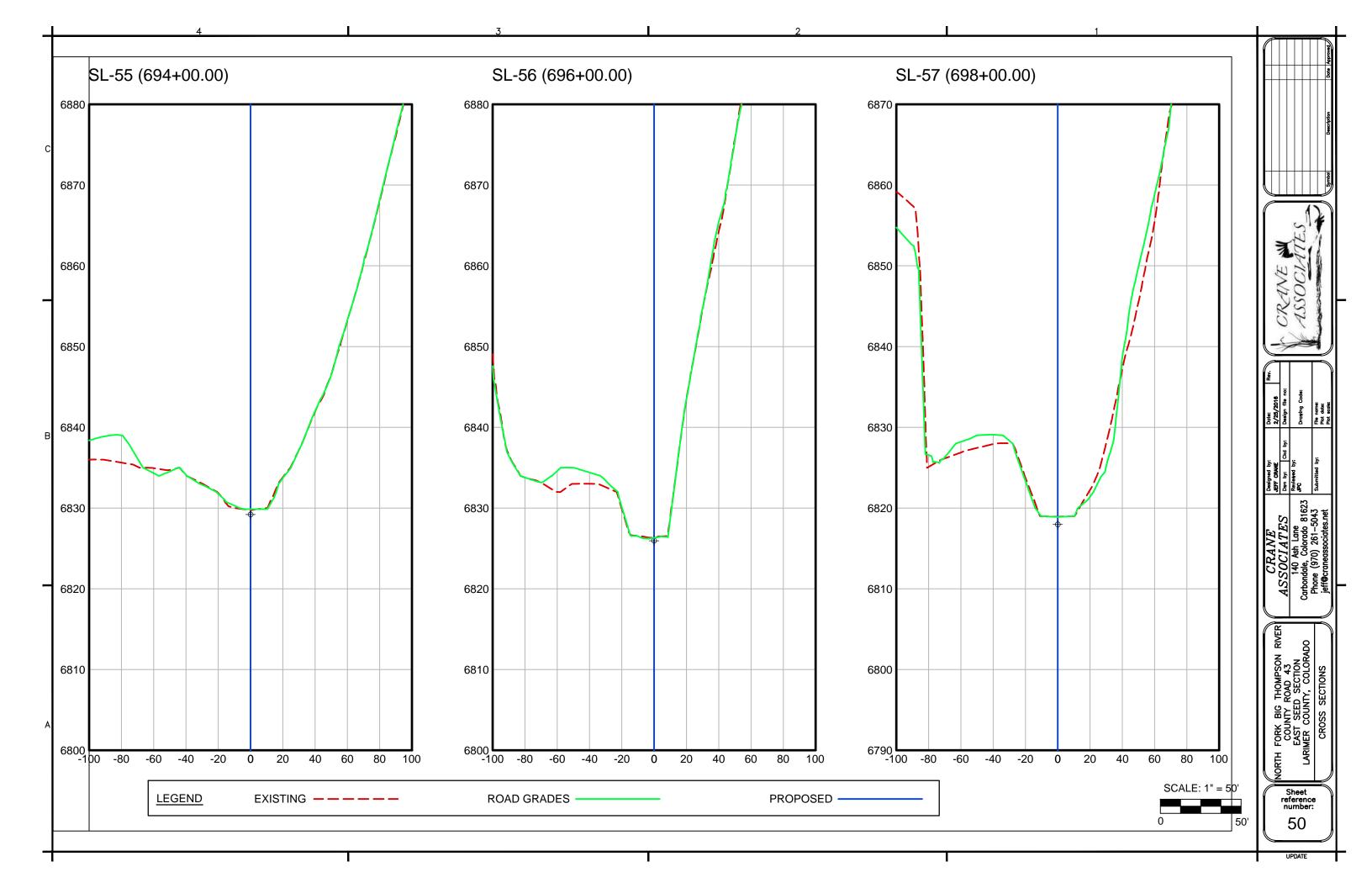


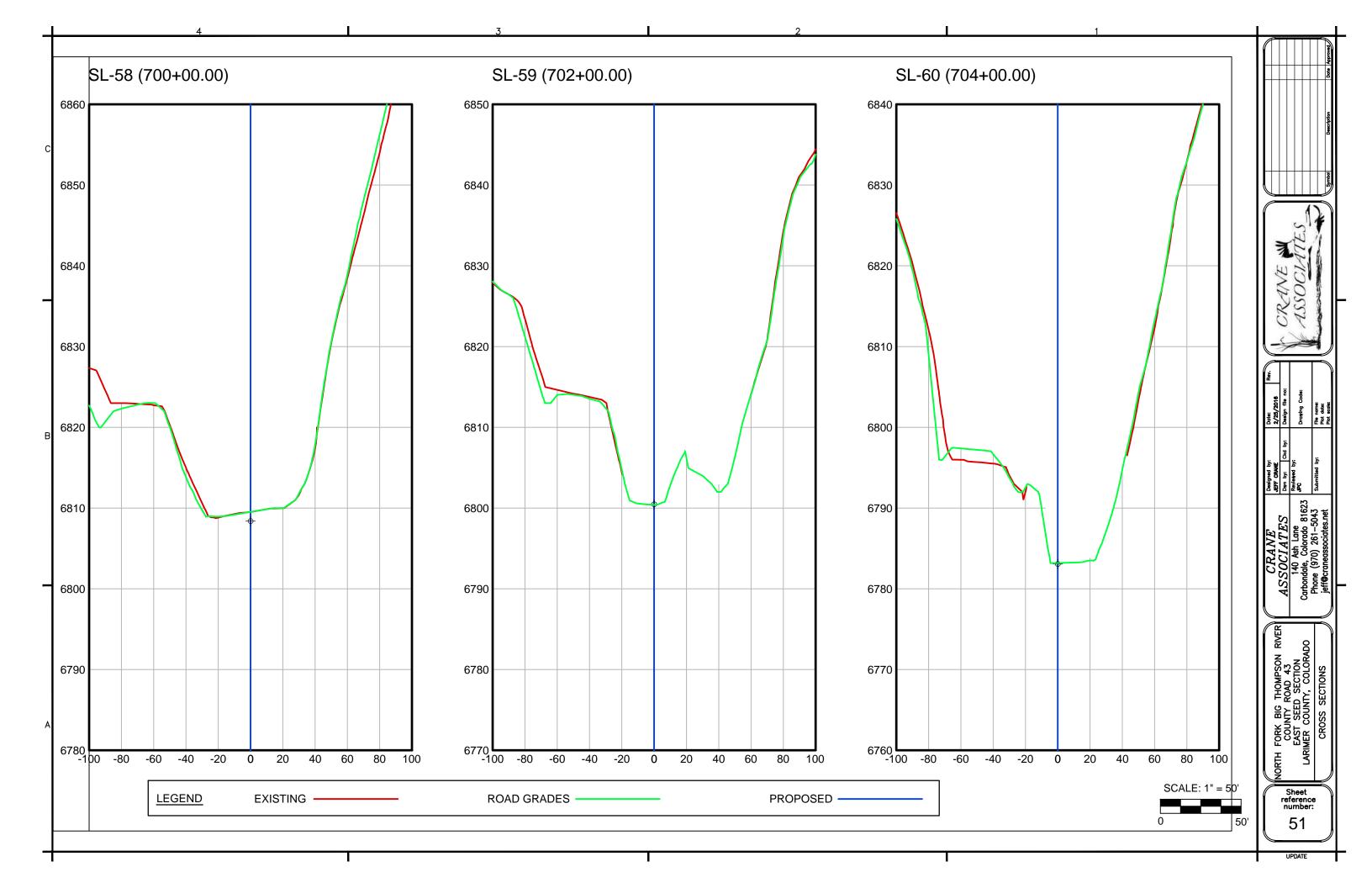


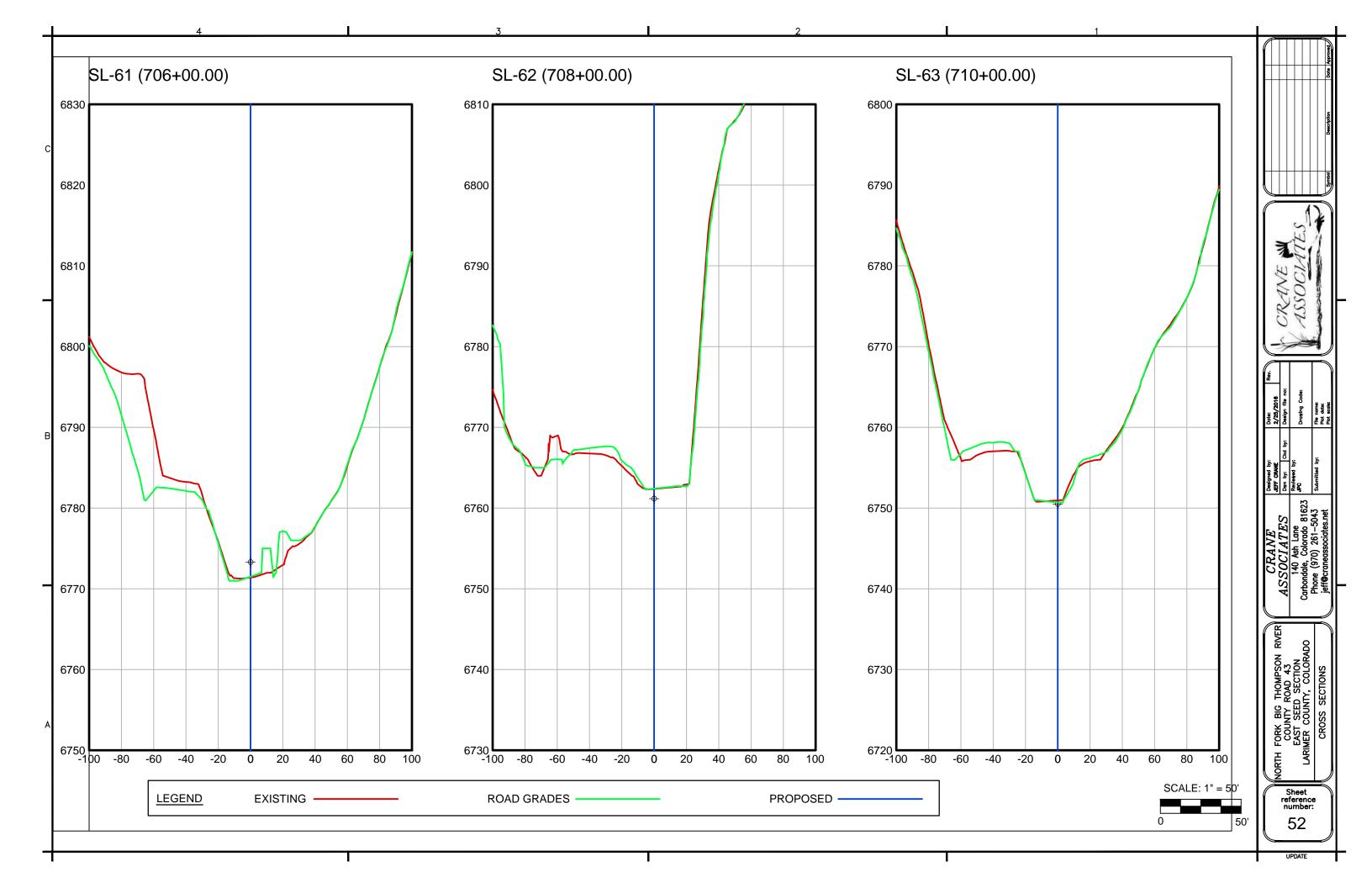


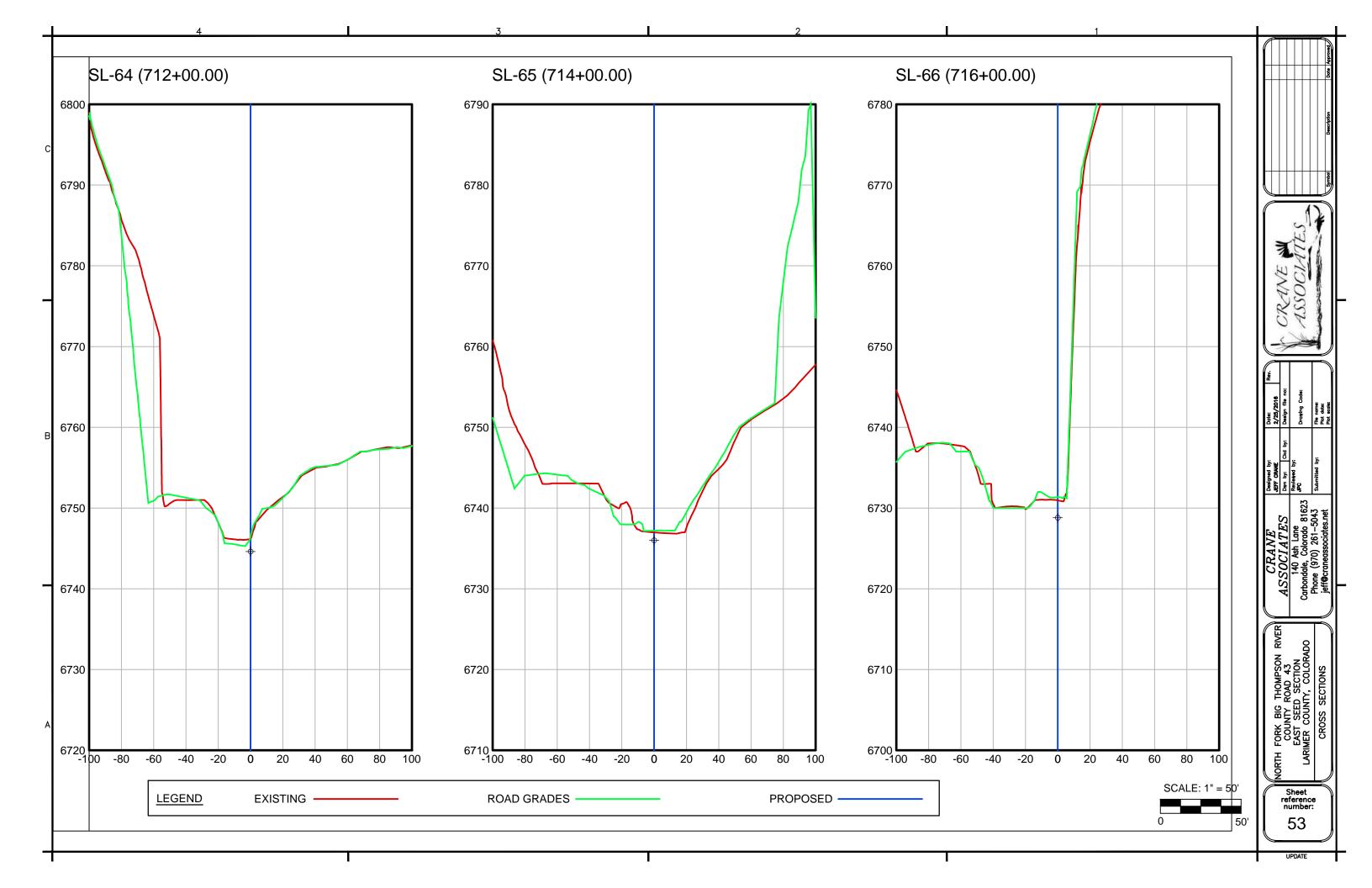


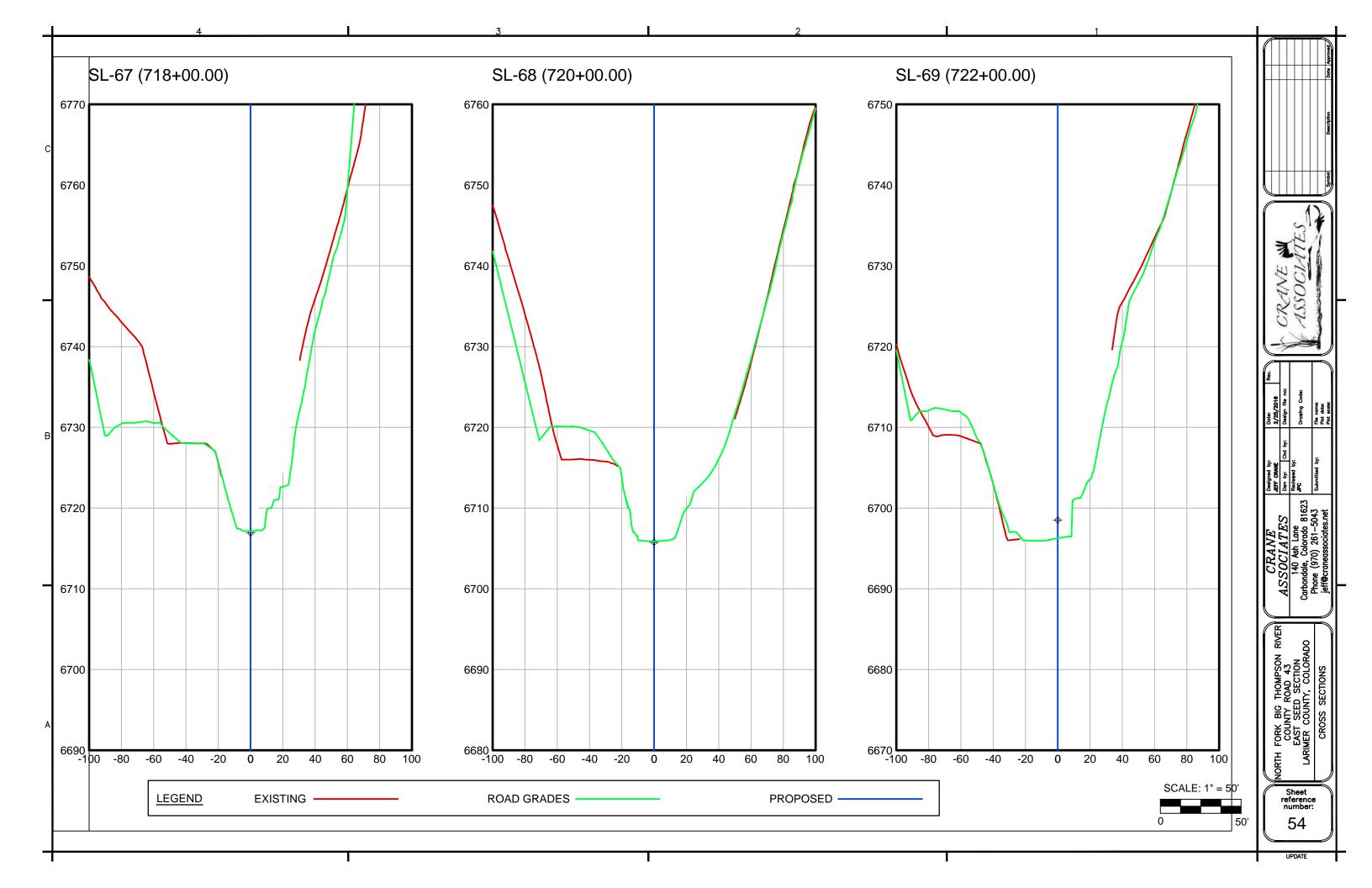


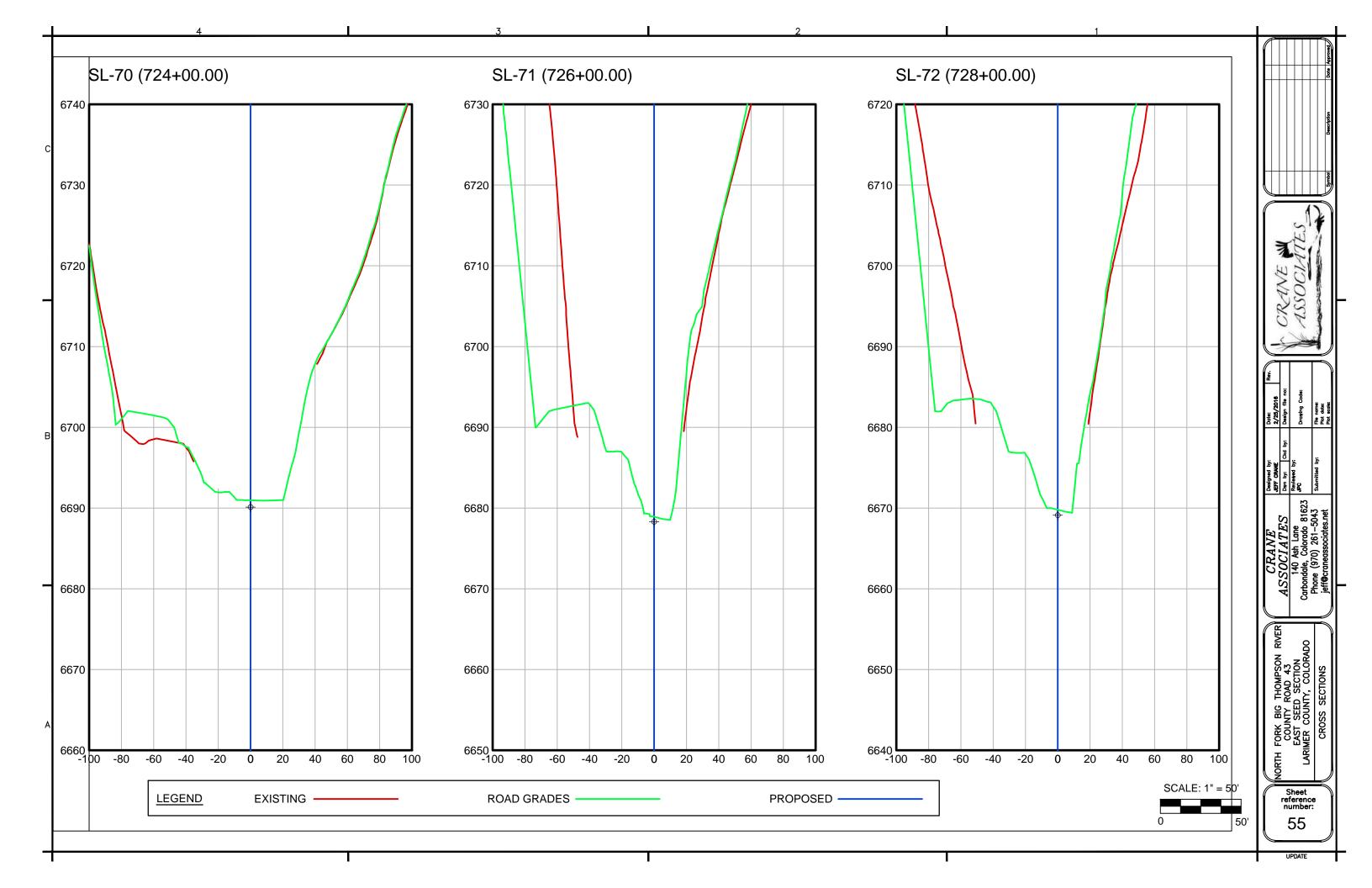


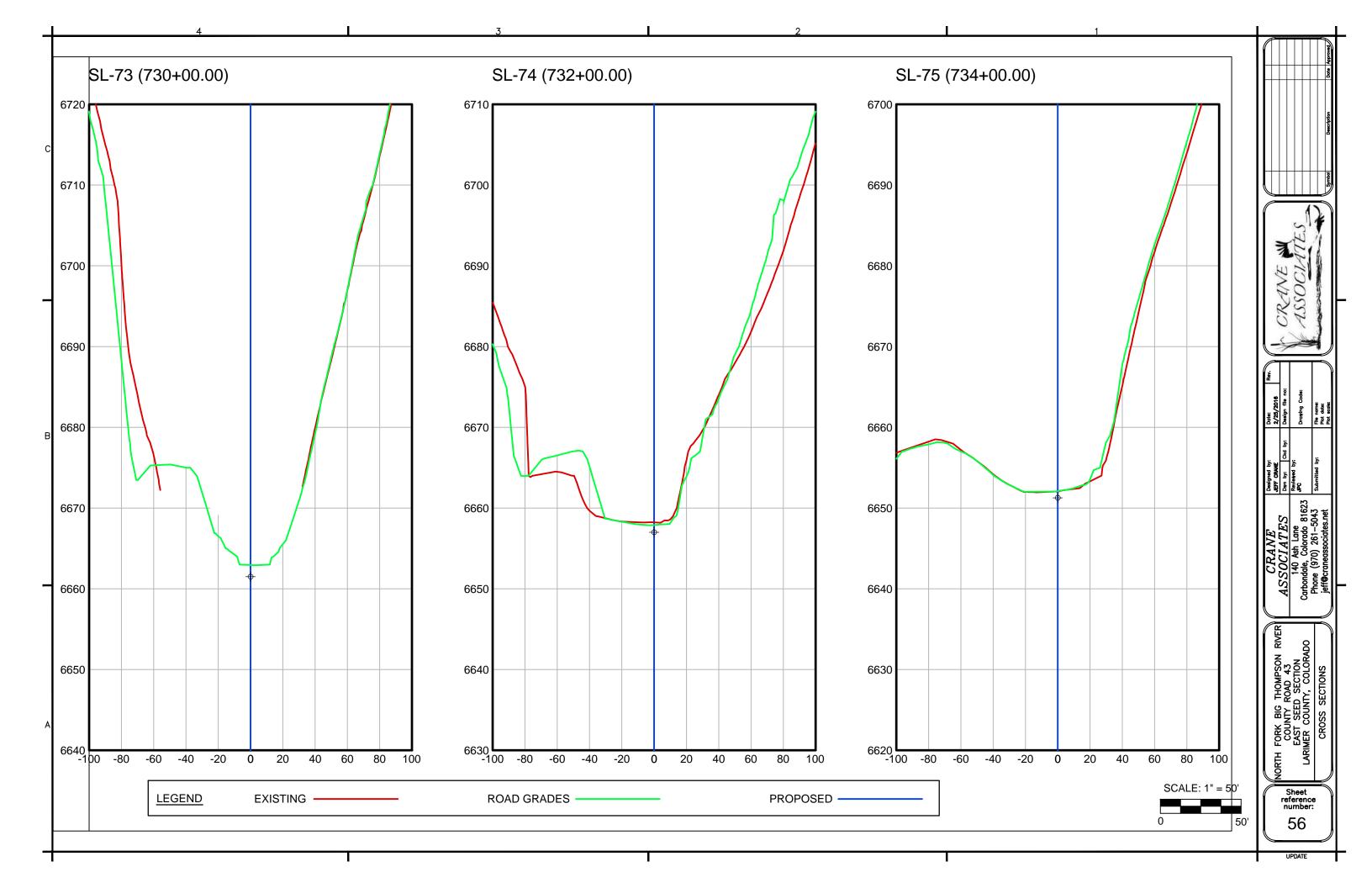


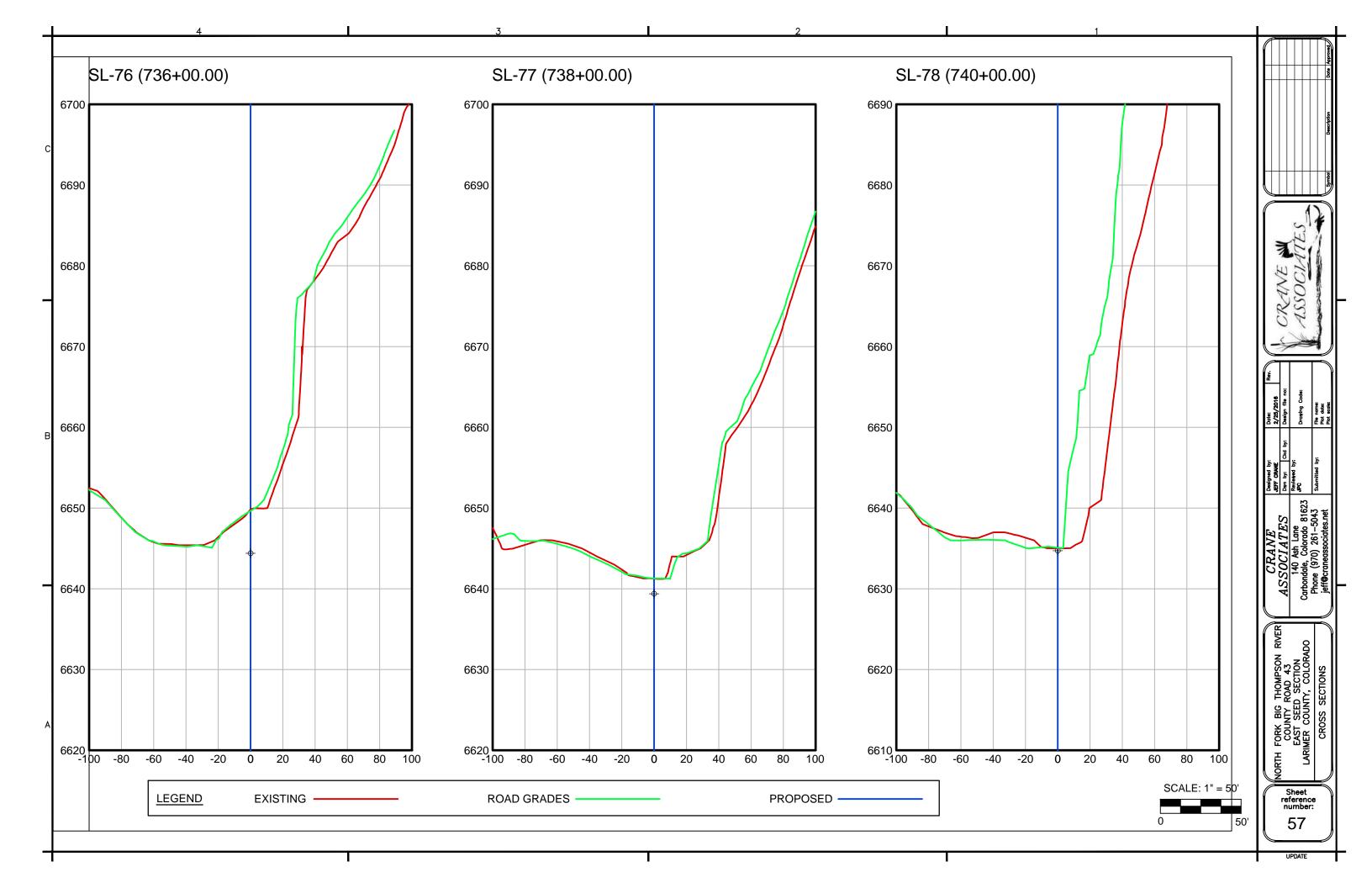


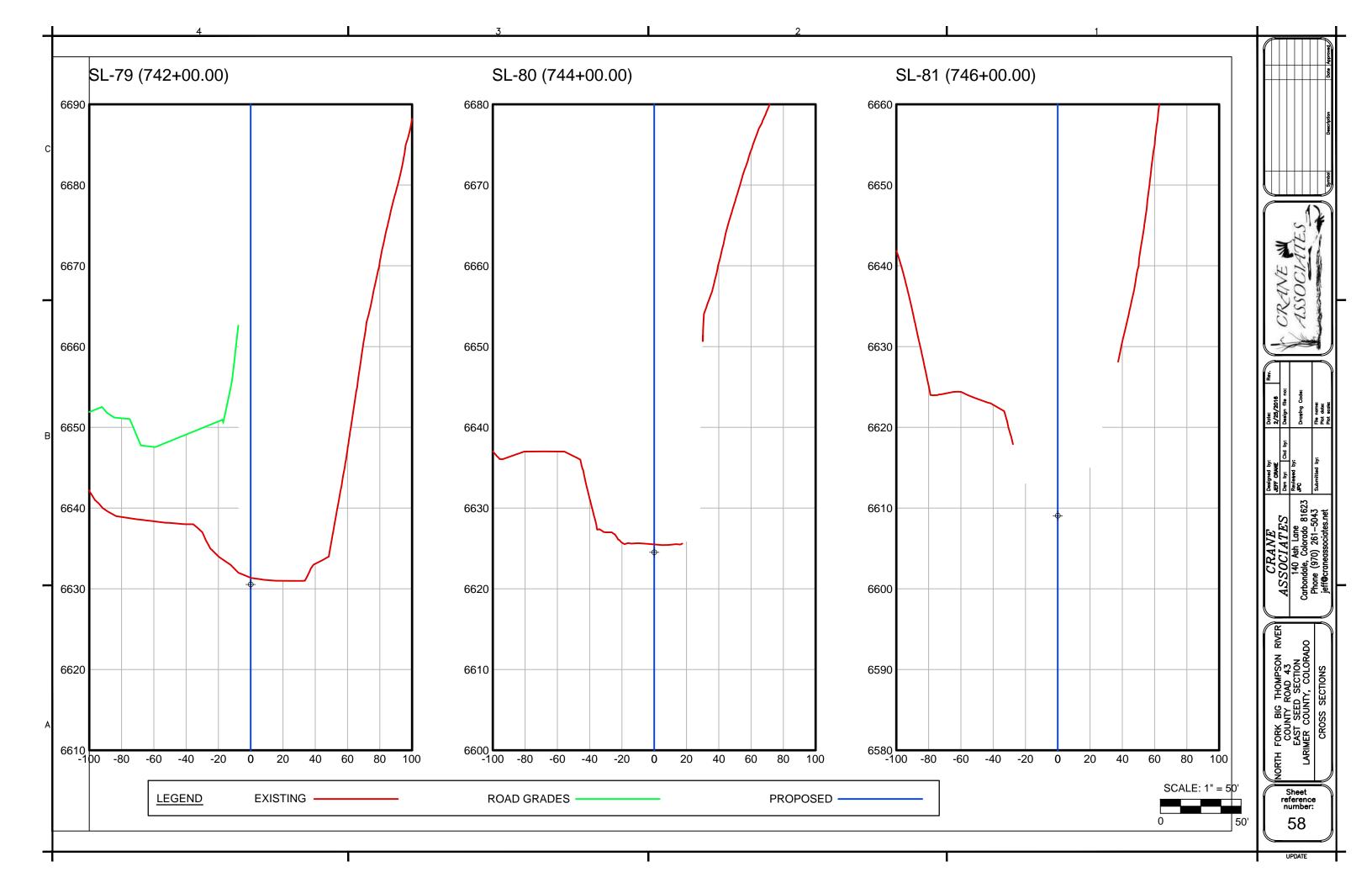


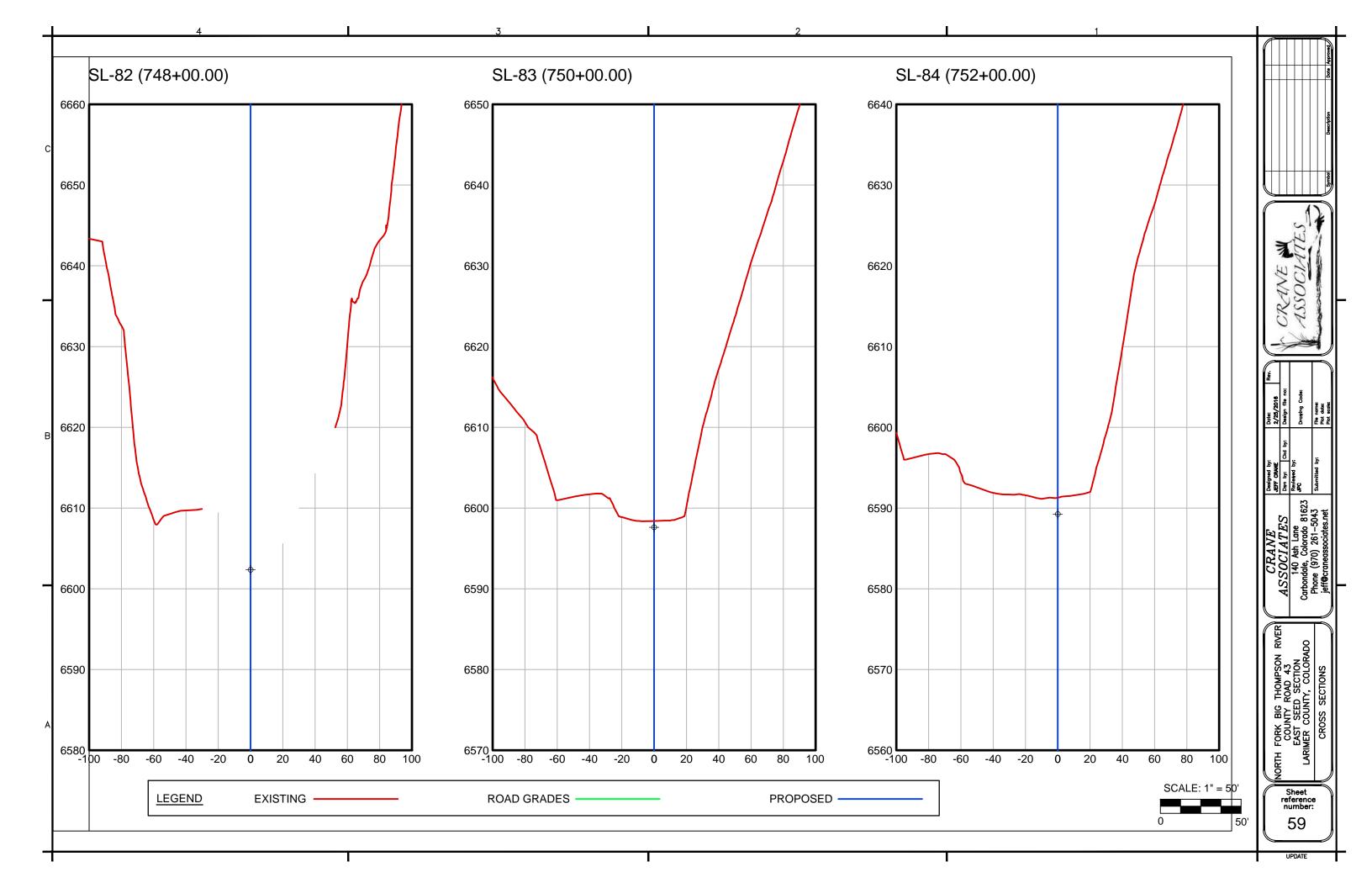


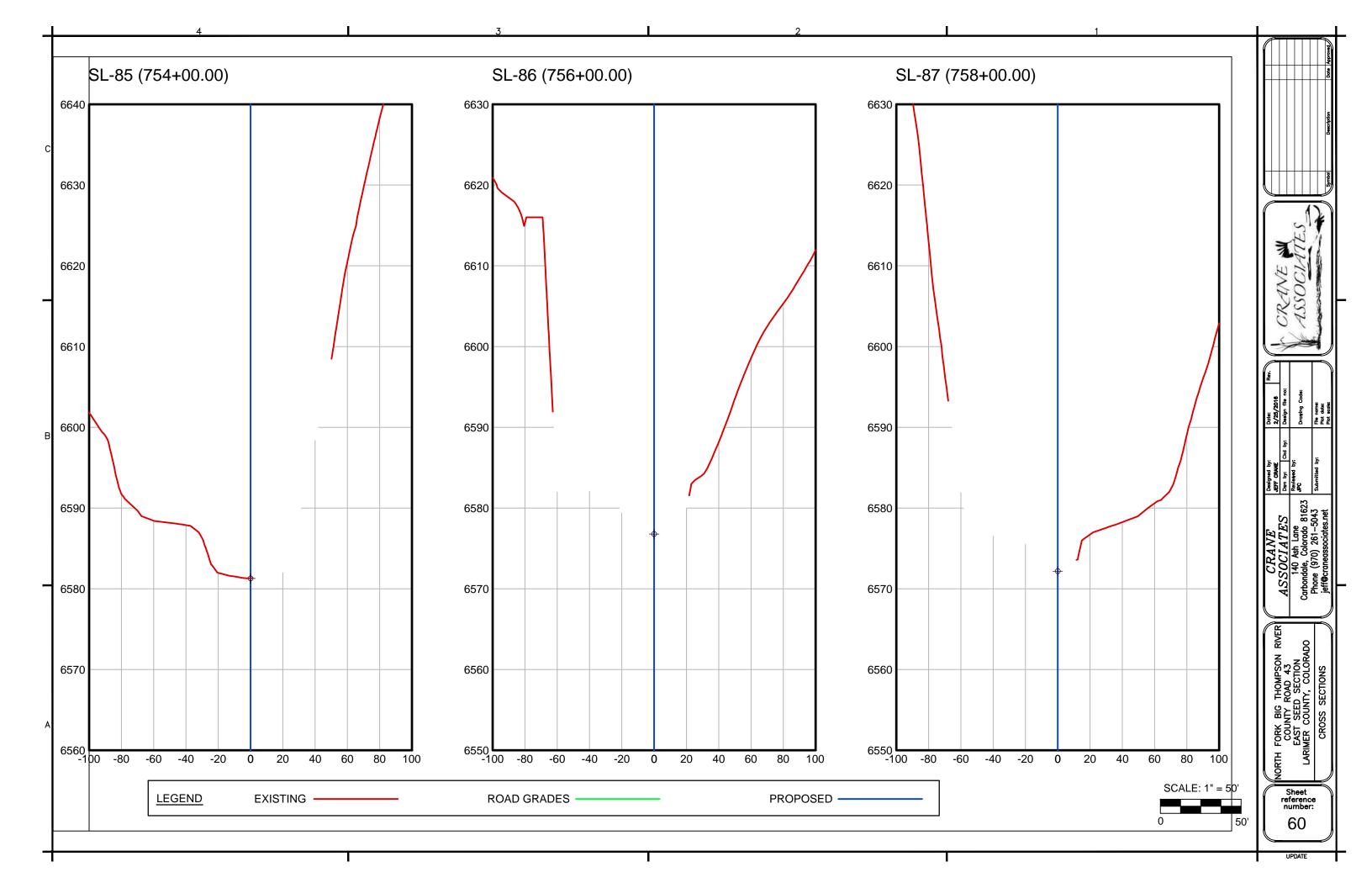


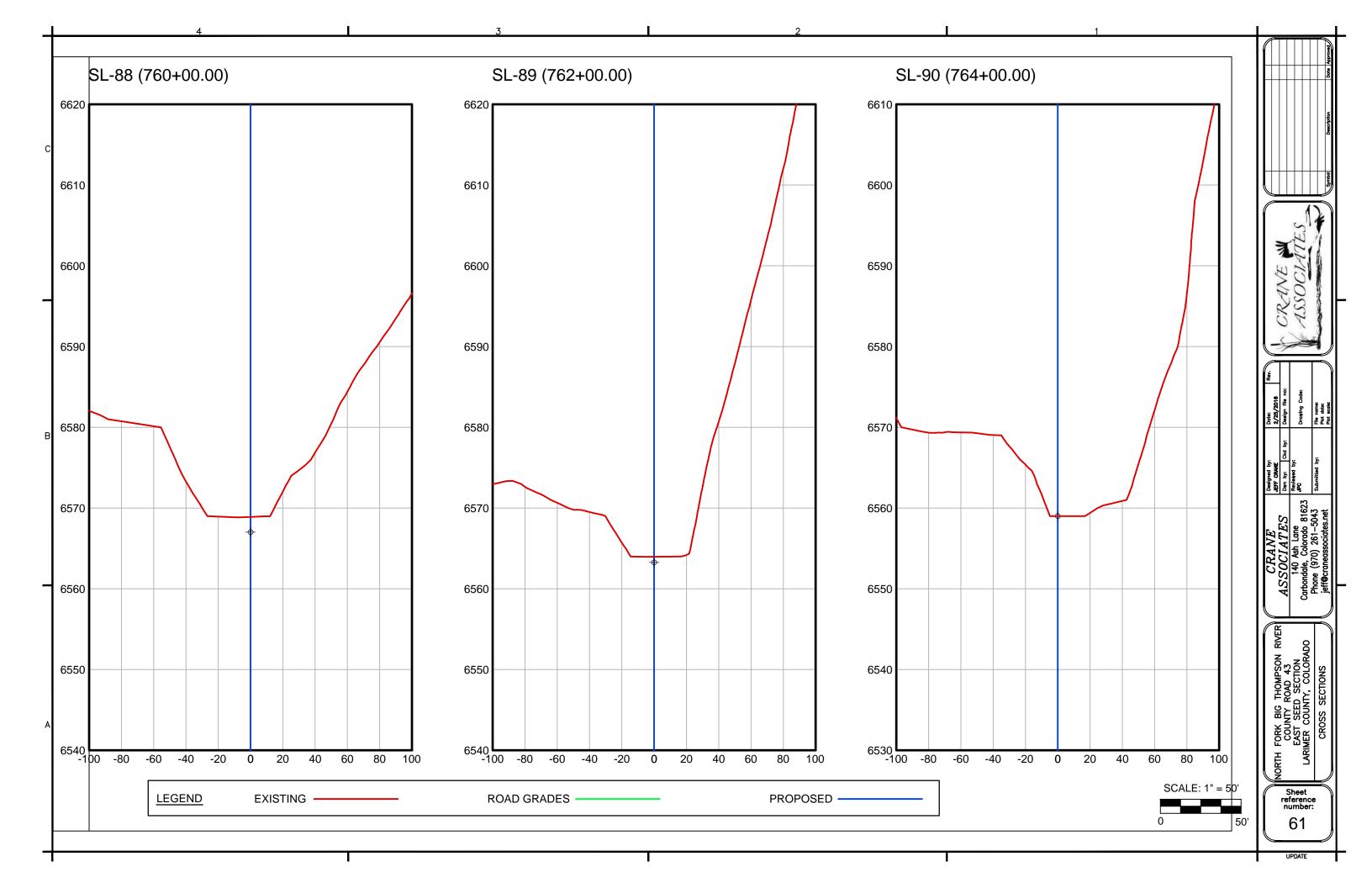


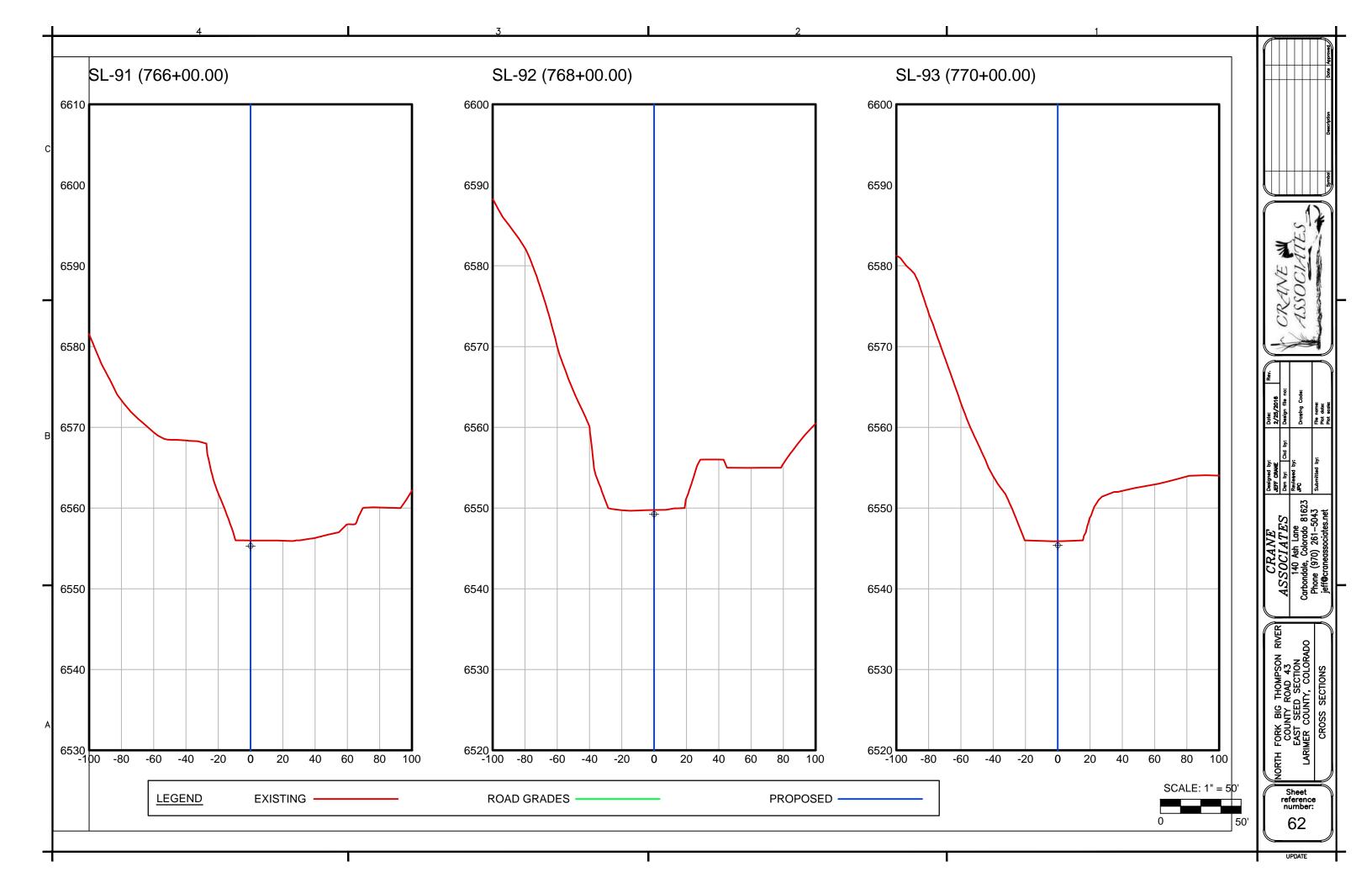


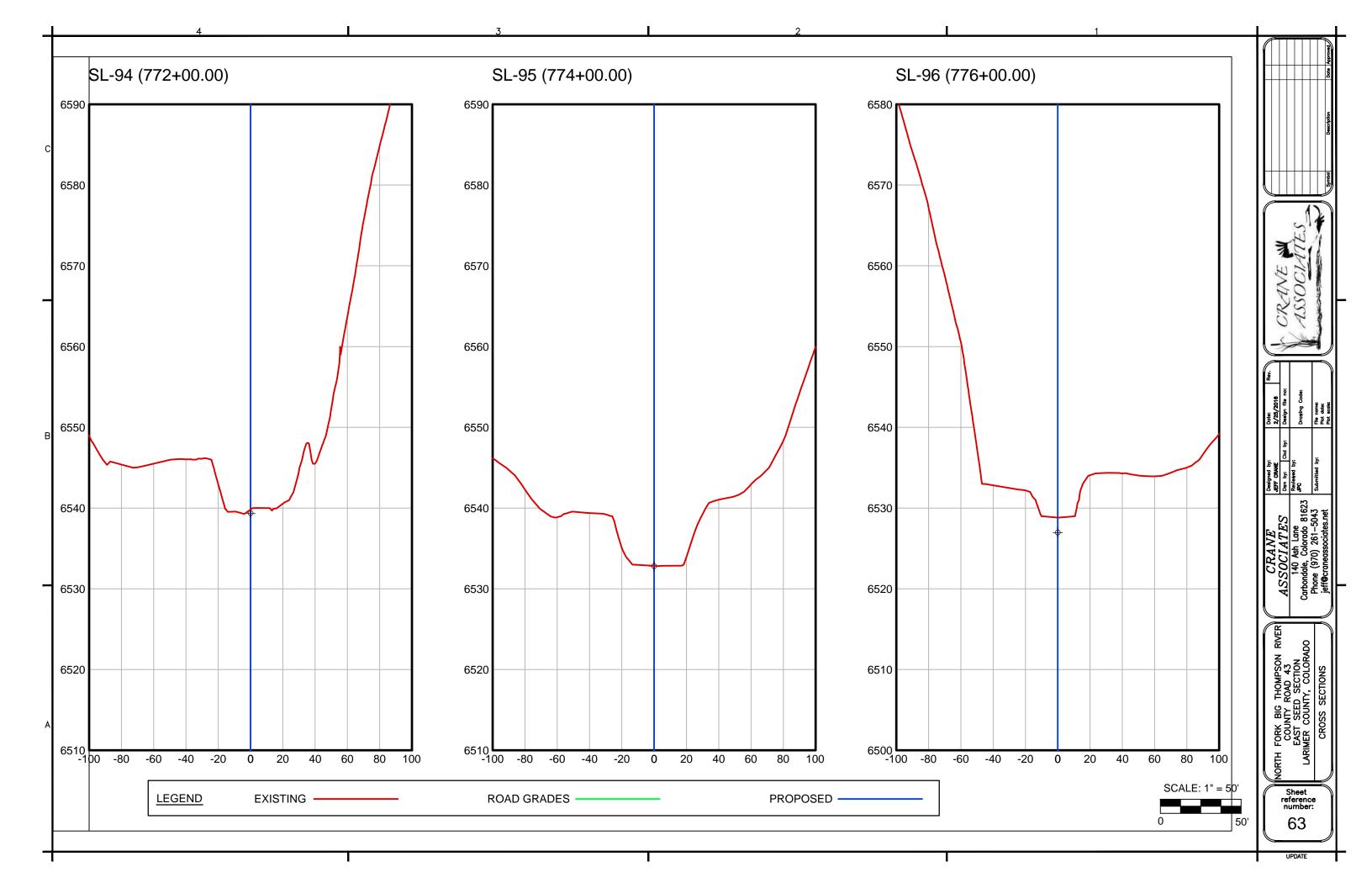


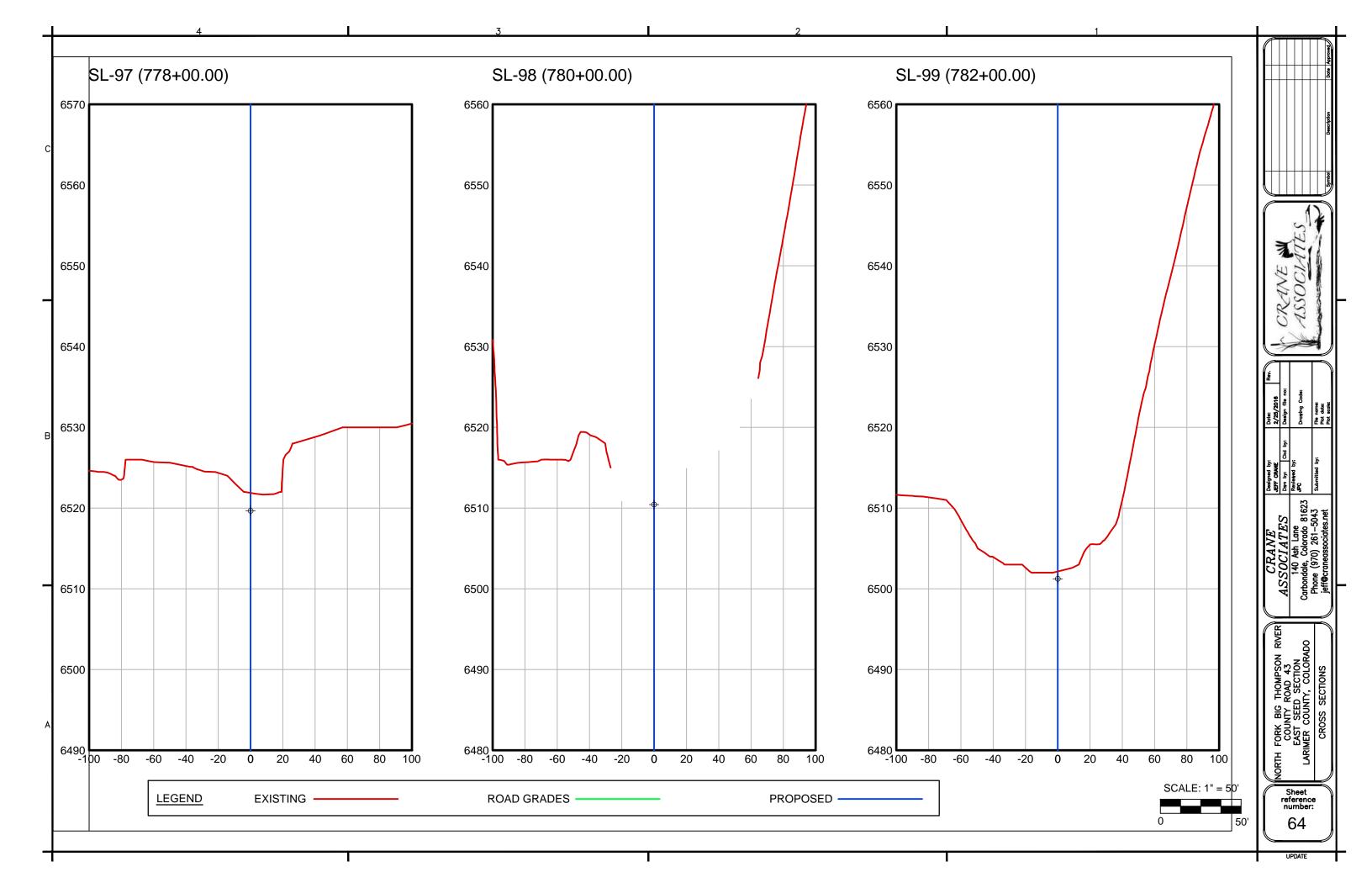


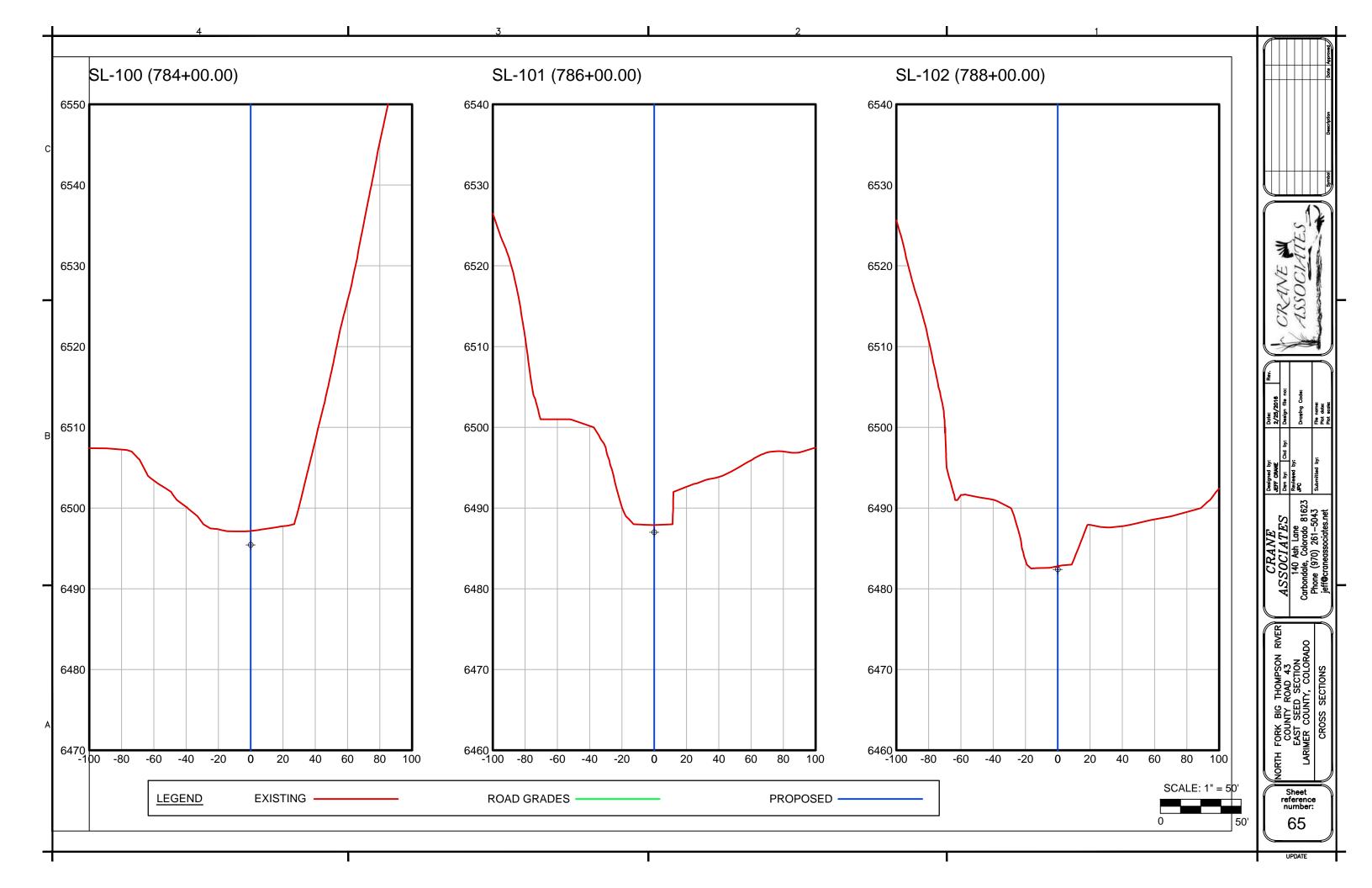


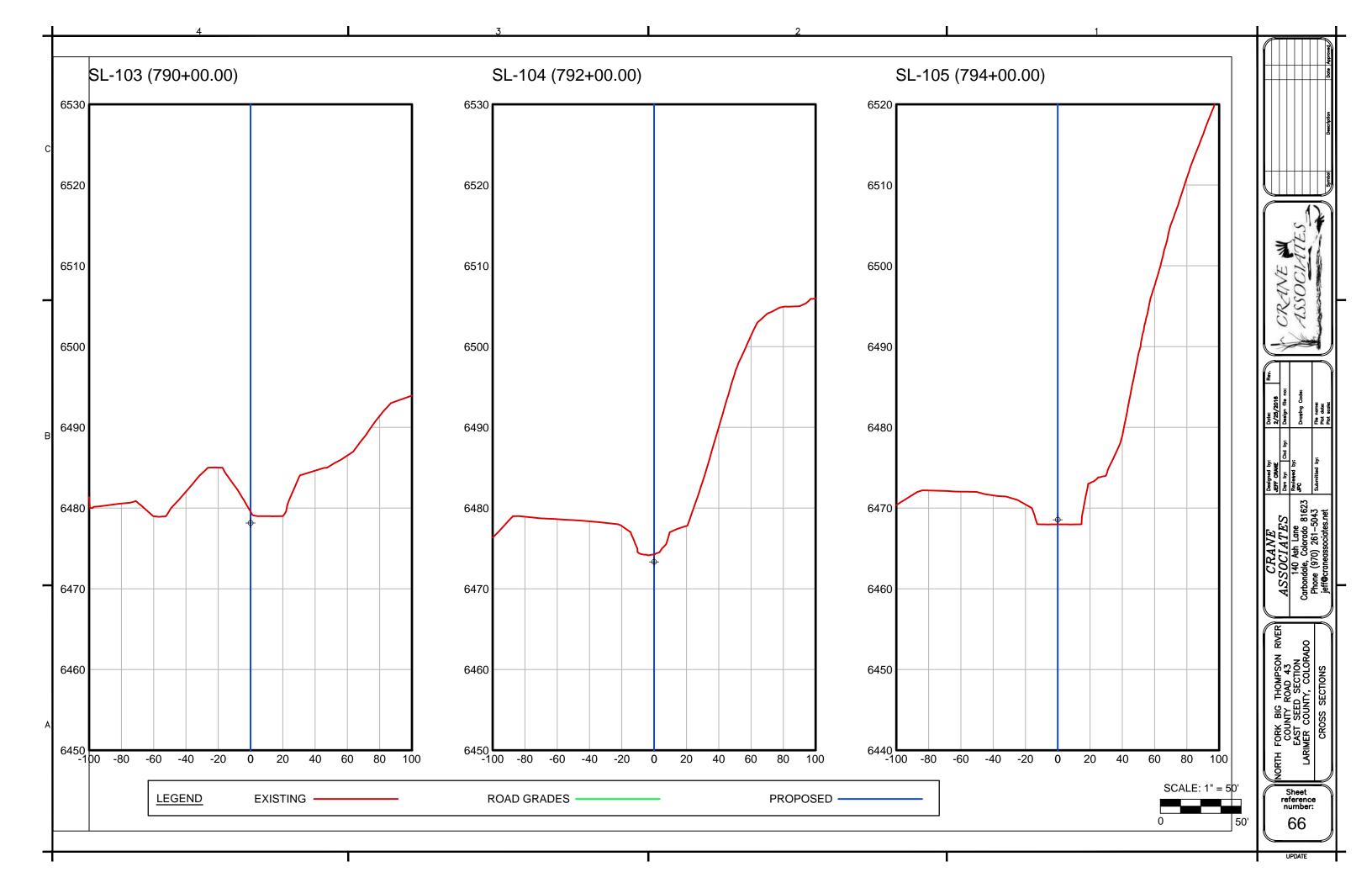


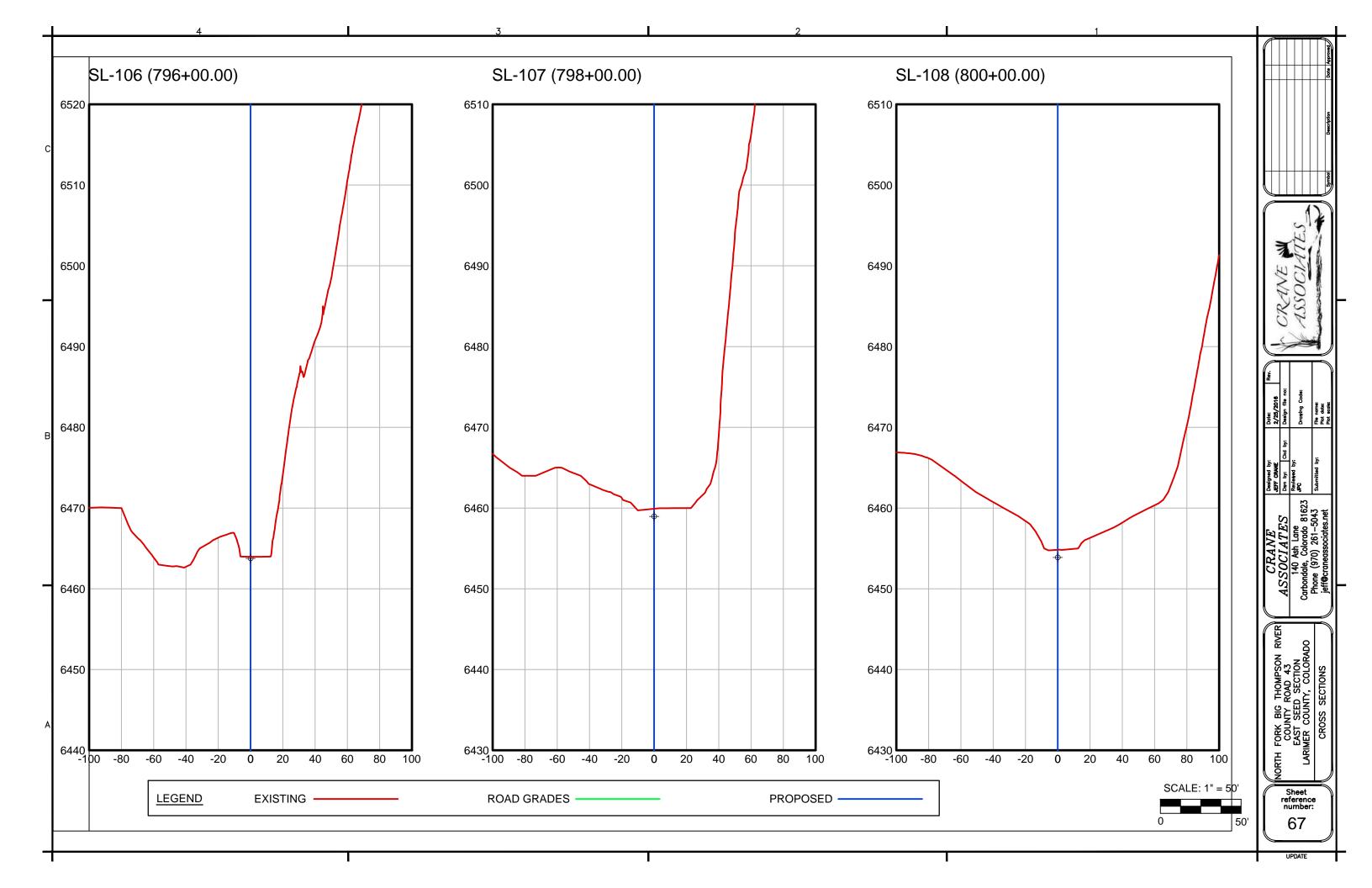


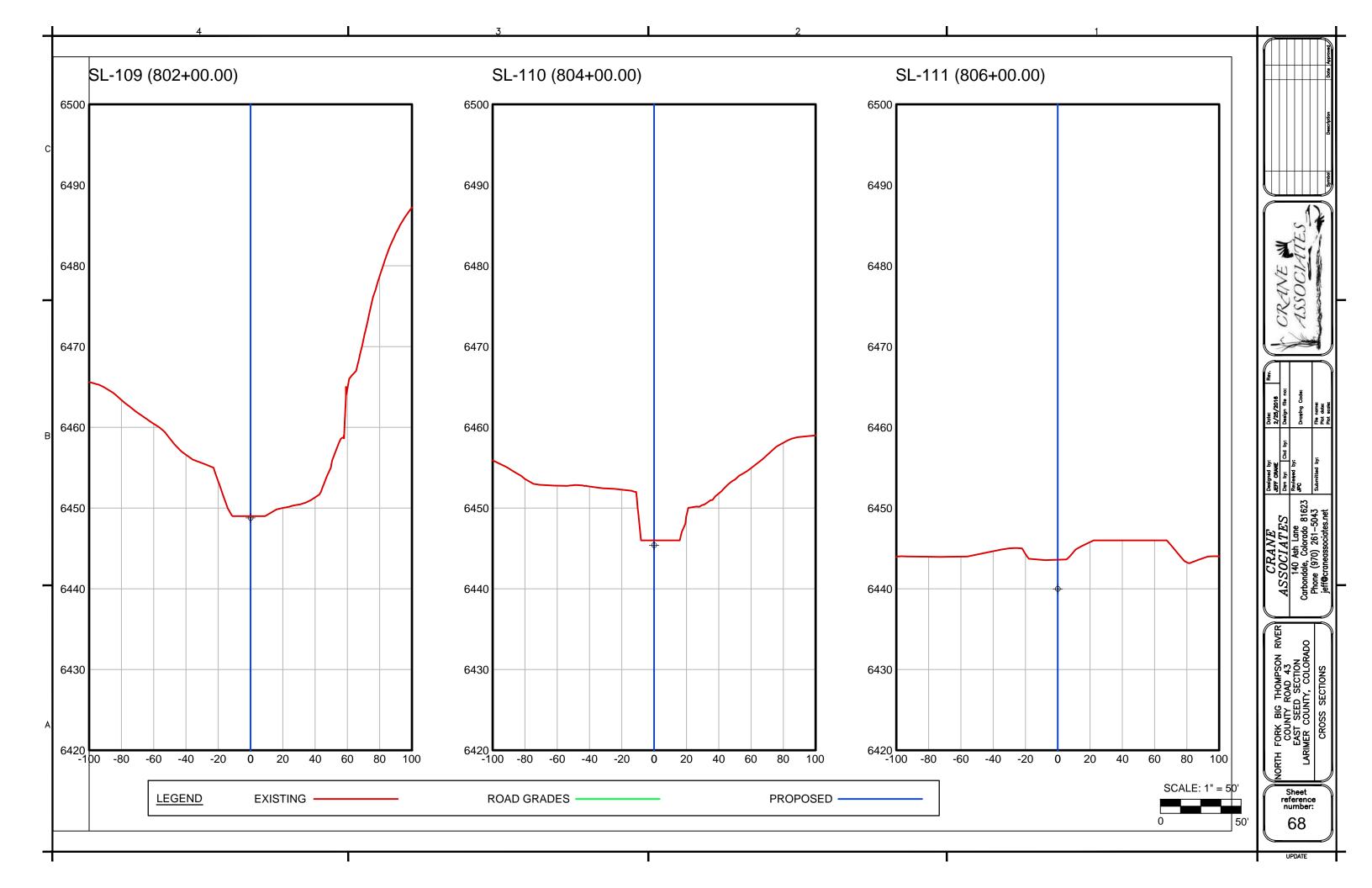


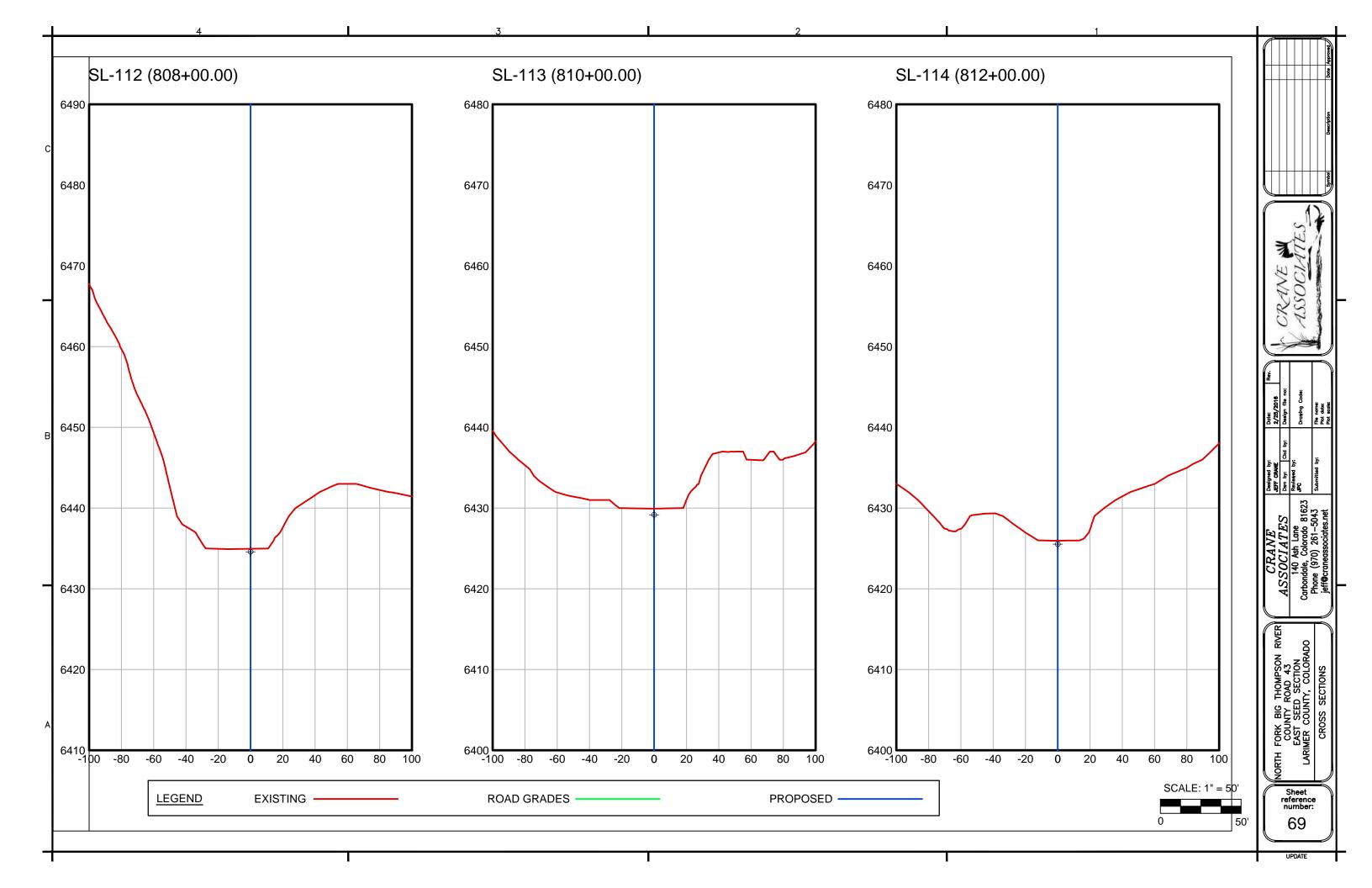


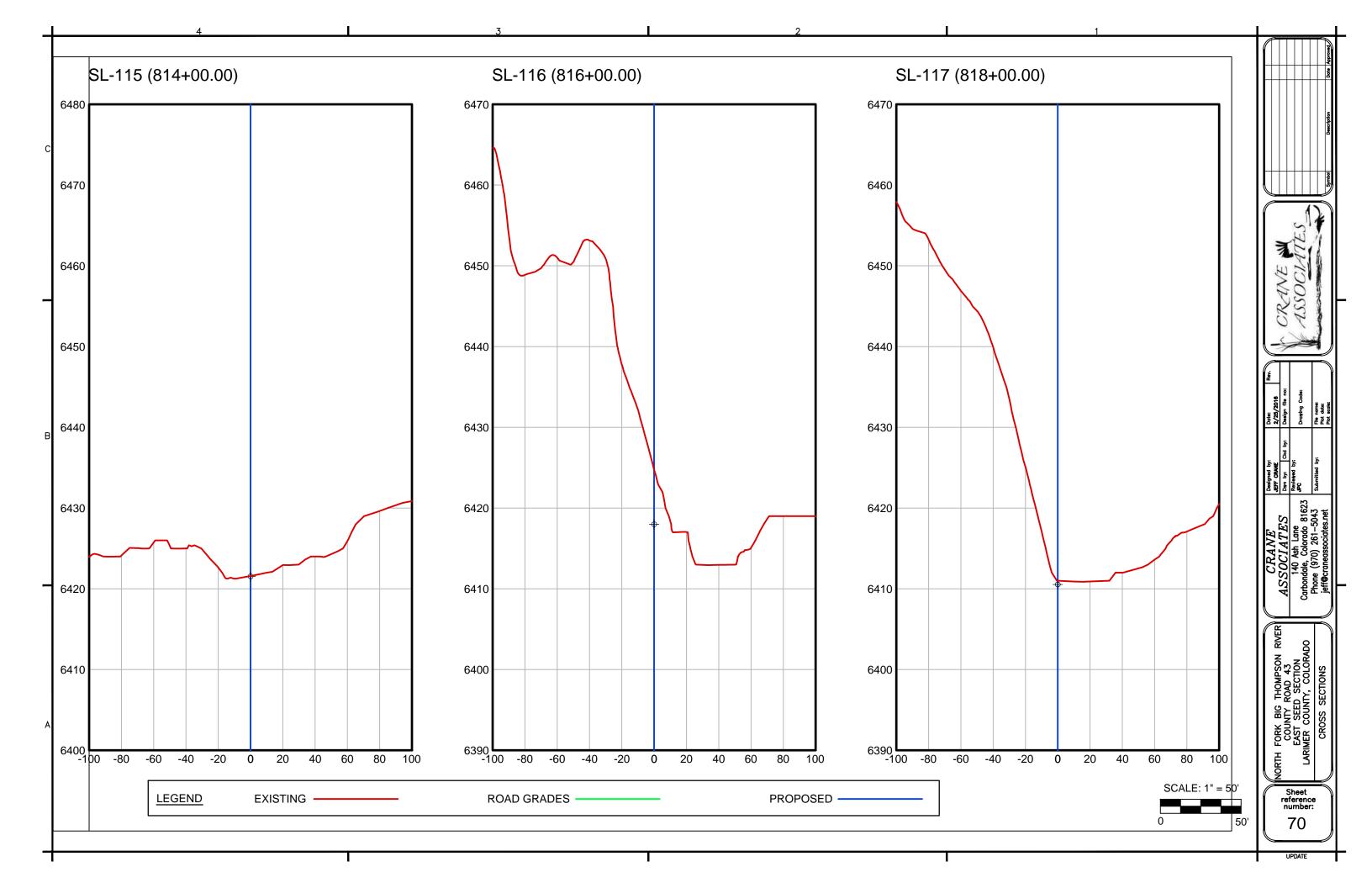


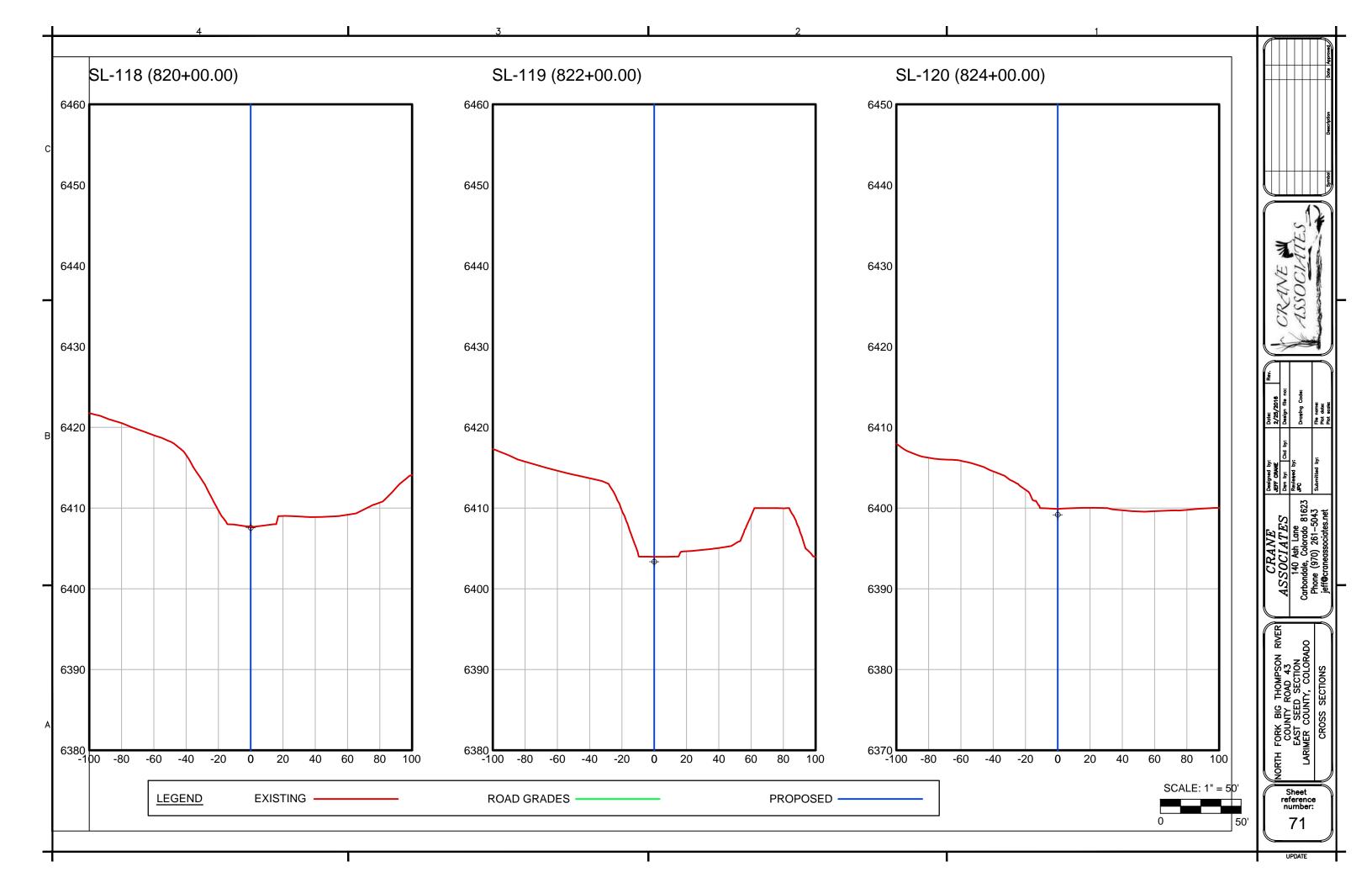


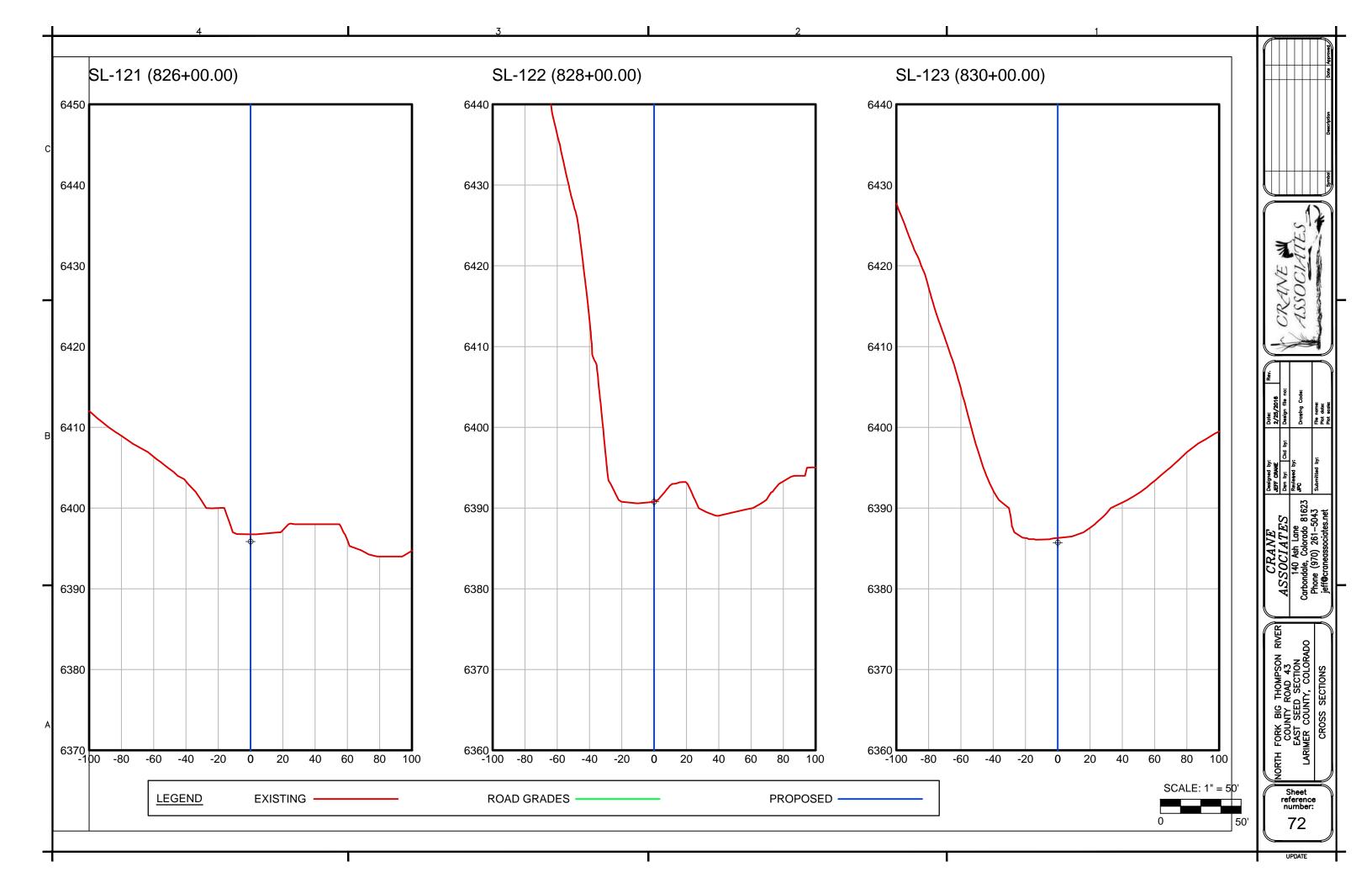


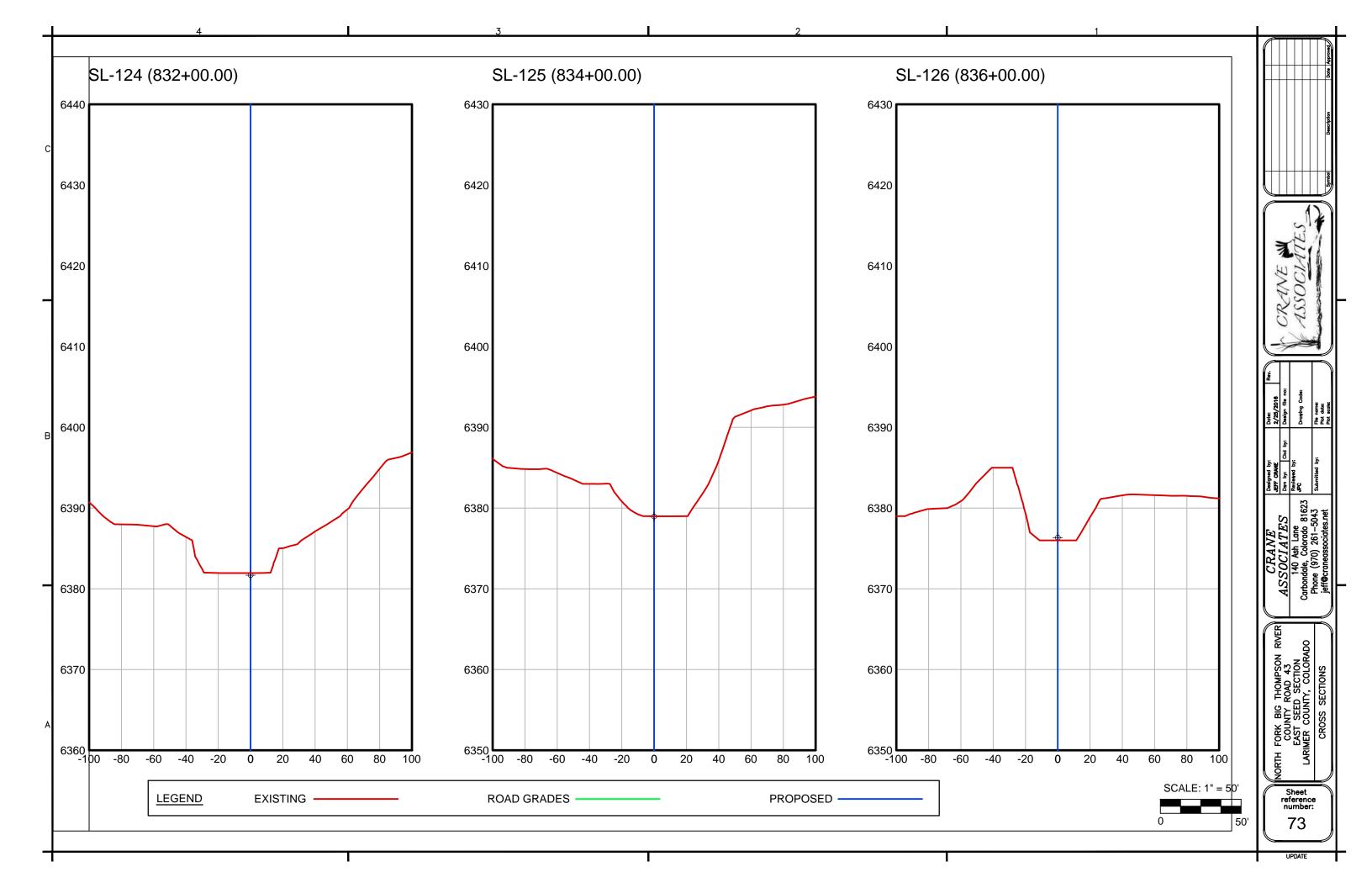


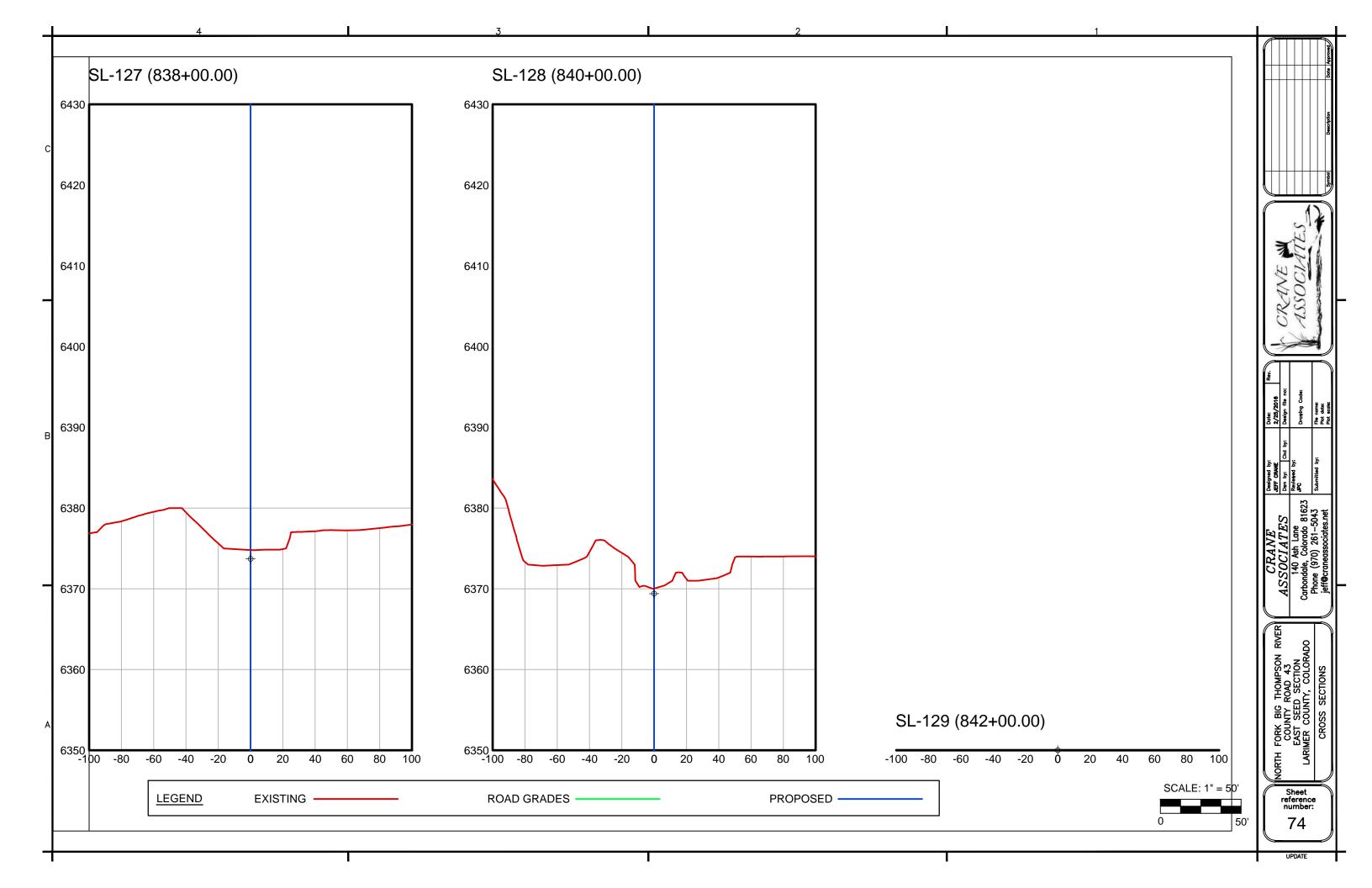




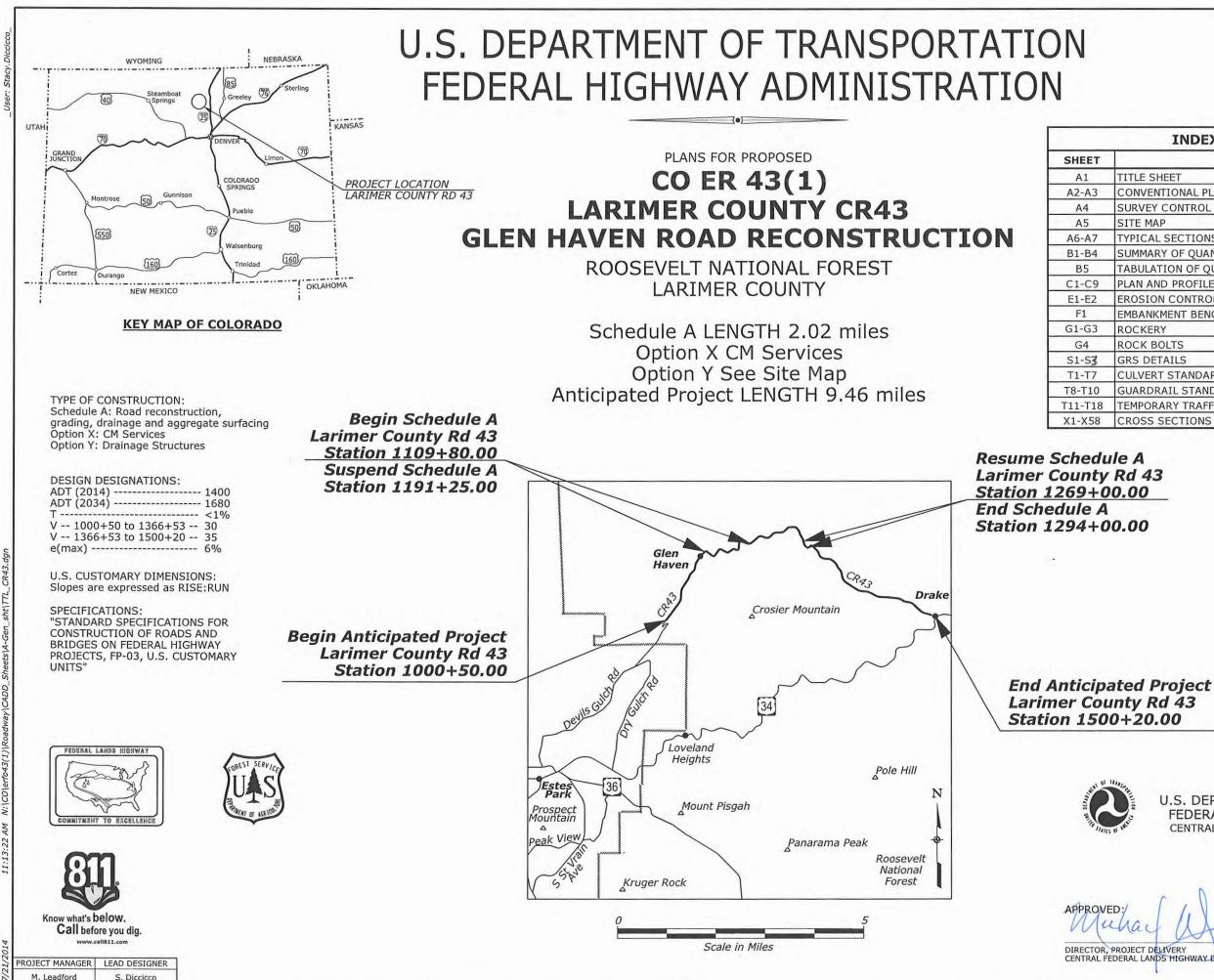








Appendix C: Central Federal Lands Road Construction Plans



REG	STATE	PROJECT	SHEET NO.	TOTAL SHEETS
IMR	со	ER 43(1) Larimer County CR43 Glen Haven Rd Reconstruction	A1	A7

	INDEX TO SHEETS
ET	DESCRIPTION
	TITLE SHEET
13	CONVENTIONAL PLAN SYMBOLS & ABBREVIATIONS
	SURVEY CONTROL
e	SITE MAP
17	TYPICAL SECTIONS - MAINLINE
34	SUMMARY OF QUANTITIES
	TABULATION OF QUANTITIES
29	PLAN AND PROFILE - MAINLINE
2	EROSION CONTROL DETAILS
	EMBANKMENT BENCHING
53	ROCKERY
	ROCK BOLTS
53	GRS DETAILS
7	CULVERT STANDARDS
10	GUARDRAIL STANDARDS
18	TEMPORARY TRAFFIC CONTROL STANDARDS
58	CROSS SECTIONS - MAINLINE

PLANS PREPARED BY

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION DENVER, COLORADO

DIRECTOR, PROJECT DELIVERY CENTRAL FEDERAL LANDS HIGHWAY DIVISION

r	4000	ATIONS	1			
	ABBREVIA	ATIONS			DRAINAGE SYMBOLS	
cciccc	$\mathcal{Q} \\ \Delta$	centerline curve delta	L L	length of curve lamination	Ditch (Existing, Proposed)	
Stacy.Diccicco	A abut. ADT	abutment average daily traffic	lat. long.	latitude longitudinal	Flow Arrow	\sim
ser: St	aggr. AH	aggregate ahead	LPŠM Lt. or LT	lump sum left	Drainage or Small Creek	
<i>ŋ</i> _	alt. appr. asph.	alternate approach asphalt	LW M mag.	low water magnetic maintenance	Lake, Pond or Reservoir	
	B b.f. beg.	both faces beginning,begin	<i>maint. matl. max.</i>	material maximum	Large Creek	000
	BK BM	back bench mark	min. mon.	minimum monument	Wetland	
	BP br.	balance point bridge	mtn(s). N N	mountain(s) north	River	
	brg. C CBC c-c	<i>bearing concrete box culvert center to center</i>	NC neg. no. or #	normal crown negative number	Spring	SPRING
	clr. CMP	clear corrugated metal pipe	0 0.C. 0.f.	on centers other face		■ \
	Co. col.	county column	OD P PC	outside diameter point of curve	Bridge (Existing, Proposed)	
	conc. constr. constr. jt.	concrete construction construction joint	PCC perf.	point of compound curve perforate point of intersection	Box Culvert (Existing, Proposed)	
	cont. corr.	continuous corrugated	PI pl. POC	point of intersection plate point on curve	Pipe Culvert (Existing, Proposed)	
	cr. CS	creek point of curve to spiral	POS POT	point on spiral point on tangent	With End Sections (Existing, Proposed)	⊳∢
	ctrs. CTSM culv.	centers contingent sum culvert	proj. psi	project pounds per square inch point of tangent	With Headwalls (Existing, Proposed)	├
	D decr. DHV	decrement design hour volume	PT pvmt. Q quant., Qty	pavement	With Drop Inlet (Existing, Proposed)	
	DI dia. or D	drop inlet diameter	R R R.	radius range	Underdrain (Existing, Proposed)	
	diag. diaph.	diagonal diaphragm	R/W rd.	right-of-way road	Riprap Apron (Proposed)	F.IS
	dist. Dist.	distance district	rdwy. reconst.	roadway reconstruction		
	DLC	donation land claim	reinf.	reinforcement		
	dwg(s). E E	drawing(s) east	reqd. res.	required reservoir	ERACIAN & CEDIMENT CONT	
	e El. 94.066	superelevation rate elevation with number	Res. ret. wall	Reservation retaining wall	EROSION & SEDIMENT CONT	
lgn	elev.	elevation	RH	reference hub	Bonded Fiber Matrix Mulching	
243.dgn	emb. engr(s).	embankment Engineer(s)	Rt. or RT	right route	Check Dam	\boxtimes
M_CI	EOP EQ or eq.	edge of pavement equation	s s	south seasonal average daily traffic	Diversion Berm	·····
tISY	ER	edge of road	SADT SC	point of spiral to curve	Erosion Control Mat	
n_sh	et al et ux	and others and wife	sec. shldr.	section shoulder	Riprap	
Sheets\A-Gen_sht\SYM_CF	EW exc.	edge of water excavation	spa. spec.	spacing, Spaces or Spaced specification	Sediment Log	
	exp. jt. ext. F f.f.	expansion joint exterior fill face	st. ST	street point of spiral to tangent station	Silt Fence	
N: CO erfo43(1) Roadway CADD_	Fed. FES	federal flared end section	sta. std. stiff.	station standard stiffener	Stabilized Construction Entrance	
yewbe	fin. ftg.	finish footing	str.	straight structural	Temporary Inlet Protection	\bigcirc
(1)\Ro	G ga. galv. adr	gage (gauge) galvanized girder	struc. sym. T T	symmetrical tangent length townshin	Wattle	
f043	gdr. H hdwl.	headwall	tan.	township tangent		
:0ler	HES hex.	homestead entry survey hexagon	TBM TCE	temporary bench mark temporary construction easement	FENCE & CATTLEGUARD SYM	BOLS
N:1C	horiz.	horizontal	transv.	transverse	Fence (Existing, Proposed)	xx
×	HW hwy.	high water highway	TS typ.	point of tangent to spiral typical	Fence w/ Gate (Existing, Proposed)	x—x— ₹~?+x— -x xx xx x≯ < *x xx -x*
15 PM	I ID	inside diameter		design speed		
1:11:15	incl. incr. int.	inclusive,including increment interior	vert. vph VPI	vertical vehicles per hour vertical point of intersection	Cattleguard (Existing, Proposed)	
	J jt.	joint		west		
					GEOLOGIC SYMBOLS	
					Boring Location (Existing, Proposed)	
7/15/2014					Material Source	\searrow
1/1						

CONVENTIONAL PLAN SYMBOLS AND ABBREVIATIONS Sheet 1 of 2

U.S. CUSTOMARY

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

	LANDSCAPING & VEGETATIO	N SYMBOLS	GUARDRAIL, BARRIER & WAL		PROJECT SP
	Tree	** © ** O	Guardrail (Existing, Proposed)		
וחיליושי	Treeline		Guardwall (Existing, Proposed)		
· · · · ·	MAPPING SYMBOLS		Median & Side Barrier (Existing, Proposed)		
	Building (Existing, Proposed)		Retaining Wall (Existing, Proposed)		
	Coordinate Grid Tick		ROADWAY SYMBOLS		
	North Arrow		Clearing/Construction Limits Slope Stake Limits		
	Railroad Single Track		Top of Cut Transition Toe of Fill		
	Double Track	÷•••••••••••••••••••••••••••••••••••••	Edge of Roadway Existing	====>>>====>>>======	
	Spot Elevation	× 999.9	Proposed		
	Trail		Roadway Centerline (With Station ticks)		
	Survey Control Point	-	Roadway Obliteration		
			SIGN SYMBOLS		
	RIGHT-OF-WAY SYMBOLS Boundaries National State County City		Signs Commercial (Existing, Proposed) Delineator (Existing, Proposed) Portable (Proposed) Post Mounted (Existing, Proposed)		
ugueron _ rue nie	<i>Township or Range Line Section ¹/₄ Section ¹/₁₆ Section Bureau of Indian Affairs Bureau of Land Management National Forest National Park National Wildlife Refuge</i>		UTILITY SYMBOLS Irrigation Ditch Underground (Existing, Proposed) Surface (Existing, Proposed) Support Pole (Existing, Proposed) Support Pole Anchor (Existing, Proposed) Street Light (Existing, Proposed) Telephone Booth (Existing, Proposed)		
יוחם שבייות החוור החוור	Easements Permanent (Existing) Permanent (Proposed) Temporary (Proposed)	— — — — — — — — — — — — — — — — — — —	<i>Telephone Pedestal (Existing, Proposed) Underground Utility (Existing, Proposed) CATV Fiber Optic Gas Oil</i>	$\Box^{TP} \qquad \blacksquare^{TP} \qquad \blacksquare^{TP}$ $= \underbrace{\neg}_{TV} \underbrace{\neg}_{FO} \underbrace{\neg}$	
nunu v	Monument (As described)	\otimes	Power Sanitary Sewer		
1/-)	Parcel Number	400	Telephone Water	$+T \vdash+T \vdashT \blacksquare T \vdash+T \blacksquareT \blacksquare T \vdashT \blacksquare T \blacksquareT \blacksquare T \vdashT \blacksquare T \blacksquareT \blacksquare T \T \blacksquare T \blacksquareT \blacksquare T \T \blacksquare T \blacksquareT \blacksquare T \blacksquareT \blacksquare T \blacksquare $	
	Property Line Right-of-Way Line (Existing) Right-of-Way Line (Proposed)		<i>Overhead Utility Line (Existing, Proposed) CATV Fiber Optic Power Telephone</i>		
11 00.1111	Section Corner (Found, Projected)		MISCELLANEOUS SYMBOLS See Note 4	(d)	
+102/01	¼ Section Corner (Found, Projected) ¼6 Section Corner (Found)	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$			
$\langle $			1		1

U.S. CUSTOMARY CONVENTIONAL PLAN SYMBOLS AND ABBREVIATIONS Sheet 2 of 2

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

PECIFIC SYMBOLS

REG	STATE	PROJECT	SHEET NO.	TOTAL SHEETS
IMR	со	ER 43(1) Larimer County CR43 Glen Haven Rd Reconstruction	A3	A7

PROJECT : CO ER 43(1) LARIMER COUNTY ROAD 43

Review FP03, Section 152.02

DATE OF FIELD WORK : JANUARY 2014 DATE OF FINAL ADJUSTMENT : JANUARY 2014

PROJECT UNITS : US SURVEY FOOT COORDINATE SYSTEM : LAMBERT NAD83; CO NORTH 0501 EPOCH DATE : NAD_83(2011)(EPOCH:2010.0000) VERTICAL DATUM : NAVD88 MODELED USING GEOID12A

GPK FILE DATED:

POINT NUMBER	NORTH	EAST	FLEVATION			ELLIPSOID	MADDING	COMPTHER															
			ELEVATION	ELEVATION		ELEVATION 7222.95										LATITUDE	LONGITUDE	HEIGHT	MAPPING ANGLE	COMBINED FACTOR	JOB???.GPK STATION	222 OFFSET	DESCRIPTION
2001 1	1408377.72	3014571.99	7222.95	40°27'15.79208"N	105°26'51.49249"W	7180.77	0°02'02"	0.99961994			FHWA ALUMINUM CAP												
2002 1	1408885.54	3016594.12	7115.66	40°27'20.80766"N	105°26'25.32928"W	7073.30	0°02'19"	0.99962516			FHWA ALUMINUM CAP												
2003 1	1408544.97	3018203.60	7074.28	40°27'17.42103"N	105°26'04.51137"W	7031.75	0°02'32"	0.99962708			FHWA ALUMINUM CAP												
2004 1	1409350.45	3019020.47	7006.35	40°27'25.37465"N	105°25'53.93607"W	6963.77	0°02'39"	0.99963047			FHWA ALUMINUM CAP												
2005 1	1410355.94	3021544.53	6928.44	40°27'35.29029"N	105°25'21.27143"W	6885.62	0°03'00"	0.99963438			FHWA ALUMINUM CAP												
2005 1	1405901.28	3032451.47	6452.25	40°26'51.15182"N	105°23'00.24066"W	6408.13	0°04'31"	0.99965644			FHWA ALUMINUM CAP												

PROJECT AVERAGES = 0.99963224

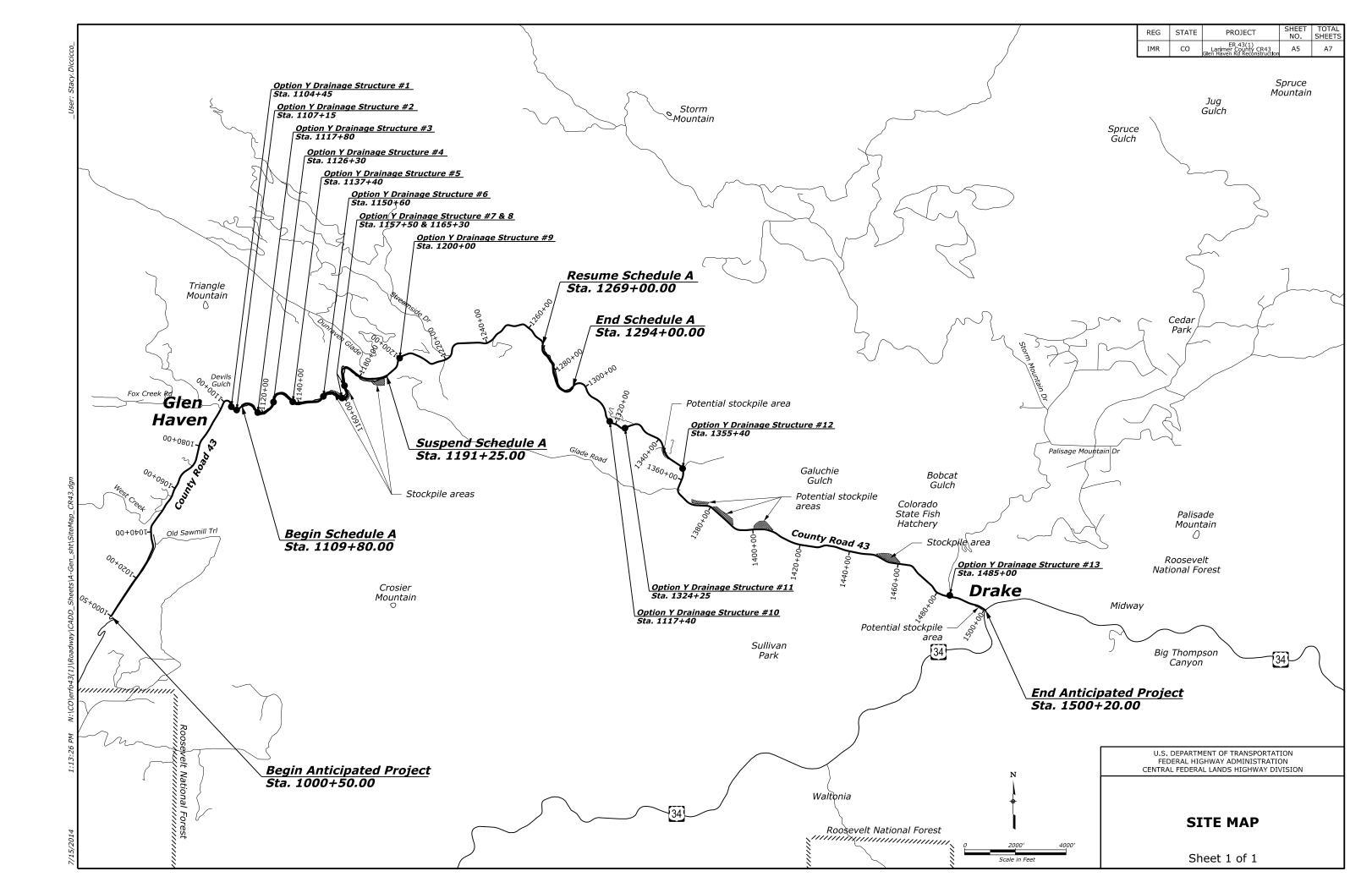
NOTE: TO PRECISELY CHECK DISTANCES BETWEEN POINTS AS MEASURED ON THE GROUND : INVERSE THE STATE PLANE COORDINATES AND DIVIDE THE COMPUTED DISTANCE BY A MEAN COMBINED FACTOR OF THE TWO POINTS.

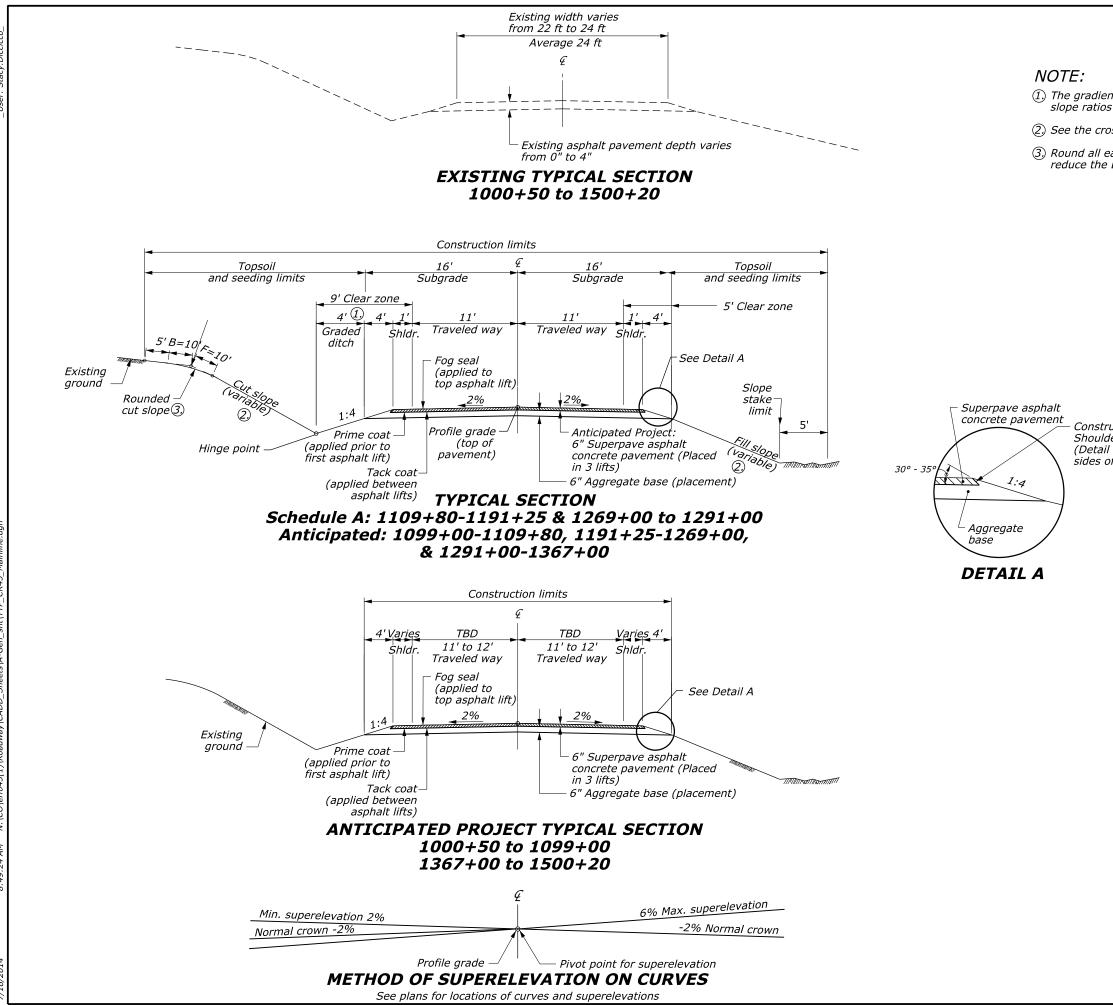
TO COMPUTE GEODETIC AZIMUTHS USE THE FOLLOWING FORMULA : GEODETIC AZIMUTH = GRID AZIMUTH + MAPPING ANGLE

			_	U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION
6				CENTRAL FEDERAL LANDS HIGHWAY DIVISION
5				
4				
3				SURVEY CONTROL
2				Sheet 1 of 1
1	Created	2/3/14	klw	Sheet 1 01 1
NO.	DESCRIPTION REVISIONS (OR CHANGE NOTICES)	DATE	INIT.	

7/15/2014

REG	STATE	PROJECT	SHEET NO.	TOTAL SHEETS
IMR	со	ER 43(1) Larimer County CR43 Glen Haven Rd Reconstruction	A4	A7





REG	STATE	PROJECT	SHEET NO.	TOTAL SHEETS
IMR	со	ER 43(1) Larimer County CR43 Glen Haven Rd Reconstruction	A6	A7

(1) The gradient and width of roadway ditches and the excavation and embankment slope ratios may be adjusted by the CO to assure adequate drainage and stability.

(2) See the cross sections for cut and fill slope ratios.

(3) Round all earth slopes and all rippable rock slopes. For cut heights less than B, reduce the B and F dimensions to the actual cut height.

Construct 30° - 35° pavement edge (safety edge) Shoulder up with aggregate base (Detail applies to both right and left sides of roadway)

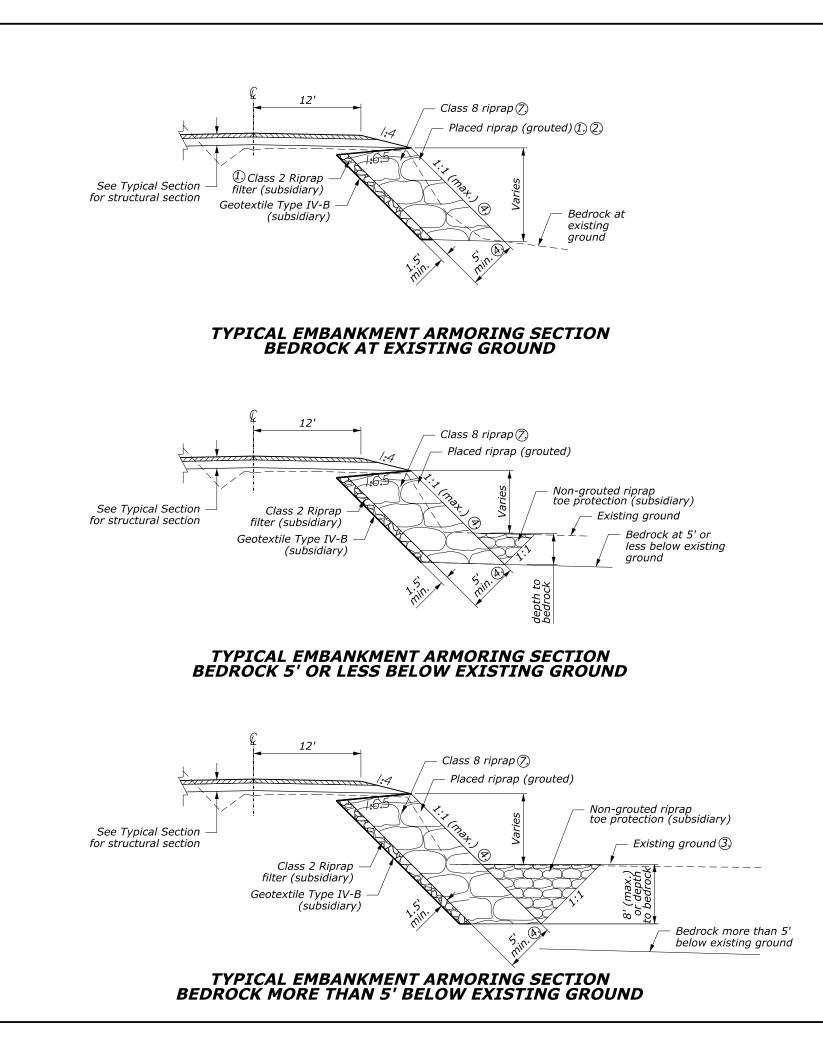
LENGTH	OF PROJE	ECT
Station to Station	Schedule A (ft)	Anticipated Project (ft)
1000+50 to 1109+80		10930
1109+80 to 1191+25	8145	8145
1191+25 to 1269+00		7775
1269+00 to 1294+00	2500	2500
1294+00 to 1500+20		20620
TOTALS (ft)	10645	49970
TOTAL (mi)	2.02	9.46

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

TYPICAL SECTIONS MAINLINE

NO SCALE

Sheet 1 of 2



NOTE:

- ground as directed by the CO.

REG	STATE	PROJECT	SHEET NO.	TOTAL SHEETS
IMR	со	ER 43(1) Larimer County CR43 Glen Haven Rd Reconstruction	A7	A7

(1) In Embankment Armoring areas where the existing riprap is on bedrock and determined by the CO to be hydraulically stable, grout the existing riprap. Riprap filter will not be installed.

(2) In Embankment Armoring areas where the existing riprap is on bedrock and hydraulically unstable, provide one of two repairs as determined by the CO:

• Excavate existing riprap to a minimum thickness of 6.5 feet and replace with riprap filter and hydraulically stable grouted armor riprap.

• Leave existing riprap in-place and grout existing riprap to a thickness of five feet.

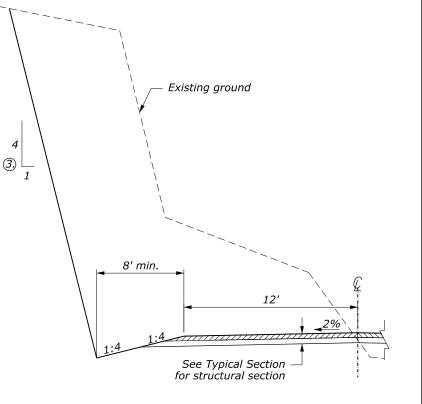
(3) Existing ground elevation will be the lowest point in channel cross section, not the embankment material placed during emergency access repairs. Field verify existing

(4) Grouted armor riprap slope may be steepened to a 3(V):1(H) maximum slope by the CO. When steepened slope is used, base riprap width will be increased to seven feet minimum.

5. Provide grout according to Section 601 of the FP-03, (3000 psi concrete). For estimating purposes, assume 15% grout by volume of riprap.

6. Classification of all riprap will be determined by the CO to fit conditions.

(7) A larger class of riprap may be required in select locations, as determined by the CO.



TYPICAL ROCK FALL DITCH SECTION

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION **TYPICAL SECTIONS** MAINLINE NO SCALE Sheet 2 of 2

SUMMARY OF QUANTITIES - Schedule A

									She	et and Descr	iption				Estimated		
А Л	Line				B5										Quantities	Remarks and/or	
	Item No.	Pay Item Number	Pay Item Description		Unit	Tabulation of Quantities	Allowance									Bid Schedule	Determination of Esti Quantity
	A0100	15101-0000	MOBILIZATION	LPSM											ALL		
	A0150	15206-0000	SLOPE, REFERENCE, AND CLEARING AND GRUBBING STAKE	STA											106.450		
	A0200		SURVEY AND STAKING, GRADE FINISHING STAKES	STA											106.450		
	A0250	15301-0000	CONTRACTOR QUALITY CONTROL	LPSM											ALL		
	A0300	15401-0000	CONTRACTOR TESTING	LPSM											ALL		
	A0350	15501-0000	CONSTRUCTION SCHEDULE	LPSM											ALL		
	A0400	15705-0100	SOIL EROSION CONTROL, SILT FENCE	LNFT											20,000		
	A0450	15705-1500	SOIL EROSION CONTROL, SEDIMENT WATTLE	LNFT											10,000		
	A0500	20101-0000	CLEARING AND GRUBBING	ACRE											20.4		
	A0550	20401-0000	ROADWAY EXCAVATION	CUYD	116,106	5,894									122,000		
	A0600		PLACED RIPRAP (PRODUCTION ONLY)	CUYD	36,825	1,875									38,700		
	A0650	25101-0000	PLACED RIPRAP (GROUTED)	CUYD	15,010	740									15,750		
	A0700	25210-0000	ROCKERY	SQYD											2,000		
	A0750	26001-0000	ROCK BOLT	LNFT											1,000		
	A0800	30103-0000	AGGREGATE BASE (PRODUCTION ONLY)	CUYD	29,347	1,453									30,800		
	A0850	30103-0000	AGGREGATE BASE (PLACEMENT)	CUYD	6,084	316									6,400		
			PULVERIZING, 4-INCH DEPTH	SQYD	5,842	258									6,100		
			MAGNESIUM CHLORIDE	TON											500		
-			PRIME COAT	SQYD	29,489	1,476									30,965		
-			24-INCH PIPE CULVERT	LNFT											5,770		
-			30-INCH PIPE CULVERT	LNFT											1,180		
			36-INCH PIPE CULVERT END SECTION FOR 24-INCH PIPE	LNFT											360		
	A1250		CULVERT END SECTION FOR 30-INCH PIPE	EACH											80		
	A1300		CULVERT END SECTION FOR 36-INCH PIPE	EACH										╞──┨	19		
			CULVERT	EACH				_							6		
			CATCH BASIN, TYPE 1	EACH							 	 			20		
			INLET TOP, METAL FRAME AND GRATE TYPE A	EACH											20		
	A1450		DUMP TRUCK, 10 CUBIC YARD MINIMUM CAPACITY	HOUR											250		

Date Completed: 07/21/14 Report Date: 07/21/14

21-

REG	STATE	PROJECT	SHEET NO.	TOTAL SHEETS
IMR	со	ER 43(1) Larimer County CR43	B1	B5

SUMMARY OF QUANTITIES - Schedule A

								She	et and Descri	ption				Estimated	
1	Line	Davida			B5									Quantities	Remarks and/or
: 	Item No.	Pay Item Number	Pay Item Description	Unit	Tabulation of Quantities	Allowance								Bid Schedule	Determination of Estin Quantity
	A1500	62201-0550	BACKHOE LOADER, 6 CUBIC FOOT												
			MINIMUM RATED CAPACITY												
			BUCKET, 24-INCH WIDTH	HOUR										250	
	A1550	62201-1000	WHEEL LOADER, 4 CUBIC YARD												
			MINIMUM RATED CAPACITY	HOUR										250	
	A1600		BULLDOZER, 350HP MINIMUM												
_			FLYWHEEL POWER	HOUR										250	
	A1650	62201-2850	MOTOR GRADER, 12 FOOT MINIMUM											250	
-	A1700	62201 2000	BLADE HYDRAULIC EXCAVATOR	HOUR HOUR										250 250	
			GENERAL LABOR	HOUR										300	
			SPECIAL LABOR, SLOPE SCALING	HOUR										300	
			SPECIAL LABOR, HIRED	HOUR										300	
			TECHNICAL SERVICES	HOUR										500	
	A1900	62302-1100	SPECIAL LABOR, HIRED SURVEY												
			SERVICES	HOUR										250	
	A1950	63501-1000	TEMPORARY TRAFFIC CONTROL,												
			TRAFFIC AND SAFETY SUPERVISOR	LPSM										ALL	
	A2000	63502-0700	TEMPORARY TRAFFIC CONTROL,												
			CONE	EACH										750	
	A2050	63502-1300	TEMPORARY TRAFFIC CONTROL,												
			DRUM	EACH										100	
	A2100		TEMPORARY TRAFFIC CONTROL,												
			PORTABLE CHANGEABLE MESSAGE												
			SIGN	EACH										8	
	A2150	63502-3100	TEMPORARY TRAFFIC CONTROL,	FAOL										0	
-	42200	62502 0200	TRAFFIC SIGNAL SYSTEM TEMPORARY TRAFFIC CONTROL,	EACH										2	
	A2200		BARRICADE TYPE 3	LNFT										200	
_	A2250		TEMPORARY TRAFFIC CONTROL,											200	
	12200	00000 0400	CONCRETE BARRIER	LNFT										1,000	
	A2300	63503-0500	TEMPORARY TRAFFIC CONTROL,											.,	
			MOVING CONCRETE BARRIER	LNFT										3,000	
	A2350	63504-1000	TEMPORARY TRAFFIC CONTROL,												
			CONSTRUCTION SIGN	SQFT										1,000	
	A2400	63506-0500	TEMPORARY TRAFFIC CONTROL,												
			FLAGGER	HOUR										10,000	
T	A2450	63506-0600	TEMPORARY TRAFFIC CONTROL,												
			PILOT CAR	HOUR										7,000	

Date Completed: 07/21/14 Report Date: 07/21/14

21-

REG	STATE	PROJECT	SHEET NO.	TOTAL SHEETS
IMR	со	ER 43(1) Larimer County CR43	B2	B5

						SU	MMAI	ry of	- QUA		IES ·	- Opti	ion X			REG STATE	PROJECT SHEET TOTAL NO. SHEETS ER 43(1) arimer County CR43 B3 B5
A M	Line				B5					Shee	et and Descri	ption				Estimated Quantities	Remarks and/or
E N D	Item No.	Pay Item Number	Pay Item Description	Unit	Tabulation of Quantities	Allowance										Bid Schedule	Determination of Estimated Quantity
	X0100		SPECIAL LABOR, HIRED														
			TECHNICAL SERVICES	HOUR												200	

MileStone: 100% PS&E Date Completed: 07/21/14 Report Date: 07/21/14

SUMMARY OF QUANTITIES - Option Y

•								She	et and Descr	iption			Estimated	
M	Line	Daviltar			B5								Quantities	Remarks and/or
E N D	Item No.	Pay Item Number	Pay Item Description	Unit	Tabulation of Quantities	Allowance							Bid Schedule	Determination of Estimated Quantity
	Y0100	15101-0000	MOBILIZATION	LPSM									ALL	
	Y0150	15215-2000	SURVEY AND STAKING, BRIDGE	EACH									13	
	Y0200	15301-0000	CONTRACTOR QUALITY CONTROL	LPSM									ALL	
	Y0250	15401-0000	CONTRACTOR TESTING	LPSM									ALL	
	Y0300	20301-1900	REMOVAL OF PIPE CULVERT	EACH									13	
	Y0350	20435-2500	BACKFILL, PERMEABLE	CUYD									8,450	
	Y0400	20801-0000	STRUCTURE EXCAVATION	CUYD									5,330	
	Y0450		PLACED RIPRAP (CLASS 8 GROUTED)	CUYD									5,850	
	Y0500	25904-0000	GEOSYNTHETIC REINFORCEMENT	SQYD									43,056	
	Y0550		PRECAST, PRESTRESSED CONCRETE SLABS, 48" VOIDED	LNFT									5,005	
	Y0600	55601-0900	BRIDGE RAILING, STEEL	LNFT									1,430	
	Y0650	55901-1000	MEMBRANE WATERPROOFING, TYPE 1	SQYD									2,224	
	Y0700		GUARDRAIL SYSTEM G4, TYPE 2, CLASS B STEEL POSTS	LNFT									1,690	
	Y0750	61702-0800	TERMINAL SECTION TYPE TANGENT	EACH									52	
	Y0800	61707-0000	STRUCTURE TRANSITION RAILING	LNFT									975	
	Y0850	62006-0000	CONCRETE MASONRY UNITS	SQYD									585	

REG	STATE	PROJECT	SHEET NO.	TOTAL SHEETS
IMR	СО	ER 43(1) Larimer County CR43	B4	B5

			SCHEDULE	A GRADING SU	MMARY			
	Roadway	Excavation	Pay Item 20401-0000	Additional Excavation	Embar For inf	kment o only	For info only	For info only
Station to Station	Roadway Prism	Approach Roads	ROADWAY EXCAVATION	(-) Unavailable Material (see note 1)	Roadway Prism	Approach Roads	(+) Various Backfill Material Generated Onsite (see note 2)	Waste (see note 3)
	BCY	BCY	CUYD	BCY	ССҮ	ССҮ	ССҮ	CUYD
1109+80 - 1191+25	86,391	0	86,391	240	45,328	0	490	
1269+00 - 1294+00	29,715	0	29,715	0	6,582	0	0	
Emergency Access Embankment (6)					-30,000			
Aggregate Base (production only)							29,347	
Placed Riprap (production only)							36,825	
Class 2 riprap filter (subsidiary)							11,050	
TOTALS	116,106	0	116,106	240	21,910	0	77,712	39,403

NOTE:

1. Unavailable material includes pavement removal in cuts. Assumed 4 inch depth in undamaged areas, 0 inch depth in damaged areas. 2. Various backfill material generated onsite includes pavement removal under fill, aggregate base (production), placed riprap

(production) and class 2 riprap filter (subsidiary). Additional uses for material generated onsite are anticipated (potentially rockery rocks, rock backdrain and GRS backfill).

3. Waste or Unclassified borrow quantity calculated using volumes adjusted for an estimated 1.2 swell.

4. The quantities shown herein are approximations. Payment will be made for the actual quantities of work performed.

5. BCY = Bank cubic yard - one cubic yard of material as it lies in the natural state.

CCY = Compacted cubic yard - one cubic yard of material after it has been compacted to specification density.

6. Earthwork quantities are based on contour data obtained prior to Emergency Access repairs. Embankment quantity adjusted to approximately account for embankment placed during Emergency Access repair work.

	F		ATION ONLY:	ANTICIPATED PROJE	ECT SURFACING	SUMMARY		
Item Number	for info only	for info only	for info only	for info only	for info only	for info only	for info only	for info only
Station to Station	AGGREGATE BASE (PLACEMENT)	PULVERIZING, 4 INCH DEPTH	MAGNESIUM CHLORIDE	SUPERPAVE PAVEMENT, 1-INCH NOMINAL MAXIMUM SIZE AGGREGATE, EQUAL OR >30 MILLION ESAL	ANTISTRIP ADDITIVE, TYPE 3	FOG SEAL, EMULSIFIED ASPHALT GRADE CSS-1 OR CSS-1H, SS-1 OR SS-1H	PRIME COAT	таск соат
	CUYD	SQYD	TON	TON	TON	TON	SQYD	TON
1000+50 - 1109+80.	6272	25471		9946.9	99.5	19.6	45670	19.6
1109+80 - 1191+25.				7467.8	74.7	14.7	34287	14.7
1191+25 1269+00.	4510	13916		7024.8	70.2	13.8	32253	13.8
1269+00 1294+00.				2365.4	23.7	4.7	10860	4.7
1294+00 1500+20.	12480	54853		19819.1	198.2	39.1	90997	39.1
Project wide			1500					
TOTALS	23,263	94,240	1,500	46624.0	466.3	91.9	214067	91.9

	SCHEDULE	A SURFACI	NG SUMMARY	,	
Item Number	30103-0000	30103-0000	30306-2000	30606-0000	41103-0000
Station to Station	AGGREGATE BASE (PRODUCTION ONLY)	AGGREGATE BASE (PLACEMENT)	PULVERIZING, 4 INCH DEPTH	MAGNESIUM CHLORIDE	PRIME COAT
	CUYD	CUYD	SQYD	TON	SQYD
1109+80 - 1191+25.	4640	4640	5842		22521
1269+00 1294+00.	1445	1445	0		6968
Project Wide	23263			500	
TOTALS	29,347	6,084	5,842	500	29,489

Values used for estimating purposes:

Aggregate base . . . 139 lb/ft3 Magnesium chloride 0.20 gal/sqyd, 8.3 lb/gal

SCH	HEDULE A F		RIPRAP S	SUMMARY
Item Number ->	25101-0000	25101-0000	for info only	
Station to Station	PLACED RIPRAP (PRODUCTION ONLY)	PLACED RIPRAP (GROUTED)	grout	NOTES
	CUYD	CUYD	CUYD	
1109+80 1191+25.	10640	10640	1596	Assume grout 15% by volume
1269+00 1294+00.	4370	4370	656	Assume grout 15% by volume
Project Wide	21815			
TOTALS	36825	15010	2252	

Values used for estimating purposes:

Aggregate base . . . 139 lb/ft3

Hot asphalt concrete pavement . . . 145.2 lb/ft3

Antistrip 1 % of weight of mix

Fog seal . . . 0.1 gal/yd2 (240.7 gal/ton)

Prime coat . . . 0.33 gal/yd2 (251 gal/ton)

Tack coat . . . 0.1 gal/yd2 (233 gal/ton)

Magnesium Chloride . . . 0.2 gal/yd2 (8.3 lb/gal)

REG	STATE	PROJECT	SHEET NO.	TOTAL SHEETS
IMR	со	ER 43(1) Larimer County CR43 Glen Haven Rd Reconstruction	B5	B5

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION
TABULATION OF QUANTITIES Sheet 1 of 1

